MORE THAN ARCHITECTS

ADDENDUM

NO. 1

TO THE DRAWINGS AND THE PROJECT MANUAL

PROJECT NAME: Lehman High School 2025 Additions and Renovations

CLIENT NAME: Hays CISD

LOCATION: KYLE, TX

PROJECT NUMBER: 1954-08-01

PROPOSAL DATE: 20 May, 2025

ADDENDUM DATE: 08 May, 2025

For additional information regarding this project, contact Gigi Morgan at 800.687.1229.

THIS ADDENDUM INCLUDES:

Civil Items	8 Pages
Sports Items	4 Pages
Structural Items	21 Pages
Architectural Items	20 Pages
Plumbing Items	1 Pages
Electrical Items	3 Pages

AND ALL ATTACHED REVISED SPECIFICATION & DRAWING REFERENCES IN THE ADDENDUM

Huckabee

Project Name: Lehman High School 2025 Additions and Renovations
Client: Hays CISD
Kyle, TX
Project Number: 1954-08-01

CIVIL ITEMS FOR ADDENDUM NO. 1

NOTICE TO PROPOSERS:

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REFERENCE IS MADE TO THE DRAWINGS AS NOTED:

DRAWINGS:

AD No 1, Civil Item 1: To the Drawings, Sheet C2.02, "DEMOLITION PLAN (2 OF 2),"
1) Wall and sidewalk added to demolition scope.

AD No 1, Civil Item 2: To the Drawings, Sheet C3.00, "EROSION CONTROL PLAN PHASE 1,"
1) Limits of disturbance increased.
2) Additional inlet protection added.

AD No 1, Civil Item 3: To the Drawings, Sheet C3.01, "EROSION CONTORL PLAN PHASE 2,"
1) Limits of disturbance increased.

AD No 1, Civil Item 4: To the Drawings, Sheet C4.02, "DIMENSION CONTROL PLAN (2 OF 2),"
1) Additional sidewalk area.
2) Addition of tree planters.

AD No 1, Civil Item 5: To the Drawings, Sheet C5.01, "PAVING PLAN,"
1) Additional sidewalk area.

AD No 1, Civil Item 6: To the Drawings, Sheet C6.02, "GRADING PLAN (2 OF 2),"
1) Additional sidewalk and grading area.

AD No 1, Civil Item 7: To the Drawings, Sheet C6.03, "PVR PLAN (1 OF 2),"
1) Reduction of PVR boundaries.

AD No 1, Civil Item 8: To the Drawings, Sheet C6.04, "PVR PLAN (2 OF 2),"
1) Reduction of PVR boundaries.

END OF CIVIL ADDENDUM

Huckabee

Project Name: Lehman High School 2025 Additions and Renovations
Client: Hays CISD
Kyle, TX
Project Number: 1954-08-01

05.08.25

ARCHITECTURAL ITEMS FOR ADDENDUM NO. 1

NOTICE TO PROPOSERS:

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REFERENCE IS MADE TO THE DRAWINGS AND THE PROJECT MANUAL AS NOTED:

PROJECT MANUAL:

AD. No 1, Arch. Item 1 **To the Project Manual, Section 00-3132, "GEOTECHNICAL DATA"**
Section replaced in its entirety to Project Manual

AD. No 1, Arch. Item 2 **To the Project Manual, Section 01-2300, "ALTERNATES,"**
Section replaced in its entirety to Project Manual

AD. No 1, Arch. Item 3 **To the Project Manual, Section 10-1400, "SIGNAGE"**
Section replaced in its entirety to Project Manual
Omit 1.01 D. Double Sided Electronic Message Sign
Omit 2.01 C Double Sided Electronic Message Sign Information
Omit 2.02 F Double Sided Electronic Message Sign Information
Omit 3.03 C Electronic Message Sign information

DRAWINGS:

AD. No 1, Arch. Item 4 **To the Drawings, Sheet G1.02, "GENERAL DATA,"**
1) Added alternates to reflect alternate scope

AD. No 1, Arch. Item 5 **To the Drawings, Sheet AS1.01 - "ARCHITECTURAL SITE PLAN,"**
1) Updated civil background

AD. No 1, Arch. Item 6 **To the Drawings, Sheet AD1.01 - "DEMOLITION FLOOR PLAN – AREA B,"**
1) Updated demolition scope

AD. No 1, Arch. Item 7 **To the Drawings, Sheet AD1.02 - "DEMOLITION FLOOR PLAN – AREA C,"**
1) Updated demolition scope and added more views for clarification

Huckabee

AD. No 1, Arch. Item 8 **To the Drawings, Sheet AD1.03 - "DEMOLITION FLOOR PLAN – AREA D,"**

1) Updated demolition scope and added more views for clarification

AD. No 1, Arch. Item 9 **To the Drawings, Sheet A1.1A1 - "FLOOR PLAN - AREA A,"**

1) Updated grid spacing
2) Updated field striping arrows

AD. No 1, Arch. Item 10 **To the Drawings, Sheet A1.1B1 - "FLOOR PLAN – AREA B – LEVEL 1,"**

1) Updated bleacher configuration
2) Updated athletic dividing curtain location

AD. No 1, Arch. Item 11 **To the Drawings, Sheet A1.1B2 - "FLOOR PLAN – AREA B – LEVEL 2,"**

1) Updated wall type tag for clarification
2) Updated opening to a door with card reader
3) Added wall section detail for clarification

AD. No 1, Arch. Item 12 **To the Drawings, Sheet A1.20 - "ENLARGED PLANS,"**

1) Updated bleacher layout, bleacher dimensions, and seat counts

AD. No 1, Arch. Item 13 **To the Drawings, Sheet A1.40, "PLAN DETAILS,"**

1) Refer to attached drawings 1, 3, 18, 19 to show updated fence location

AD. No 1, Arch. Item 14 **To the Drawings, Sheet A2.1C1, "REFLECTED CEILING PLAN – AREA C – LEVEL 1,"**

1) Changed ceiling expansion joint call outs to section markers

AD. No 1, Arch. Item 15 **To the Drawings, Sheet A2.1C2, "REFLECTED CEILING PLAN – AREA C – LEVEL 2,"**

1) Changed ceiling expansion joint call outs to section markers

AD. No 1, Arch. Item 16 **To the Drawings, Sheet A3.01, "DOOR AND SPECIAL OPENING SCHEDULES AND CONFIGURATIONS,"**

1) Updated schedule to include new door
2) Updated counter coiling door detail

AD. No 1, Arch. Item 17 **To the Drawings, Sheet A4.1B1, "FINISH PLAN – AREA B,"**

1) Changed flooring material from resinous to tile in restrooms.

AD. No 1, Arch. Item 18 **To the Drawings, Sheet A4.1C1, "FINISH PLAN – AREA B,"**

1) Changed flooring material from resinous to tile in restrooms.

AD. No 1, Arch. Item 19 **To the Drawings, Sheet A4.1E1, "FINISH PLAN – AREA E"**

1.) Changed flooring material from resinous to sealed concrete in multiuser restrooms and concessions areas.
2.) Added Turf to Rubber transition detail in weight room area

AD. No 1, Arch. Item 20 **To the Drawings, Sheet A4.14, "TYPICAL INTERIOR FINISH DETAILS"**

1.) Added Turf to Rubber transition detail for weight room area

AD. No 1, Arch. Item 21 **To the Drawings, Sheet A5.02, "EXTERIOR ELEVATIONS – AREA B & C,"**

1) Changed existing conditions and grade line at all exterior elevations

AD. No 1, Arch. Item 22 **To the Drawings, Sheet A5.03, "EXTERIOR ELEVATIONS - AREA D & E,"**

1) Changed grade line to reflect civil grade at 3/A5.03 & 6/A5.03
2) Added gutters and sloped canopy roofs at 2/A5.03
3) Cleaned up graphics to reflect proposed design at 5/A5.03

AD. No 1, Arch. Item 23 **To the Drawings, Sheet A6.05, "WALL SECTIONS – AREAS A, B,"**

1) Refer to attached drawings 1, 2, 3 to show updated fence locations
2) Refer to attached drawing 1 for clarification on bottom of steel brace height

AD. No 1, Arch. Item 24 **To the Drawings, Sheet A7.12, "TYPICAL COMMON ROOFING DETAILS,"**

1) Added detail

END OF ARCHITECTURAL ADDENDUM

Project Name: Lehman High School 2025 Additions and Renovations
Client: Hays CISD
Kyle, TX
Project Number: 1954-08-01

STRUCTURAL ITEMS FOR ADDENDUM NO. 1

NOTICE TO PROPOSERS:

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REFERENCE IS MADE TO THE DRAWINGS AND THE PROJECT MANUAL AS NOTED:

PROJECT MANUAL:

DRAWINGS:

AD No 1, Struct Item 1: **To the Drawings, Sheet S1.1, "GENERAL NOTES,"**
1) Section 1.1.12 – Updated grading elevation to align with civil.
2) Section 1.3.2 and 1.3.6 – Clarified loading.
3) Section 3.2.1 – Clarified steel grades.
4) Section 3.2.6 – Clarified concrete cover.
5) Section 3.3.1 – Revised concrete strength where shown.

AD No 1, Struct Item 2: **To the Drawings, Sheet S2.1A1, "FOUNDATION PLAN - AREA A,"**
1) Revised grids and perimeter columns where shown.

AD No 1, Struct Item 3: **To the Drawings, Sheet S2.1A2, "ROOF FRAMING PLAN - AREA A,"**
1) Revised grids and perimeter columns where shown.

AD No 1, Struct Item 4: **To the Drawings, Sheet S2.1B1, "FOUNDATION PLAN - AREA B,"**
1) Updated detail references and elevations where shown.

AD No 1, Struct Item 5: **To the Drawings, Sheet S2.1B2, "LEVEL 2 FRAMING PLAN - AREA B,"**
1) Updated detail reference where shown.

AD No 1, Struct Item 6: **To the Drawings, Sheet S2.1B3, "ROOF FRAMING PLAN - AREA B,"**
1) Clarified loading where shown.

AD No 1, Struct Item 7: **To the Drawings, Sheet S2.1C1, "FOUNDATION PLAN - AREA C,"**
1) Clarified demolition extent.
2) Updated detail refences where shown.

AD No 1, Struct Item 8: **To the Drawings, Sheet S2.1C3, "ROOF FRAMING PLAN - AREA C,"**
1) Clarified RTU loading.

AD No 1, Struct Item 9: **To the Drawings, Sheet S2.1D1, "FOUNDATION PLAN - AREA D,"**
1) Clarified canopy base detailing.

AD No 1, Struct Item 10: **To the Drawings, Sheet S2.1E1, "FOUNDATION PLAN - AREA E,"**
1) Revised beam sizes where shown.

AD No 1, Struct Item 11: **To the Drawings, Sheet S3.3, "CONCRETE DETAILS,"**
1) Detail 12 – Revised beam dimensions and reinforcing.

AD No 1, Struct Item 12: **To the Drawings, Sheet S3.4, "CONCRETE DETAILS,"**
1) Detail 12 – Revised EJ detailing.

AD No 1, Struct Item 13: **To the Drawings, Sheet S3.5, "CONCRETE DETAILS,"**
1) Detail 4 – New detail.

AD No 1, Struct Item 14: **To the Drawings, Sheet S3.7, "CONCRETE DETAILS,"**
1) Detail 22 – New Detail.

AD No 1, Struct Item 15: **To the Drawings, Sheet S4.1, "TYPICAL MASONRY DETAILS,"**
1) Detail 15 – Clarified joist support.

AD No 1, Struct Item 16: **To the Drawings, Sheet S4.2, "MASONRY DETAILS,"**
1) Detail 6 – Clarified brick support detailing.

AD No 1, Struct Item 17: **To the Drawings, Sheet S5.2, "TYPICAL STEEL DETAILS,"**
1) Detail 3 – Revised girder reinforcing.

AD No 1, Struct Item 18: **To the Drawings, Sheet S5.3, "TYPICAL STEEL DETAILS,"**
1) Detail 4 – Revised stair perimeter detailing.

AD No 1, Struct Item 19: **To the Drawings, Sheet S5.4, "STEEL DETAILS,"**
1) Details 7, 14, 15, 17, 19, 20, 21 – Modified details where shown.
2) Detail 23 – New detail.

AD No 1, Struct Item 20: **To the Drawings, Sheet S5.5, "STEEL DETAILS,"**
1) Details 4 and 5 – New details.

AD No 1, Struct Item 21: **To the Drawings, Sheet S5.6, "STEEL DETAILS,"**
1) Detail 18 – New detail.
2) Detail 24 – Modified detail where shown.

END OF STRUCTURAL ADDENDUM

Project Name: Lehman High School 2025 Additions and Renovations
Client: Hays CISD
Kyle, TX
Project Number: 1954-08-01

ELECTRICAL ITEMS FOR ADDENDUM NO. 1

NOTICE TO PROPOSERS: **05/08/2025**

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REFERENCE IS MADE TO THE DRAWINGS AND THE PROJECT MANUAL AS NOTED:

PROJECT MANUAL:

AD No 1, Elec Item 1: **To the Project Manual, Sections as listed below:**

1) Added Section 26 4113, "LIGHTNING PROTECTION SYSTEM," in its entirety.

DRAWINGS:

AD No 1, Elec Item 2: **To the Drawings, Sheet E2.01C1, "FIRST FLOOR PLAN - AREA C - LIGHTING,"**

1) Adjusted lighting locations in C103 and C111.

AD No 1, Elec Item 3: **To the Drawings, Sheet E2.02C2, "SECOND FLOOR PLAN - AREA C - LIGHTING,"**

1) Adjusted lighting locations in C203 and C211.

AD No 1, Elec Item 4: **To the Drawings, Sheet ES1.00, "SITE PLAN - ELECTRICAL,"**

1) Added notation on front parking alternate as shown.

END OF ELECTRICAL ADDENDUM

Huckabee

Project Name: Lehman High School 2025 Additions and Renovations
Client: Hays CISD
Kyle, TX
Project Number: 1954-08-01

05/08/2025

FIRE PROTECTION ITEMS FOR ADDENDUM NO. 1

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REFERENCE IS MADE TO THE DRAWINGS AND THE PROJECT MANUAL AS NOTED:

PROJECT MANUAL:

AD No 1, F Item 1: **To the Project Manual, Section 21 0000, "Fire Protection,"**
To paragraph 1.01, B, 5, Standpipes shall have fire department valve in cabinet.

END OF FIRE PROTECTION ADDENDUM

Huckabee

Project Name: Lehman High School 2025 Additions and Renovations
Client: Hays CISD
Kyle, TX
Project Number: 1954-08-01

05/08/2025

PLUMBING ITEMS FOR ADDENDUM NO. 1

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REFERENCE IS MADE TO THE DRAWINGS AND THE PROJECT MANUAL AS NOTED:

DRAWINGS:

AD No 1, Plumb Item 1: **To the Drawings, Sheet P2.01A1, "First Floor Plan – Area A - Plumbing,"**

1) Added entire new sheet.

END OF PLUMBING ADDENDUM

Project Name: Lehman High School 2025 Additions
Client: Hays CISD
Kyle, TX
Project Number: 1954-08-01

2025-05-06

F-7524

SPORTS ITEMS FOR ADDENDUM NO. 1

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REFERENCE IS MADE TO THE DRAWINGS AND THE PROJECT MANUAL AS NOTED:

SPECS:

AD No 1, Sports Item 1: **To the Specs, Section 31 0010, "EARTHWORK AND GRADING,"**

1) Updated all parts of this spec section to reflect the new Geotech report.

DRAWINGS:

AD No 1, Sports Item 2: **To the Drawings, Sheet F1**

1) Sheet labeled as Alternate #1.
2) Adjusted field event layout within detention pond to correspond with civils new background.
3) Adjusted viewport to fit new layout of field events.
4) Added two shotputs in existing detention pond and called out with detail 92F.

AD No 1, Sports Item 3: **To the Drawings, Sheet F2**

1) Sheet labeled as Alternate #1.
2) Adjusted field event layout within detention pond to correspond with civils new background.
3) Adjusted viewport to fit new layout of field events.
4) Removed fence and gates from sports plans.
5) Updated note 73A to say "Soil shall be stabilized with 8 inches of lime stabilization. Sub-grade shall be stabilized and compacted subgrade per geotech report to achieve PRV of 1.5 inches."

AD No 1, Sports Item 4: **To the Drawings, Sheet F3**

1) Sheet labeled as Alternate #1.
2) Removed all mention of alternate #1 from all utility boxes.

AD No 1, Sports Item 5: **To the Drawings, Sheet F4**

1) Sheet labeled as Alternate #1.
2) Removed fence detail 73N and gate detail 73Q.
3) Added detail 92F for shotput.

Huckabee

4) Added "Soil shall be stabilized with 8 inches of lime stabilization. Sub-grade shall be stabilized and compacted subgrade per geotech report to achieve PRV of 1.5 inches." To detail 73D.

END OF SPORTS ADDENDUM

SECTION 00 3132
GEOTECHNICAL DATA

PART 1 - GENERAL

1.01 SUMMARY

A. Related Documents: General and Supplementary Conditions of the Contract, Division 01 General Requirements, and Drawings are applicable to this Section.

1.02 INVESTIGATION

A. An investigations of subsurface soil conditions at the building site was authorized by the Owner, and these investigations were made by USE Professional Solutions 44, LLC and are attached as follows:
   1. Report number 24-0925, dated September 4, 2024.
   2. Report number A251017, dated April 16, 2025.

1.03 REPORT

A. The complete report of the testing laboratory follows this section and is provided for information only.

B. Report and log of borings are available for Contractor's information but is not a warranty of subsurface conditions, nor is it a part of the Contract Documents.

1.04 RESPONSIBILITY

A. Bidders are expected to examine the site and subsurface investigation reports and then decide for themselves the character of the materials to be encountered.

B. The Owner and Architect assume no responsibility for variations of subsoil quality or conditions.

C. The Owner and the Architect assume no responsibility for any conclusions or interpretations made on the basis of subsurface information contained in the contract documents.

PART 2 - NOT USED

PART 3 - NOT USED

END OF SECTION

GEOTECHNICAL EXPLORATION

ADDITIONAL PARKING – LEHMAN HIGH SCHOOL

1700 Lehman Road
Kyle, Texas
UES Report No. 24-0925
September 4, 2024

Prepared for:

HAYS CONSOLIDATED INDEPENDENT SCHOOL DISTRICT
21003 IH - 35
Kyle, Texas 78640

Attention: Nate Wensowitch

Prepared By:

UES™

September 4, 2024

Hays Consolidated Independent School District
21003 IH - 35
Kyle, Texas 78640

Attention: Nate Wensowitch

Re: Geotechnical Exploration
Additional Parking – Lehman High School
1700 Lehman Road
Kyle, Texas
UES Report No. 24-0925

Attached is the report of the geotechnical exploration performed for the project referenced above. This study was authorized by Nathan Wensowitch on July 24, 2024 and performed in accordance with accordance with UES Professional Solutions 44, LLC (hereinafter UES) Proposal No. 24-1455, dated July 15, 2024.

This report contains results of field explorations and laboratory testing and an engineering interpretation of these with respect to available project characteristics. The results and analyses were used to develop recommendations to aid design and construction of foundations and pavement.

UES Professional Solutions 44, LLC appreciates the opportunity to be of service on this project. If we can be of further assistance, such as providing materials testing services during construction, please contact our office.

Sincerely,
UES PROFESSIONAL SOLUTIONS 44, LLC

Sebastian L. Aleman
Geotechnical Project Manager

Lee E. Gurecky, P.E.
Geotechnical Department Manager – San Antonio

SLA/LEG
Copies: (1-PDF) Nathan Wensowitch, Hays CISD

TABLE OF CONTENTS

UES REPORT NO. 24-0925

1.0 PURPOSE AND SCOPE

The purpose of this geotechnical exploration is for UES Professional Solutions 44, LLC (UES) to evaluate for Hays Consolidated Independent School District (Client) some of the physical and engineering properties of subsurface materials at selected locations on the subject site with respect to formulation of appropriate geotechnical design parameters for the proposed construction. The field exploration was accomplished by securing subsurface samples from widely spaced test borings performed across the expanse of the site. Engineering analyses were performed from results of the field exploration and results of laboratory tests performed on representative samples.

Also included are general comments pertaining to reasonably anticipated construction problems and recommendations concerning earthwork and quality control testing during construction. This information can be used to evaluate subsurface conditions and to aid in ascertaining construction meets project specifications.

Recommendations provided in this report were developed from information obtained in test borings depicting subsurface conditions only at the specific boring locations and at the particular time designated on the logs. Subsurface conditions at other locations may differ from those observed at the boring locations, and subsurface conditions at boring locations may vary at different times of the year. The scope of work may not fully define the variability of subsurface materials and conditions that are present on the site.

The nature and extent of variations between borings may not become evident until construction. If significant variations then appear evident, our office should be contacted to re-evaluate our recommendations after performing on-site observations and possibly other tests.

2.0 PROJECT CHARACTERISTICS

It is proposed to construct a new approximate 19,000 SF parking area. The project site is located at the existing Lehman High School at 1700 Lehman Road in Kyle, Texas. A site plan illustrating the general outline of the property is provided as Figures 1A to 1C, the Boring Location Plan, in the Appendix.

We understand the proposed parking will be designed for both Asphalt Concrete Pavements and Portland Concrete Pavements. Grading plans were not provided to us for this study. For the purpose of our analysis, we have assumed maximum cuts and fills of 2 ft to achieve final grades. Grading plans should be provided to UES prior to design finalization.

2.1 Pre-Existing Conditions

During our field exploration activities for this project, our field representative observed that the site is currently an operating High School green area between an existing parking area and Bunton Creek Road. The area is relatively flat without a noticeable slope. Vegetation at the site is generally comprised of grass and weeds. No rock out-crops were observed at this site.

3.0 FIELD EXPLORATION

Subsurface conditions on the site were explored by drilling a total of two (2) test bores to their planned termination depth of 15 ft. The test borings were drilled in general accordance with ASTM Standard D 420 using air rotary drilling equipment. The approximate location of each test boring is shown on the Boring Location Plans, Figure 1B and 1C, enclosed in the Appendix. Details of drilling and sampling operations are briefly summarized in Methods of Field Exploration, Section A-1 of the Appendix.

Subsurface types encountered during the field exploration are presented on Log of Boring sheets included in the Appendix. The boring logs contain our Field Technician's and Engineer's interpretation of conditions believed to exist between actual samples retrieved. Therefore, these boring logs contain both factual and interpretive information. Lines delineating subsurface strata on the boring logs are approximate and the actual transition between strata may be gradual.

4.0 LABORATORY TESTS

Selected samples of the subsurface materials were tested in the laboratory to evaluate their engineering properties as a basis in providing recommendations for foundation design and earthwork construction. A brief description of testing procedures used in the laboratory can be found in Methods of Laboratory Testing, Section B-1 of the Appendix. Individual test results are presented on the Log of Boring sheets enclosed in the Appendix.

5.0 GENERAL SUBSURFACE CONDITIONS

The Geologic Map of Texas, published by the University of Texas at Austin, Bureau of Economic Geology, has mapped the Pecan Gap Chalk (Kpg) formation in the general area of the project site. The Pecan Gap Chalk formation generally consists of clay, chalk, chalky marl, and limestone.

Within the 15-ft maximum depth explored on the site, subsurface materials consist generally of non-plastic to high plasticity FILL: LEAN CLAY with SAND (CL) and FILL: POORLY GRADED SILTY SAND (SP-SM), overlying LEAN CLAY with SAND (CL). Fill material was encountered to depths of about 4½ to 6½ ft below the existing ground surface in the borings at this site. The letters in parenthesis represent the soils' classification according to the Unified Soil Classification System (ASTM D 2488). More detailed stratigraphic information is presented on the boring logs attached to this report.

The clayey materials encountered are considered relatively impermeable and are anticipated to have a relatively slow response to water movement. The granular materials are considered relatively permeable and are anticipated to have a relatively fast response to water movement. Therefore, several days of observation would be required to evaluate actual groundwater levels within the depths explored. Also, the groundwater level at the site is anticipated to fluctuate seasonally depending on the amount of rainfall, prevailing weather conditions and subsurface drainage characteristics.

Groundwater was not encountered at the boring locations during drilling at this site. However, it is common to detect seasonal groundwater from natural fractures within the clayey matrix, in and the granular materials, particularly during or after periods of precipitation. If more detailed groundwater information is required, monitoring wells or piezometers can be installed. Further details concerning subsurface materials and conditions encountered can be obtained from the boring logs provided in the Appendix. *Note: Granular materials were encountered in the borings at this site. From our experience, these materials can be difficult to excavate (including trenching) and could require forming and/or casing especially if groundwater is encountered during construction.*

6.0 DESIGN RECOMMENDATIONS

The following design recommendations were developed on the basis of the previously described Project Characteristics (Section 2.0) and General Subsurface Conditions (Section 5.0). Should the project criteria change, including the construction location on the site, our office should conduct a review to determine if modifications to the recommendations are required. Further, it is recommended our office be provided with a copy of the final plans and specifications for review prior to construction.

6.1 General Considerations

Design criteria given in this report were developed assuming final grades are within 2 ft of existing grade. Substantial cutting and filling (more than 2 ft) on the site can alter our recommendations. Therefore, it is recommended UES be contacted before performing other cutting and filling on site to verify the appropriate design parameters are utilized for final design.

6.1.1 Existing Fill

As stated in Section 5.0, existing fill was encountered to a depths of up to 6½ ft below the existing ground surface in the borings at this site. Such fill may also exist in areas other than those explored to greater or lesser depths. We understand the fill was placed during initial development of the school and was tested by others. Evaluation of the consistency and compaction of the existing fill are considered beyond the scope of this study. The following recommendations are predicated upon our understanding that all of the existing fill encountered on the site meets or exceeds the recommendations contained in

Section 7.3 of this report. *Note: It is recommended the Client obtain written confirmation from a Professional Engineer that the existing fill was tested upon placement and has been adequately compacted for support of the proposed pavements.*

If documentation of the fill is not available, removal and replacement of all fill is the only method of eliminating the risk of unusual settlement associated with the fill. Samples obtained of the uncontrolled fill were generally free of significant voids. In the absence of documented density control, the possibility of under-compacted zones or voids may exist within the uncontrolled fill.

Although not encountered at the borings, uncontrolled fills can contain boulders, rubble, debris, organic materials, and other unsuitable materials. Excavation and grading contracts should contain provision for removal of unsuitable materials. Test pits could be performed prior to construction to assess the depth, lateral extent, and nature of the existing fill. UES would be pleased to assist with a test pit program if desired.

6.1.2 Vertical Movements

Expansive soils are present at this site. This report provides recommendations to help the effects of soil shrinkage and expansion. However, even if these recommendations are followed, some movement and cracking in the pavements should be anticipated. The severity of cracking and other damage such as uneven pavements will probably increase if any modification of the site results in excessive wetting or drying of the expansive soils.

Grade supported pavements at this site could experience soil-related seasonal movement (i.e. PVR) up to about 2 inches. *Note: These PVR values were estimated using on-site or similar soil with a Plasticity Index (PI) of 30 or less to raise grades a maximum of 2 ft.*

These potential seasonal movements were estimated in general accordance with methods outlined by the Texas Department of Transportation (TxDOT) Test Method Tex-124-E, using swell tests (ASTM D 4546, Method B), engineering judgment, and experience. The estimated movement was calculated assuming the moisture content of the in-situ soil within the normal zone of seasonal moisture content change varies between a "dry" condition and a "wet" condition as defined by Tex-124-E.

Movements exceeding those predicted above could occur if positive drainage of surface water is not maintained or if soils are subject to an outside water source, such as leakage from a utility line or subsurface moisture migration from off-site locations. However, soil movements may be reduced by implementing the subgrade improvement recommendations presented below, in Section 6.2 of this report.

6.2 Pavements

The soils encountered near the ground surface should be improved and prepared prior to construction of pavements at this site. To permit correlation between information from the borings and actual subgrade conditions exposed during construction, a qualified UES Geotechnical Engineer should be retained to provide subgrade monitoring and testing during construction. If there is any change in project criteria, the recommendations contained in this report should be reviewed by our office.

Calculations used to determine the required pavement thickness are based only on the physical and engineering properties of the materials and conventional thickness determination procedures. Pavement joining the buildings should be constructed with a curb and the joint between the building and curb should be sealed. Related civil design factors such as subgrade drainage, shoulder support, cross-sectional configurations, surface elevations, reinforcing steel, joint design and environmental factors will significantly affect the service life and must be included in preparation of the construction drawings and specifications, but were not included in the scope of this study. Normal periodic maintenance will be required for all pavements to achieve the design life of the pavement system.

Recommendations for both Portland Cement Concrete (PCC) and asphalt concrete pavements are provided below. These types of pavement are not considered equal in performance. Over the life of the pavement structure, asphalt concrete pavement should be expected to have a shorter life and higher maintenance costs. Also, pavement in dumpster areas and areas receiving heavy truck traffic should consist of PCC. The dumpster pads should be extended to include all wheels of any garbage trucks.

Based on our knowledge of the project, we anticipate that traffic loads will be produced primarily by automobile traffic, occasional delivery, and trash removal trucks. For this project General Parking and Access Drives pavement section alternatives are provided. General Parking is for areas expected to receive only car traffic. Access Drives assumes areas with some delivery truck traffic, trash trucks, and main access drive areas. If heavier traffic loading is expected, UES should be provided with the information and allowed to review these pavement sections.

Note: The recommended pavement sections provided below are considered the minimum necessary to provide satisfactory performance based on the expected traffic loading. In some cases, City minimum standards for pavement section construction may exceed those provided below.

6.2.1 Pavement Subgrade Preparation

After final subgrade elevation has been achieved, the exposed subgrade preparation should consist of scarifying the exposed subgrade soils to a depth of at least 6 inches and then lime treating or recompacting the scarified soils to at least 95 percent of standard

Proctor maximum dry density (ASTM D 698) and within the range of -1 to +3 percentage points of the material's optimum moisture content. The pavement subgrade should be proof-rolled as described in Section 7.1 in this report. Recommendations for subgrade preparation (lime treated subgrade or recompacted subgrade) are presented in Section 6.2.4.

It is recommended that subgrade preparation (lime treated subgrade or recompacted subgrade) extend at least 1 ft beyond the edge of the pavement to reduce effects of seasonal shrinking and swelling upon the extreme edges of pavement. Also, the curb should be constructed such that the base of the curb extends at least 6 inches into the pavement subgrade.

Pavement will have the same potential for movement as discussed in Section 6.1.2 (up to about 2 inches). Good perimeter surface drainage with a minimum slope of 2 percent away from the pavement is recommended. The use of sand as a leveling course below pavement supported on expansive clays should be avoided. Normal maintenance of pavement should be expected over the life of the pavement structures.

6.2.2 Portland Cement Concrete Pavements

Subgrade preparation as described in Section 6.2.1 is required for PCC pavement. The minimum recommended PCC pavement sections to be constructed are provided in Table A:

TABLE A
FOLLOWING SUBGRADE IMPROVEMENT PROVIDED IN SECTION 6.2.1
PORTLAND CEMENT CONCRETE PAVEMENT SECTIONS

	General Parking 30,000 ESAL (inches)		Access Drives 100,000 ESAL (inches)	
Reinforced PCC	5.0	5.5	6.0	6.5
Lime Treated Subgrade	6.0	----	6.0	----
Recompacted Subgrade	----	6.0	----	6.0

A minimum of 7 inches of PCC is recommended for dumpster pads. PCC should have a minimum compressive strength of 4,000 psi at 28 days. Concrete should be designed with 5 ± 1 percent entrained air.

Pavement Joints and Reinforcement – The following is recommended for all concrete pavement sections in this report. *Note: Refer to ACI 330 for additional information on pavement joints and reinforcement.*

Contraction Joints:

Spacing:	12½ ft each way for pavement thickness of 5 or 5½ inches; 15 ft each way for pavement thickness of 6 or 6½ inches.
Depth:	At least one-fourth (¼) of pavement thickness.
Width:	One-fourth (¼) inch or as required by joint sealant manufacturer.

Construction Joints:

Spacing:	Install at location of contraction joints.
Width/Depth:	Full depth of pavement thickness. Construct sealant reservoir along one edge of the joint. Width of reservoir to be one-fourth (¼) inch or as required by joint sealant manufacturer. Depth of reservoir to be at least one-fourth (¼) of pavement thickness.

Isolation Joints:

Spacing:	As required to isolate pavement from structures, etc.
Depth:	Full depth of pavement thickness.
Width:	One-half (½) to one (1) inch or as required by the joint sealant manufacturer.

Expansion Joints: None (see note below)

Note: In this locale, drying shrinkage of concrete typically significantly exceeds anticipated expansion due to thermal affects. As a result, the need for expansion joints is eliminated provided all joints (including saw cuts) are sealed. Construction of an unnecessary joint may also become a maintenance problem. All joints should be sealed. If all joints, including saw cuts, are not sealed then expansion joints should be installed.

Distributed Steel: Steel reinforcement should consist of No 4 reinforcing steel bars at 18 inches on-center-each-way, Grade 60. *Note: It is imperative that the distributed steel be positioned accurately in the pavement cross section.*

All construction joints have dowels. Dowel information varies with pavement thickness as presented as follows.

Pavement Thickness:	5, 5½ inches	6, 6½ inches	7, 7½ inches
Dowels:	⅝ inch diameter	¾ inch diameter	7/8-inch diameter
Dowel Spacing:	12 inches on center	12 inches on center	12 inches on center
Dowel Length:	12 inches long	14 inches long	14 inches long
Dowel Embedment:	5 inches	6 inches	6 inches

6.2.3 Asphalt Concrete Pavements

Subgrade preparation as described in Section 6.2.1 is required for asphalt concrete pavement. The minimum recommended asphalt concrete pavement sections to be constructed are provided in Table B. Pavement materials are described in Section 6.2.4.

TABLE B
FOLLOWING SUBGRADE IMPROVEMENT PROVIDED IN SECTION 6.2.1
ASPHALT CONCRETE PAVEMENT SECTIONS

	General Parking 18,000 ESAL (inches)		Access Drives 75,000 ESAL (inches)	
HMAC Surface Course – Type D	2.0	2.0	2.5	2.5
Flexible Base	12.0	8.0	14.0	10.0
Lime Treated Subgrade	----	6.0 [1]	----	6.0 [1]
Recompacted Subgrade	6.0	----	6.0	----

[1] *Geogrid may be used in lieu of the 6-inch lime treated subgrade. Criteria for geogrid can be found in Section 6.2.4.*

6.2.4 Pavement Materials

Presented below are various materials that may be used to construct the pavement sections at this site. Submittals should be made for each pavement material. The submittals should be reviewed by the Geotechnical Engineer and appropriate members of the design team and should provide test information necessary to verify full compliance with the recommended or specified material properties.

Hot Mix Asphaltic Concrete (HMAC) Courses - The HMAC surface course should be plant mixed, hot laid Type D (TxDOT Standard Specifications Item 341). Each mix should meet the master specifications requirements of 2014 TxDOT Standard Specifications Item 341, Item SS 3224 (2011) and specific criteria for the job mix formula.

Flexible Base – Flexible base should meet TxDOT Standard Specification Item 247 Grade 1-2, Type A. Flexible base should be compacted to a minimum of 95 percent of the materials maximum standard Proctor dry density (ASTM D 698) at a moisture content of -2 to +2 percentage points of optimum moisture.

Lime Treated Subgrade – Due to the presence of clayey soils (with a PI over 20) at this site, the pavement subgrade may be treated with hydrated lime. The subgrade should be scarified to a depth of 6 inches and mixed with a minimum 7 percent hydrated lime (by dry soil weight) in conformance with TxDOT Standard Specification Item 260. Assuming an in-place unit weight of 100 pcf for the pavement subgrade soils, this percentage of lime equates to about 32 lbs of lime per square yard of treated

subgrade. The actual amount of lime required should be confirmed by additional laboratory tests (ASTM C 977 Appendix XI) prior to construction. The soil-lime mixture should be compacted to at least 95 percent of standard Proctor maximum dry density (ASTM D 698) and within the range of 0 to 4 percentage points above the mixture's optimum moisture content. In all areas where hydrated lime is used to stabilize subgrade soil, routine Atterberg-limit tests should be performed to verify the resulting plasticity index of the soil-lime mixture is at/or below 20 percent. Subgrade preparation utilizing lime stabilization as described herein will not prevent normal seasonal movement of the underlying untreated materials.

Geogrid – Geogrid can be used in lieu of the 6 inches of lime treated subgrade for asphalt pavements. The geogrid should consist of Tensar TX130S, Tensar Biaxial Type 1 or TxDOT Type 2. The geogrid should be placed at the bottom of the flexible base material layer. However, the pavement subgrade should still be moisture conditioned and compacted to a depth of about 6 inches.

Recompacted Subgrade – The subgrade should be scarified to a depth of 6 inches and compacted to a dry density of at least 95 percent of standard Proctor maximum dry density (ASTM D 698) and within the range of 0 to +4 percentage points of optimum moisture content.

6.3 Drainage

Adequate drainage should be provided to reduce seasonal variations in the moisture content of foundation soils. All pavement and sidewalks within 10 ft of buildings should be sloped away from buildings to prevent ponding of water around buildings. Final grades within 10 ft of existing structures should be adjusted to slope away from those structures at a minimum slope of 2 percent. **Maintaining positive surface drainage throughout the life of the structure is essential.**

In areas with pavement or sidewalks adjacent to the structures, a positive seal must be maintained between the structure and the pavement or sidewalk to minimize seepage of water into the underlying supporting soils. Post-construction movement of pavement and flatwork is common. Normal maintenance should include examination of all joints in paving and sidewalks, etc. as well as resealing where necessary.

Several factors relate to civil and architectural design and/or maintenance, which can significantly affect future movements of the flatwork and pavement systems:

- Large trees and shrubs should not be allowed closer to the pavements or flatwork than a horizontal distance equal to roughly their mature canopy due to their significant moisture demand upon maturing. *Note: A landscape expert may be consulted to evaluate the precise extents of potential root growth for specific tree and shrub species so that root growth beneath the flatwork and pavements can be avoided.*

- Moisture conditions should be maintained "constant" around the edge of the pavements and sidewalks. Ponding of water in planters, in unpaved areas, and around joints in paving and sidewalks can cause soil induced movements beyond those predicted in this report.

- Planter box structures placed adjacent to the pavements and sidewalks should be provided with a means to assure concentrations of water are not available to the subsoil stratigraphy.

- The root systems from any existing trees cleared/removed at this site will have dried and desiccated the surrounding clay soils, resulting in soil with near-maximum swell potential. Clay soils surrounding tree root mats within the pavement areas or flatwork areas should be removed to a depth of 3 ft and compacted in-place with moisture and density control as described in Section 7.3.

Trench backfill for utilities should be properly placed and compacted as outlined in Section 7.3 and in accordance with requirements of local City standards. Since granular bedding backfill is used for most utility lines, the backfilled trench should not become a conduit and allow access for surface or subsurface water to travel toward the structures. Concrete cut-off collars or clay plugs should be provided where utility lines cross building lines to prevent water from traveling in the trench backfill and entering beneath the structures.

7.0 GENERAL CONSTRUCTION PROCEDURES AND RECOMMENDATIONS

Variations in subsurface conditions could be encountered during construction. To permit correlation between boring data and actual subsurface conditions encountered during construction, it is recommended UES be retained to observe construction procedures and materials.

Some construction problems, particularly degree or magnitude, cannot be anticipated until the course of construction. The recommendations offered in the following paragraphs are intended not to limit or preclude other conceivable solutions, but rather to provide our observations based on our experience and understanding of the project characteristics and subsurface conditions encountered in the borings.

7.1 Site Preparation and Grading

All areas supporting pavement, flatwork, or areas to receive new fill should be properly prepared.

- After completion of the necessary stripping, clearing, and excavating and prior to placing any required fill, the exposed soil subgrade should be carefully evaluated by probing and testing. Any undesirable material (organic material, wet, soft, or loose soil) still in place should be removed.

- The exposed soil subgrade should be further evaluated by proof-rolling with a heavy pneumatic tired roller, loaded dump truck or similar equipment weighing approximately 15 tons to check for pockets of soft or loose material hidden beneath a thin crust of possibly better soil.

- Proof-rolling procedures should be observed routinely by a UES Professional Engineer, or his designated representative.

- Any undesirable material (organic material, wet, soft, or loose soil) exposed during the proofroll should be removed and replaced with well-compacted material as outlined in Section 7.3.

- Prior to placement of any fill, the exposed soil subgrade should then be scarified to a minimum depth of 6 inches and recompacted as outlined in Section 7.3.

Slope stability analysis of embankments (natural or constructed) was not within the scope of this study. If fill is to be placed on existing slopes (natural or constructed) steeper than six (6) horizontal to one (1) vertical (6:1), the fill materials should be benched into the existing slopes in such a manner as to provide a minimum bench width of five (5) ft. This should provide a good contact between the existing soils and new fill materials, reduce potential sliding planes and allow relatively horizontal lift placements.

The contractor is responsible for designing any excavation slopes, temporary sheeting or shoring. Design of these structures should include any imposed surface surcharges. Construction site safety is the sole responsibility of the contractor, who shall also be solely responsible for the means, methods and sequencing of construction operations. The contractor should also be aware that slope height, slope inclination or excavation depths (including utility trench excavations) should in no case exceed those specified in local, state and/or federal safety regulations, such as OSHA Health and Safety Standard for Excavations, 29 CFR Part 1926, or successor regulations. Stockpiles should be placed well away from the edge of the excavation and their heights should be controlled so they do not surcharge the sides of the excavation. Surface drainage should be carefully controlled to prevent flow of water over the slopes and/or into the excavations. Construction slopes should be closely observed for signs of mass movement, including tension cracks near the crest or bulging at the toe. If potential stability problems are observed, a geotechnical engineer should be contacted immediately. Shoring, bracing or underpinning required for the project (if any) should be designed by a professional engineer registered in the State of Texas.

Due to the nature of the clayey soils found near the surface at the borings, traffic of heavy equipment (including heavy compaction equipment) may create pumping and general deterioration of shallow soils. Therefore, some construction difficulties should be anticipated during periods when these soils are saturated.

7.2 Excavations

All excavations should be monitored to verify bearing stratum consists of suitable material. The bearing stratum exposed in the base of all excavations should be protected against any detrimental change in conditions. Surface runoff water should be drained away from excavations and not allowed to collect. All concrete should be placed as soon as practical after the excavation is made. Prolonged exposure of the bearing surface to air or water will result in changes in strength and compressibility of the bearing stratum. All excavations should not be left open for more than 48 hours.

Groundwater was not encountered during drilling at this site. However, from our experience, seasonal groundwater seepage could be encountered at the site during excavation, and the risk of encountering seepage is increased during or after periods of precipitation. UES should be contacted for further review and evaluation if groundwater seepage is encountered during excavation.

7.3 Fill Compaction

The following fill compaction recommendations provided below are applicable for general site grading. *Note: Imported soils used as general fill should consist of material with a PI not greater than 30 percent.*

General Fill (Clay) – Clay soils should be compacted to a dry density between 95 and 100 percent of Standard Proctor maximum dry density (ASTM D 698). The compacted moisture content of the clays during placement should be within the range of 0 to 4 percentage points above optimum. Clayey materials used as fill should be processed and the largest particle or clod should be less than 6 inches prior to compaction.

General Fill (Granular) – Granular materials should be compacted to a dry density between 95 and 100 percent of Standard Proctor maximum dry density (ASTM D 698). The compacted moisture content of the granular soils during placement should be within the range of -2 to +2 percentage points of optimum.

Prior to placement of any fill or foundation, the subgrade should be scarified to a depth of 6 inches and recompacted to a dry density of at least 95 percent of standard Proctor maximum dry density (ASTM D 698) and within the range of +1 to +4 percentage points of the material's optimum moisture content.

In cases where mass fills outside the structure areas are more than 12 ft deep, the fill below 12 ft should be compacted to at least 100 percent of standard Proctor maximum dry density (ASTM D-698) and within 2 percentage points of the material's optimum moisture content. The portion of the fill shallower than 12 ft should be compacted as outlined above. *Note: Even if fill is properly*

compacted, fills in excess of about 12 ft are still subject to settlements over time of up to about 1 to 2 percent of the total fill thickness. This should be considered when designing structures and pavements on relatively deep backfill. UES should be contacted if alterative backfill recommendations are required to reduce settlement of mass fills.

Compaction should be accomplished by placing fill in about 8-inch thick loose lifts and compacting each lift to at least the specified minimum dry density. Field density and moisture content tests should be performed on each lift. A qualified geotechnical engineering firm should be retained to perform sufficient in-place density tests during the filling operations to evaluate that proper levels of compaction, including dry unit weight and moisture content, are being attained. Controlled, compacted fill should consist of approved materials that are free of organic matter and debris or materials exceeding 4 inches in maximum dimension. *Note: We recommend any imported fill to be used at this site be approved by UES prior to placement.*

7.4 Utilities

Where utility lines are deeper than 12 ft, the fill/backfill below 12 ft should be compacted to at least 100 percent of standard Proctor maximum dry density (ASTM D 698) and within 2 percentage points of the material's optimum moisture content. The portion of the fill/backfill shallower than 12 ft should be compacted as previously outlined. Density tests should be performed on each lift (maximum 12-inch thick) and should be performed as the trench is being backfilled. *Even if fill is properly compacted, fills in excess of about 12 ft are still subject to settlements over time of up to about 1 to 2 percent of the total fill thickness. This should be considered when designing pavement over utility lines.*

If utility trenches or other excavations extend to or beyond a depth of 5 ft below construction grade, the contractor or others shall be required to develop an excavation safety plan to protect personnel entering the excavation or excavation vicinity. The collection of specific geotechnical data and the development of such a plan, which could include designs for sloping and benching or various types of temporary shoring, is beyond the scope of this study. Any such designs and safety plans shall be developed in accordance with current OSHA guidelines and other applicable industry standards.

7.5 Groundwater

Groundwater was not encountered during drilling at this site. However, from our experience with similar soils, seasonal groundwater seepage could be encountered in excavations for utility conduits and other general excavations. The risk of encountering seepage increases with depth of excavation and during or after periods of precipitation. Standard sump pits and pumping may be adequate to control minor seepage on a local basis in relatively shallow excavations.

In any areas where cuts are made to establish final grades at the site, attention should be given to possible seasonal water seepage that could occur through natural cracks and fissures in the newly exposed stratigraphy. Subsurface drains may be required to intercept seasonal groundwater seepage. The need for these or other de-watering devices should be carefully addressed during construction. Our office could be contacted to visually observe the final grades to evaluate the need for such drains.

8.0 LIMITATIONS

Professional services provided in this geotechnical exploration were performed, findings obtained, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. The scope of services provided herein does not include an environmental assessment of the site or investigation for the presence or absence of hazardous materials in the soil, surface water or groundwater. UES, upon written request, can be retained to provide these services.

UES is not responsible for conclusions, opinions or recommendations made by others based on this data. Information contained in this report is intended for the exclusive use of the Client (and their designated design representatives), and is related solely to design of the specific structures outlined in Section 2.0. No party other than the Client (and their designated design representatives) shall use or rely upon this report in any manner whatsoever unless such party shall have obtained UES's written acceptance of such intended use. Any such third party using this report after obtaining UES's written acceptance shall be bound by the limitations and limitations of liability contained herein, including UES's liability being limited to the fee paid to it for this report. Recommendations presented in this report should not be used for design of any other structures except those specifically described in this report. In all areas of this report in which UES may provide additional services if requested to do so in writing, it is presumed that such requests have not been made if not evidenced by a written document accepted by UES. Further, subsurface conditions can change with passage of time. Recommendations contained herein are not considered applicable for an extended period of time after the completion date of this report. It is recommended our office be contacted for a review of the contents of this report for construction commencing more than one (1) year after completion of this report. Non-compliance with any of these requirements by the Client or anyone else shall release UES from any liability resulting from the use of, or reliance upon, this report.

Recommendations provided in this report are based on our understanding of information provided by the Client about characteristics of the project. If the Client notes any deviation from the facts about project characteristics, our office should be contacted immediately since this may materially alter the recommendations. Further, UES is not responsible for damages resulting from workmanship of designers or contractors. It is recommended the Owner retain qualified personnel, such as a Geotechnical Engineering firm, to verify construction is performed in accordance with plans and specifications.

APPENDIX

A-1 METHODS OF FIELD EXPLORATION

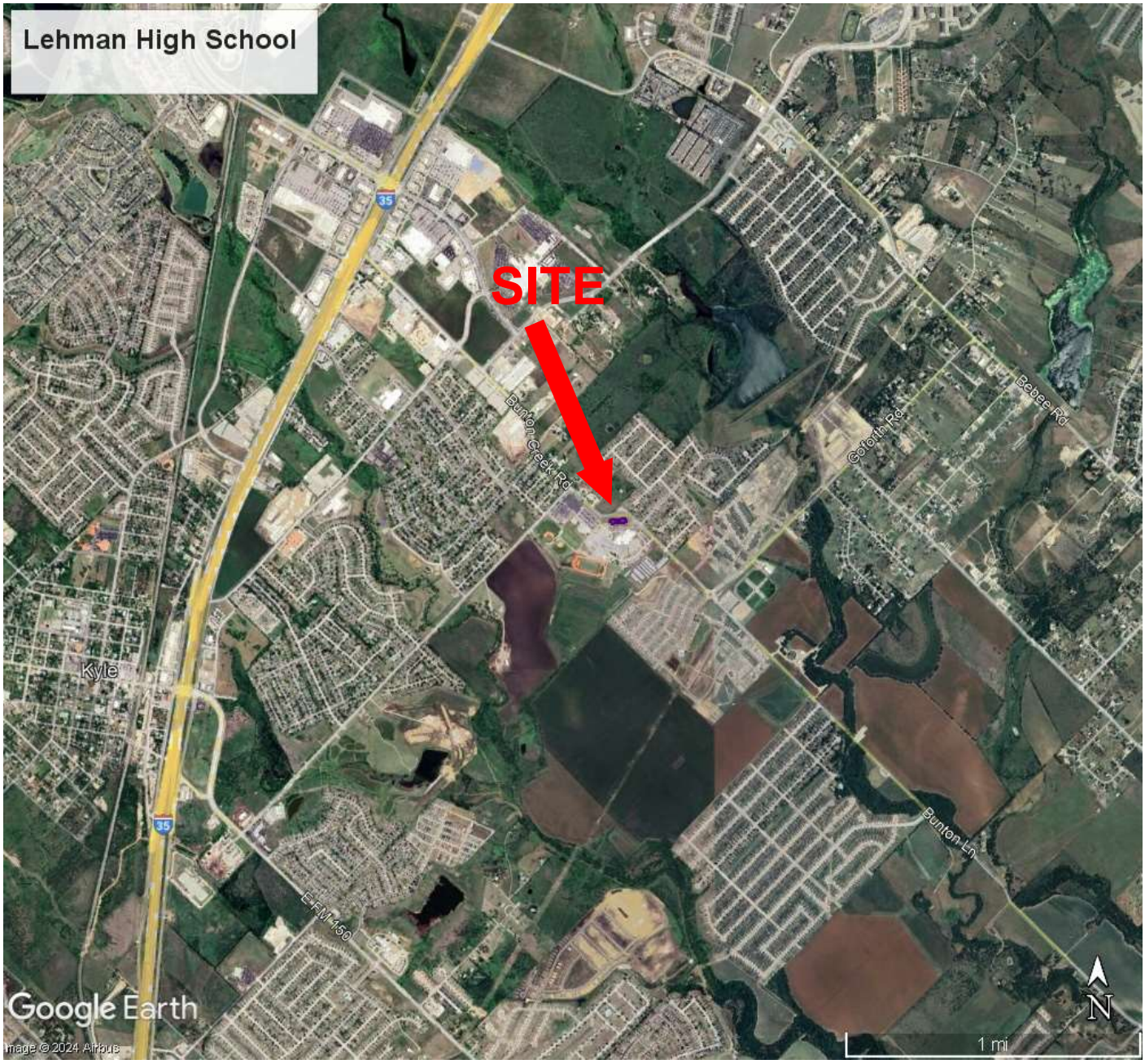
A truck-mounted, rotary drill rig equipped with continuous flight augers were used to advance the boreholes. A total of two (2) borings were performed for this geotechnical exploration at the approximate locations shown on the Boring Location Plans, Figure 1B and 1C. The boring locations were staked by either pacing or taping and estimating right angles from landmarks which could be identified in the field and as shown on the site plan provided during this study. The locations of the borings shown on the Boring Location Plan are considered accurate only to the degree implied by the methods used to define them. The approximate latitude and longitude coordinates at each boring location were obtained using a handheld GPS device.

Samples of granular and cohesive materials were obtained using split-spoon sampling procedures in general accordance with ASTM Standard D 1586. Disturbed samples were obtained at selected depths in the borings by driving a standard 2-inch O.D. split-spoon sampler 18 inches into the subsurface material using a 140-pound hammer falling 30 inches. The number of blows required to drive the split-spoon sampler the final 12 inches of penetration (N-value) is recorded in the appropriate column on the boring logs. However, if the sampler was not driven the initial 6-inch seating increment with 50 hammer blows, refusal (i.e. “ref”) is recorded along with the inches driven on the logs.

Our field representative prepared field logs as part of the field exploration. The field logs included visual descriptions of the materials encountered during drilling and their interpretation of the subsurface conditions between samples. The Log of Boring sheets included in this report represent the engineer’s interpretation of the field logs and include modifications based on visual observations using the Unified Soil Classification System (USCS) and testing of the samples in the laboratory. **Samples not consumed by testing will be retained in our laboratory for at least 30 days and then discarded unless the Client requests otherwise.**

SITE VICINITY MAP

LOCATION IS APPROXIMATE
FIGURE 1A



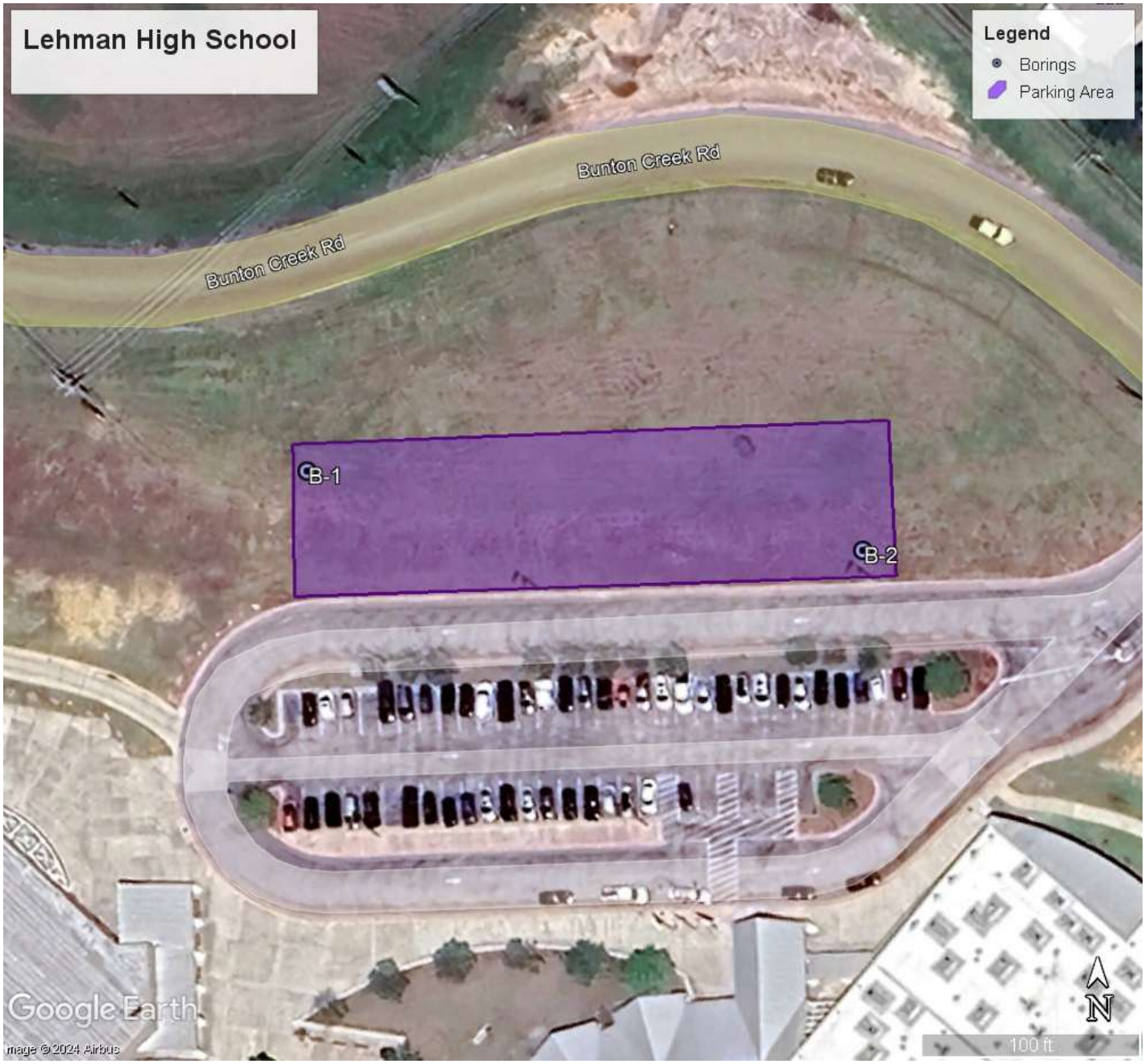
LEHMAN HS – ADDITIONAL PARKING
1700 Lehman Road
Kyle, Texas

Hays CISD
UES Project No.: 24-0925

BORING LOCATION PLAN

LOCATIONS ARE APPROXIMATE

FIGURE 1B



LEHMAN HS – ADDITIONAL PARKING
1700 Lehman Road
Kyle, Texas

Hays CISD
UES Project No.: 24-0925

B-1 METHODS OF LABORATORY TESTING

Representative samples were inspected and classified by a qualified member of the Geotechnical Division and the boring logs were edited as necessary. To aid in classifying the subsurface materials and to determine the general engineering characteristics, natural moisture content tests (ASTM D 2216), Atterberg-limit tests (ASTM D 4318), and percent passing No. 200 Sieve (ASTM D 1140) were performed on selected samples. Results of these laboratory tests are provided on the Logs of Boring sheets.

LOG OF BORING 01

UES Professional Solutions 44, LLC
10856 Vandale Street
San Antonio, Texas 78216
Telephone: 210-495-8000
Fax: 210-495-8015

CLIENT: Hays CISD
PROJECT: Lehman High School - Additional Parking
LOCATION: 1700 Lehman Road, Kyle, Texas
NUMBER: 24-0925
DATE(S) DRILLED: 08/13/2024 - 08/13/2024

DRILLING METHOD(S):
Air Rotary

GROUNDWATER INFORMATION:
Groundwater was not encountered during drilling, and the boring was dry upon completion of drilling.

SURFACE ELEVATION: N/A

SOIL SYMBOL	DEPTH (FT)	SAMPLE NUMBER	SAMPLES	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT Qc: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PL	ATTERBERG LIMITS PI	DRY DENSITY POUNDS/CU.FT	COMPRESSIVE STRENGTH (TONS/SQ FT)	MINUS NO. 200 SIEVE (%)	DESCRIPTION OF STRATUM
												FIELD DATA / LABORATORY DATA
	1	SPT		N=28	5	43	19	24			75	**FILL:LEAN CLAY WITH SAND**, soft to very stiff, dark brown and light brown. (CL)
	2											
	3	SPT		N=4	9							
	4											
	5	SPT		N=12	3	NP	NP	NP			11	**FILL: POORLY GRADED SILTY SAND**, medium dense, reddish brown. (SP-SM)
	6											
	7	SPT		N=49	13							**LEAN CLAY WITH SAND**, hard, light brown with gray. (CL)
	8											
	9	SPT		N=50	12	46	18	28			81	
	10											
	11											
	12											
	13											
	14	SPT		N=42	13							
	15											Boring terminated at depth of 15 feet.

N - STANDARD PENETRATION TEST RESISTANCE
Qc - STATIC CONE PENETROMETER TEST INDEX
P - POCKET PENETROMETER RESISTANCE

REMARKS:
Boring location determined by UES. Drilling operations performed by UES.
GPS Coordinates: N 29.996503°, W -97.848233°

LOG OF BORING 02

UES Professional Solutions 44, LLC
10856 Vandale Street
San Antonio, Texas 78216
Telephone: 210-495-8000
Fax: 210-495-8015

CLIENT: Hays CISD
PROJECT: Lehman High School - Additional Parking
LOCATION: 1700 Lehman Road, Kyle, Texas
NUMBER: 24-0925
DATE(S) DRILLED: 08/13/2024 - 08/13/2024

DRILLING METHOD(S):
Air Rotary

GROUNDWATER INFORMATION:
Groundwater was not encountered during drilling, and the boring was dry upon completion of drilling.

SURFACE ELEVATION: N/A

SOIL SYMBOL	DEPTH (FT)	SAMPLE NUMBER	SAMPLES	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT Qc: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS LIQUID LIMIT LL	ATTERBERG LIMITS PLASTIC LIMIT PL	ATTERBERG LIMITS PLASTICITY INDEX PI	DRY DENSITY POUNDS/CU.FT	COMPRESSIVE STRENGTH (TONS/SQ FT)	MINUS NO. 200 SIEVE (%)	DESCRIPTION OF STRATUM
												FIELD DATA / LABORATORY DATA
	1	SPT		N=9	13							**FILL: LEAN CLAY WITH SAND**, stiff to very stiff, dark brown and light brown and gray. (CL)
	2											
	3	SPT		N=24	8	40	17	23			78	
	4											
	5	SPT		N=32	12							**LEAN CLAY WITH SAND**, hard, light brown with reddish brown. (CL)
	6											
	7	SPT		N=66	2	39	17	22			77	
	8											
	9	SPT		N=77	12							
	10											
	11											
	12											
	13											
	14	SPT		N=66	12	41	15	26			72	
	15											Boring terminated at depth of 15 feet.

N - STANDARD PENETRATION TEST RESISTANCE
Qc - STATIC CONE PENETROMETER TEST INDEX
P - POCKET PENETROMETER RESISTANCE

REMARKS:
Boring location determined by UES. Drilling operations performed by UES.
GPS Coordinates: N 29.996404°, W -97.847434°

LOG_OF_BORING 24-0925.GPJ ROCK_ETL.GDT 9/3/24

UES™

KEY TO SOIL CLASSIFICATION AND SYMBOLS

UNIFIED SOIL CLASSIFICATION SYSTEM

MAJOR DIVISIONS		SYMBOL	NAME
COARSE GRAINED SOILS	GRAVEL AND GRAVELLY SOILS	GW	Well Graded Gravels or Gravel-Sand mixtures, little or no fines
		GP	Poorly Graded Gravels or Gravel-Sand mixtures, little or no fines
		GM	Silty Gravels, Gravel-Sand-Silt mixtures
		GC	Clayey Gravels, Gravel-Sand-Clay Mixtures
	SAND AND SANDY SOILS	SW	Well Graded Sands or Gravelly Sands, little or no fines
		SP	Poorly Graded Sands or Gravelly Sands, little or no fines
		SM	Silty Sands, Sand-Silt Mixtures
		SC	Clayey Sands, Sand-Clay mixtures
	SILTS AND CLAYS LL < 50	ML	Inorganic Silts and very fine Sands, Rock Flour, Silty or Clayey fine Sands or Clayey Silts
		CL	Inorganic Clays of low to medium plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays
		OL	Organic Silts and Organic Silt-Clays of low plasticity
	SILTS AND CLAYS LL > 50	MH	Inorganic Silts, Micaceous or Diatomaceous fine Sandy or Silty soils, Elastic Silts
		CH	Inorganic Clays of high plasticity, Fat Clays
		OH	Organic Clays of medium to high plasticity, Organic Silts
NON USCS MATERIALS			Limestone
			Marl/Claystone
			Sandstone

TERMS CHARACTERIZING SOIL STRUCTURE

SLICKENSIDED - having inclined planes of weakness that are slick and glossy in appearance

FISSURED - containing shrinkage cracks, frequently filled with fine sand or silt; usually more or less vertical

LAMINATED (VARVED) - composed of thin layers of varying color and texture, usually grading from sand or silt at the bottom to clay at the top

CRUMBLY - cohesive soils which break into small blocks or crumbs on drying

CALCAREOUS - containing appreciable quantities of calcium carbonate, generally nodular

WELL GRADED - having wide range in grain sizes and substantial amounts of all intermediate particle sizes

POORLY GRADED - predominantly of one grain size uniformly graded) or having a range of sizes with some intermediate size missing (gap or skip graded)

SYMBOLS FOR TEST DATA

- Groundwater Level (Initial Reading)
- Groundwater Level (Final Reading)
- Shelby Tube Sample
- SPT Samples
- Auger Sample
- Rock Core
- Texas Cone Penetrometer
- Grab Sample

TERMS DESCRIBING CONSISTENCY OF SOIL

COARSE GRAINED SOILS		FINE GRAINED SOILS		
DESCRIPTIVE TERM	NO. BLOWS/FT. STANDARD PEN. TEST	DESCRIPTIVE TERM	NO. BLOWS/FT. STANDARD PEN. TEST	UNCONFINED COMPRESSION TONS PER SQ. FT.
Very Loose	0 - 4	Very Soft	< 2	< 0.25
Loose	4 - 10	Soft	2 - 4	0.25 - 0.50
Medium Dense	10 - 30	Firm	4 - 8	0.50 - 1.00
Dense	30 - 50	Stiff	8 - 15	1.00 - 2.00
Very Dense	over 50	Very Stiff	15 - 30	2.00 - 4.00
		Hard	over 30	over 4.00

Field Classification for "Consistency" of Fine Grained Soils is determined with a 0.25" diameter penetrometer

GEOTECHNICAL EXPLORATION

LEHMAN HIGH SCHOOL
1700 Lehman Road
Kyle, Texas 78640
UES Report No. A251017
April 10, 2025

Prepared for:

HAYS CONSOLIDATED INDEPENDENT SCHOOL DISTRICT
21003 Interstate 35 Frontage Road
Kyle, Texas 78640

Attention: Nathan Wensowitch

Prepared By:

UES™

April 10, 2025

Hays Consolidated Independent School District
21003 Interstate 35 Frontage Road
Kyle, Texas 78640

Attention: Nathan Wensowitch

Re: Geotechnical Exploration
Lehman High School
1700 Lehman Road
Kyle, Texas 78640
UES Report No. A251017

Attached is the report of the geotechnical exploration performed for the project referenced above. This study was authorized by Nathan Wensowitch with Hays Consolidated Independent School District on February 5, 2025 and performed in accordance with UES Proposal No. P25-0173 dated February 5, 2025 and Hays CISD Purchase Order #25003600.

This report contains results of field explorations, laboratory testing and an engineering interpretation of these with respect to available project characteristics. The results and analyses were used to develop recommendations to aid design and construction of foundations and pavements.

UES Professional Solutions 44, LLC (UES) appreciates the opportunity to be of service on this project. If we can be of further assistance, such as providing materials testing services during construction, please contact our office.

Sincerely,

UES PROFESSIONAL SOLUTIONS 44, LLC

Sebastian L. Aleman, E.I.T.
Geotechnical Project Manager

Lee E. Gurecky, PE
Geotechnical Department Manager

SLA/LEG
Copies: (1-PDF) Nathan Wensowitch; Hays CISD

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UES REPORT NO. A251017

APPENDIX

1.0 PURPOSE AND SCOPE

The purpose of this geotechnical exploration is for UES PROFESSIONAL SOLUTIONS 44, LLC (UES) to evaluate for HAYS CONSOLIDATED INDEPENDENT SCHOOL DISTRICT (Client) some of the physical and engineering properties of subsurface materials at selected locations on the subject site with respect to formulation of appropriate geotechnical design parameters for the proposed construction. The field exploration was accomplished by securing subsurface samples from widely spaced test borings performed across the expanse of the site. Engineering analyses were performed from results of the field exploration and results of laboratory tests performed on representative samples.

Also included are general comments pertaining to reasonably anticipated construction problems and recommendations concerning earthwork and quality control testing during construction. This information can be used to evaluate subsurface conditions and to aid in ascertaining construction meets project specifications.

Recommendations provided in this report were developed from information obtained in borings depicting subsurface conditions only at the specific boring locations and at the particular time designated on the logs. Subsurface conditions at other locations may differ from those observed at the boring locations, and subsurface conditions at boring locations may vary at different times of the year. The scope of work may not fully define the variability of subsurface materials and conditions that are present on the site.

The nature and extent of variations between borings may not become evident until construction. If significant variations then appear evident, determined by experienced on-site personnel, our office should be contacted to re-evaluate our recommendations after performing additional onsite observations and possibly other tests.

2.0 PROJECT CHARACTERISTICS

It is proposed to design and construct a new weight room, multipurpose academic building, and associated pavements to be located at the existing Lehman High School in Kyle, Texas. A Site Vicinity Plan and Boring Location Plans illustrating the general outline of the property are provided in the Appendix of this report. A site grading plan, including initial and final contours, was not available at the time of this study. For the purpose of this study, we have assumed the final site grades will be within 2-feet of existing grades.

We understand that a deep foundation system will be considered to support the proposed structures at this site. The client has indicated that design PVR/PVM of ¾ and ½-inch are desired for this project. We anticipate the maximum column loads will not exceed 300-kips. New pavements consisting of both asphalt and concrete will be considered at this site.

3.0 FIELD EXPLORATION

Subsurface conditions on the site were explored by drilling a total of five (5) test borings in general accordance with ASTM D 420 using standard air-rotary drilling equipment to advance the borings to their termination depth. The corresponding location of each boring is provided in Table A.

TABLE A		
Locations	**Boring No.**	**Boring Depth, *ft***
Multipurpose Academic Building	B-1 and B-2	35
Future Weight Room	B-3 to B-5	35

The approximate location of each boring is shown on the Boring Location Plan enclosed in the Appendix. Details of drilling and sampling operations are briefly summarized in Methods of Field Exploration, Section A-1 of the Appendix.

Subsurface types encountered during the field exploration are presented on the Log of Boring sheets (boring logs) included in the Appendix. The boring logs contain our Field Technician's and Engineer's interpretation of conditions believed to exist between actual samples retrieved. Therefore, these boring logs contain both factual and interpretive information. Lines delineating subsurface strata on the boring logs are approximate and the actual transition between strata may be gradual.

4.0 LABORATORY TESTS

Selected samples of the subsurface materials were tested in the laboratory to evaluate their engineering properties as a basis in providing recommendations for foundation design and earthwork construction. A brief description of testing procedures used in the laboratory can be found in Methods of Laboratory Testing, Section B-1 of the Appendix. Individual test results are presented on boring logs or summary data sheets enclosed in the Appendix.

5.0 GENERAL SUBSURFACE CONDITIONS

The Geologic Map of Texas, published by the University of Texas at Austin, Bureau of Economic Geology, has mapped the Pecan Gap Chalk (Kpg) formation in the general area of the project site. The Pecan Gap Chalk formation generally consists of clay, chalk, chalky marl, and limestone.

Within the 35-foot maximum depth explored on the site, subsurface materials consist generally of upper strata of low to very high plasticity FILL: SANDY FAT CLAY (CH), FILL: SANDY LEAN CLAY (CL), FILL: SILTY, CLAYEY GRAVEL with SAND (GC-GM), FAT CLAY with SAND (CH), LEAN CLAY (CL), LEAN CLAY with SAND (CL), SANDY LEAN CLAY (CL), and CLAYEY GRAVEL with SAND (GC), generally overlying FAT CLAY (CH). Fill material was encountered at depths from about 1 to 4 feet below existing grades in borings B-1 and B-3 to B-5. Boring B-4 contained 2 inches of asphalt pavement

at the existing ground surface. The letters in parenthesis represent the soils' classification according to the Unified Soil Classification System (ASTM D 2488). More detailed stratigraphic information is presented on the boring logs attached to this report.

The clayey materials encountered are considered relatively impermeable and are anticipated to have a relatively slow response to water movement. The granular materials are considered relatively permeable and are anticipated to have a relatively fast response to water movement. Therefore, several days of observation would be required to evaluate actual groundwater levels within the depths explored. Also, the groundwater level at the site is anticipated to fluctuate seasonally depending on the amount of rainfall, prevailing weather conditions and subsurface drainage characteristics.

Groundwater was encountered during drilling at depths of 16 and 31-feet below the existing ground surface in borings B-2 and B-4, respectively. Groundwater was remeasured at depths of 17.25 and 32.3-feet. Groundwater was not encountered in the remaining borings during and after drilling at this site. It is common to detect seasonal groundwater from natural fractures within the clayey matrix, and in the granular materials, particularly during or after periods of precipitation. If more detailed groundwater information is required, monitoring wells or piezometers can be installed. Further details concerning subsurface materials and conditions encountered can be obtained from the boring logs provided in the Appendix.

6.0 DESIGN RECOMMENDATIONS

The following design recommendations were developed on the basis of the previously described Project Characteristics (Section 2.0) and General Subsurface Conditions (Section 5.0). If project criteria should change, our office should conduct a review to determine if modifications to the recommendations are required. Further, it is recommended our office be provided with a copy of the final plans and specifications for our review prior to construction.

6.1 General Considerations

The foundation system being considered to provide support for the proposed structures must satisfy two independent engineering criteria. One criterion is the foundation system must be designed with an appropriate factor of safety, or a performance limit state, to reduce the possibility of soil failure when subjected to axial and lateral load conditions. The other criterion is foundation movements, whether vertical, horizontal, or rotational, must be within allowable operational limits of the structure. These criteria can be achieved for the planned structure foundations if they are designed and constructed in accordance with the recommendations contained in this report.

Design criteria given in this report were developed assuming the floor slab of the building is constructed within 2-feet of existing grade. Substantial cutting and filling (more than 2-feet) on the site can alter the recommended foundation design parameters. Therefore, it is recommended UES be contacted before performing other cutting and filling on site to verify the appropriate design parameters are utilized for final foundation design.

6.1.1 Existing Fill

As stated in Section 5.0, existing fill was encountered to a depth of up to 4-feet below the existing ground surface in some of the borings at this site. Such fill may also exist in areas other than those explored to greater or lesser depths. We understand the fill was placed during initial development of the school and was tested by others. Evaluation of the consistency and compaction of the existing fill are considered beyond the scope of this study. The following recommendations are predicated upon our understanding that all of the existing fill encountered on the site meets or exceeds the recommendations contained in Section 7.3 of this report. *Note: It is recommended the Client obtain written confirmation from a Professional Engineer that the existing fill was tested upon placement and has been adequately compacted for support of the turf fields.*

If documentation of the fill is not available, removal and replacement of all fill is the only method of eliminating the risk of unusual settlement associated with the fill. Samples obtained of the uncontrolled fill were generally free of significant voids. In the absence of documented density control, the possibility of under-compacted zones or voids may exist within the uncontrolled fill.

Although not encountered at the borings, uncontrolled fills can contain boulders, rubble, debris, organic materials, and other unsuitable materials. Excavation and grading contracts should contain provision for removal of unsuitable materials. Test pits could be performed prior to construction to assess the depth, lateral extent, and nature of the existing fill. UES would be pleased to assist with a test pit program if desired.

6.1.2 Vertical Movements

Expansive soils are present at this site. This report provides recommendations to reduce the effects of soil shrinkage and expansion. However, even if these recommendations are followed, some movement and cracking in the structures and any flatwork should be anticipated. The severity of cracking and other damage such as uneven floor slabs will probably increase if any modification of the site results in excessive wetting or drying of the expansive soils.

Grade supported structures at this site (including foundations) could experience soil-related potential seasonal movements (i.e. PVM) of about 2 to 6-inches. Note: These PVR values were estimated using on-site or similar soil with a Plasticity Index (PI) of 40 or less to raise grades a maximum of 2-feet.

These potential seasonal movements were also estimated in general accordance with methods outlined by the Texas Department of Transportation (TxDOT) Test Method Tex-124-E, engineering judgment, and experience. The estimated movement was calculated assuming the moisture content of the in-situ soil within the normal zone of seasonal moisture content change varies between a "dry" condition and a "wet" condition as defined by Tex-124-E. Also, it was assumed a 1 psi surcharge load from the floor slab acts on the subgrade soils.

Movements significantly exceeding those predicted above could occur if positive drainage of surface water is not maintained or if soils are subject to an outside water source, such as leakage from a utility line or subsurface moisture migration from off-site locations. However, soil movements may be reduced by implementing the subgrade improvement recommendations presented below, in Section 6.2 of this report.

6.1.3 Foundation Considerations

The proposed structures could be supported using a deep foundation system. The deep foundation system should consist of Drilled Piers or Auger Cast In Place (ACIP) piles with a structurally suspended floor slab. Recommendations for these types of foundation systems are provided in Sections 6.3 through 6.4.

A structural slab suspended completely above the existing highly expansive soils should be used for the buildings supported on piers or piles. At least 12-inches of void space should be provided between the bottom of the floor slab (and lowest suspended fixture) and top surface of the underlying expansive clays. This 12-inch void space should also be maintained between the bottom of any structural element or utility line above the expansive soils. Cardboard carton forms or a deeper crawl space can be used to create the minimum void space.

If a crawl space is constructed, consideration should be given to constructing a mud mat below the crawl space. The benefits of a mud mat include (but are not limited to) resistance to moisture ingress through the suspended floor slab from the precipitation of moisture from the underlying soils and improved access below the structure for maintenance and repairs of utilities. In addition, proper ventilation should be provided to reduce the possibility that a high humidity environment could develop in the void space areas. We recommend that the crawl space subgrade be sloped (not steeper than 3 horizontal to 1 vertical) to appropriate drainage outlets to reduce the possibility of water accumulation in these areas. Flexible connections should be utilized in suspended piping,

especially where it enters the ground. *Note: A structurally suspended slab will be required to achieve design PVM less than 1-inch. Subgrade improvement is not required for structurally suspended slabs.*

6.2 Subgrade Preparations (Flatwork)

As discussed above, flatwork supported within 2-feet of existing grade could experience soil-related potential seasonal movements up to about 6-inches. *Note: Care should be taken when excavating adjacent to existing structures when performing subgrade improvement. In some cases, it will be necessary to shore the existing structure to prevent undermining of existing foundations and slabs.* Potential seasonal movements can be reduced by properly preparing the building pad as recommended in the following sections. *Note: Subgrade improvement is not required if the slabs are structurally suspended above the ground surface on drilled piers or auger cast in place pile .*

6.2.1 Removal and Replacement with Select Fill

Potential seasonal movements can be reduced to about 1-inch by properly preparing the subgrade as recommended below:

Over-excavate all of the existing soils to a depth of 10-feet below the finished grade in the movement sensitive flatwork areas. The flatwork area is defined as the area directly beneath and at least 3-feet (horizontal) beyond the perimeter of the proposed flatwork and appurtenances. Appurtenances are those items attached to the building, typically including, but not limited to, the building sidewalks, porches, ramps, stoops, etc.

The building pad can be completed by placing and compacting select fill or flexible base, to the bottom of the floor slab in the building area. Select fill or flexible base materials should be placed in loose lifts of no more than 8-inches. *Note: Criteria for select fill and flexible base are provided in Section 7.3 of this report. The new exposed slope should be benched during fill placement forming horizontal cuts into the slope at vertical intervals of 1 to 3-feet.*

If not covered with concrete flatwork or pavements, the upper 2-feet of the 5-foot overbuild should consist of a cohesive clay with a Plasticity Index (PI) between 20 to 35 percent. The purpose of the clay cap is to reduce the potential for water to infiltrate the building pad causing the subgrade soils to swell. The material should have at least 70 percent by weight passing the No. 200 Sieve and no more than 15 percent by weight retained in the No. 4 Sieve. The material should be compacted as recommended in Section 7.3 of this report, to reduce the risk of surface water infiltration into the flexible base and below the floor slab.

6.2.2 Moisture Conditioned On-Site with Select Fill Cap

If a grade supported floor slab is desired, potential seasonal movements can be reduced to about 1-inch by properly preparing the subgrade as recommended below:

Over-excavate the existing on-site soils to 12-feet below the finished grade in the movement sensitive flatwork areas. The flatwork area is defined as the area directly beneath and at least 3-feet (horizontal) beyond the perimeter of the proposed flatwork and appurtenances. Appurtenances are those items attached to the building, typically including, but not limited to, the building sidewalks, porches, ramps, stoops, etc.

After over-excavating to 12-feet below the finished grade in the flatwork areas, place and compact moisture conditioned on-site soil to within 2-feet below the bottom of the floor slab in the building area. Moisture conditioning should be performed as discussed in Section 6.2.2.1.

The pad can be completed by placing and compacting select fill to the bottom of the flatwork in the movement sensitive flatwork area. Select fill material should be placed in loose lifts of no more than 8-inches.

The pad preparation should result in at least 2-feet of select fill over at least 10-feet of moisture conditioned on-site soil in the building area. To provide a more uniform support and create a more all-weather working surface, the final 6-inches of the pad could be constructed with flexible base (optional) to provide a working surface. *Note: Criteria for select fill and flexible base material are provided in Section 7.3.*

6.2.2.1 Moisture Conditioned On-Site Soil

Moisture conditioning consists of processing and compacting the specified minimum thickness of on-site soil at a "target" moisture content approximated to range between 4 to 6 percentage points above the material's optimum moisture content as determined by the standard Proctor method (ASTM D 698). **Soils with relatively lower plasticity index values may need to be placed at moisture contents closer to optimum to allow for compaction.** The moisture-conditioned soil should be placed in 8-inch thick loose lifts and compacted to a dry density of 93 to 97 percent of standard Proctor maximum dry density.

Moisture conditioning of the on-site soil should extend at least 5-feet outside the perimeter beam and adjoining flatwork. However, select fill material should not extend beyond the building limits. If flatwork or paving is not planned adjacent to the structures (i.e. above the moisture-conditioned soils), a moisture barrier consisting of a minimum of 10 mil plastic sheeting with a clay cover should be placed above the moisture-conditioned soils that are outside the

building perimeter. The clay cover should consist of a clay with a PI between 15 to 30 percent and at least 65 percent by weight passing the No. 200 Sieve. *Note: The moisture conditioned on-site soil should be maintained in a moist condition prior to placement of the required thickness of select fill, plastic sheeting, flatwork, and/or pavement.*

The resulting estimated potential seasonal movement was calculated assuming the moisture content of the moisture-conditioned soil varies between the "target" moisture content and the "wet" condition while the deeper undisturbed in-situ soil within the normal zone of seasonal moisture content change varies between the "dry" condition and the "wet" condition as defined by methods outlined in TxDOT Test Method Tex-124-E.

Note: It is the intent of the moisture-conditioning process described above to reduce the swell potential of the moisture conditioned soil to 1 percent or less. Additional laboratory tests (i.e., standard Proctors, absorption swell tests, etc.) should be conducted during construction to verify the "target" moisture content for moisture conditioning (estimated to range between 4 to 6 percentage points above the material's optimum moisture content as defined by ASTM D 698) is sufficient to reduce the swell potential of the processed soil to 1 percent or less. In addition, it is recommended samples of the moisture conditioned material be routinely obtained during construction to verify the swell of the improved material is 1 percent or less. One swell test per every 3-feet (thick) should be performed every 5,000 SF of the moisture conditioned soil in the building area.

Installation of moisture-conditioned soils should be monitored and tested on a full-time basis by a representative of UES to verify the soils tested were placed with the proper lift thickness, moisture content, and degree of compaction.

6.2.3 Comments Regarding Excavation Adjacent to Existing Building

Care should be taken when excavating adjacent to existing structures when performing subgrade improvement to prevent undermining foundations and floor slabs. In some cases, it will be necessary to shore the existing structure to prevent undermining of existing foundations and slabs. Considering the relatively shallow depth of subgrade improvement required (about 5-feet or less), another method is to excavate next to the existing building in narrow strips that are perpendicular to the building, and then quickly backfill the excavations with select fill or flexible base before proceeding to the next adjoining strip. The key to this method is to excavate in narrow enough strips so that the excavation can be made and completely backfilled in a very short period of time. The soils under the existing structure, and the existing structure itself, should be carefully monitored during excavation, and the excavation should be immediately backfilled if any indications of movement are detected.

The contractor and design team shall be required to develop an excavation safety plan to protect personnel entering the excavation or excavation vicinity, and to protect existing structures. The collection of specific geotechnical data and the development of such a plan, which could include designs for sloping and benching or various types of temporary shoring, is beyond the scope of this study. Any such designs and safety plans shall be developed by a professional engineer registered in the State of Texas who is experienced in such designs, in accordance with current OSHA guidelines and other applicable industry standards.

6.3 Deep Foundations

Auger cast in place (ACIP) piles or drilled piers may be considered to support the structural loads for the Main Building. Recommendations for an ACIP pile or drilled straight-shaft pier foundation systems are provided in the following sections. *Note: Groundwater was encountered at this site. Construction difficulties may occur during straight-shaft installation due to the potential for sidewall collapse. Casing will be needed where groundwater is encountered. A test pier is recommended just prior to construction to determine the constructability of drilled shafts at this site and for the presence of groundwater.*

6.3.1 ACIP Piles

Auger cast in place (ACIP) piles could be utilized to support the relatively heavy foundation loads associated with this project. ACIP piles are generally installed by advancing a continuous-flight hollow-stem auger to a predetermined depth in the ground, and then pumping high-strength flowable cement grout into the excavated shaft through the bottom of the hollow auger as the auger is slowly withdrawn. The grout is pumped under relatively high pressure, and a positive head of grout is maintained above the base of the auger during auger extraction. After the auger is completely removed, reinforcing steel is placed.

Depending on subsurface conditions, the load carrying capacity of an ACIP pile is derived from end bearing and/or skin friction along the shaft. ACIP piles could be designed using unit skin friction and end bearing values in Table B.

TABLE B AUGER CAST-IN-PLACE PILES ALLOWABLE END BEARING AND SKIN FRICTION VALUES						
Depth from Existing Grades, ft			Allowable End Bearing, psf	Allowable Skin Friction, psf	Allowable Uplift Resistance, psf	Allowable Uplift Adhesion, psf
0	to	15	----	----	----	1,500 [1]
15	to	25	6,500	1,200	1,000	----
25	to	30	8,000	1,500	1,200	----
30	to	35	----	1,500	1,200	----
[1]	*May be reduced to 1,000 psf for moisture-conditioned soils and neglected for the portion of the shaft that penetrate through non-expansive, select fill material.*					
[2]	*Piers should not be drilled deeper than 35 ft below existing grades. If deeper piers are required, deeper borings will be necessary to verify these recommendations.*					

The above bearing capacity values contain a factor of safety of at least three (3) considering a general bearing capacity failure and the skin friction and uplift resistance values have a factor of safety of at least three (3). The capacity of pile groups can be less than the sum of the individual pile capacities within the group. To reduce group effects, we recommend that the adjacent piles be spaced on centers no closer than three (3) pile diameters, based on the larger pile. Pile spacing closer than three (3) pile diameters could result in reduction of the load carrying capacity of single piles and increased group settlement. At a pile spacing of two (2) pile diameters, the bearing capacities should be reduced by to 75 percent of the capacity of a single pile. Piles closer than two (2) pile diameters are not recommended. Once the pile group configurations have been finalized, we would be pleased to evaluate the efficiency of the pile groups if desired.

Uplift Force – Each pile shaft should be reinforced with suitable tension steel over its entire length to adequately resist potential uplift (tensile) forces due to potential soil swell (soil-to-pier adhesion) along the pile, from post construction heave and other uplift forces applied by structural loadings. The magnitude of uplift adhesion due to soil swell along the pile shaft cannot be defined accurately and can vary according to the actual in-place moisture content of the soils during construction. The estimated soil uplift adhesion force from Table B can be considered to act uniformly over the upper 15-feet of the pile from final grade. The soil uplift force can be neglected for the portion of the pile that penetrates through non-expansive, select fill material. The uplift force can be reduced by the dead load of the structure. The uplift resistance of each pier can be computed using the allowable uplift resistance values provided in Table B.

Based on our experience, we expect settlements to be less than about ½ inch for properly installed individual, isolated ACIP piles designed with an appropriate factor of safety, with a pile tip at a depth of at least 15-feet below the existing ground surface, and constructed in accordance with the recommendations presented herein. It should be noted that groups of piles will likely settle more than individual piles subjected to the same load per foundation. Differential settlements resulting from variations in subsurface conditions

and loading conditions are also expected to be small. Settlement response of ACIP piles is impacted more by the quality of construction than by soil-structure interaction.

6.3.1.1 ACIP Load Tests

Installation of ACIP piles is a highly specialized operation, and the successful performance of ACIP piles is highly dependent on the means and methods used by the pile installation contractor. We recommend full-scale ACIP pile load tests to verify the design pile capacity is achievable and to establish the methods and procedures to be used for production piles. A pile load test should be performed for each significant loading condition or class of pile used for the project, including lateral load test(s) if significant lateral loads are present. It is critical that the test pile(s) be installed using the same procedures that will be utilized for installation of production piles. We generally recommend testing the piles to failure, although this is not necessary.

From our experience, ACIP piles frequently achieve a higher capacity during load testing than that calculated from static analysis. Therefore, pile load tests can also be used to maximize the working load capacity of the piles, potentially reducing the number of piles and/or pile depth required for the project. Also, ACIP pile contractors frequently will propose alternative pile lengths and capacities based on their experience; contractor-proposed alternates should be verified by pile load tests.

Reaction piles used during pile load tests should not be utilized as production piles after load tests are complete. The reaction piles will be subject to uplift forces and displacements during the load test, and their axial capacity will be substantially reduced.

UES should be involved in development of load test objectives, should observe and document load test procedures and data, and should analyze the load test results.

6.3.2 Drilled Straight Shaft Piers

Drilled straight-shaft piers may be used to support the structural loads of the Multipurpose Academic Building and Future Weight Room. The piers should bear at least 15-feet below existing grade. Deeper pier depths will be required to develop skin friction and/or uplift resistance. Straight-sided drilled piers can be designed based on the following parameters provided in Table C.

TABLE C DRILLED STRAIGHT-SHAFT PIERS ALLOWABLE END BEARING AND SKIN FRICTION VALUES						
Depth from Existing Grades, ft			Allowable End Bearing, psf	Allowable Skin Friction, psf	Allowable Uplift Resistance, psf	Uplift Adhesion, psf
0	to	15	----	----	----	1,500 [1]
15	to	25	6,500	1,200	1,000	----
25	to	30	8,000	1,500	1,200	----
30	to	35	----	1,500	1,200	----
[1]	*May be reduced to 1,000 psf for moisture-conditioned soils and neglected for the portion of the shaft that penetrate through non-expansive, select fill material.*					
[2]	*Piers should not be drilled deeper than 35 ft below existing grades. If deeper piers are required, deeper borings will be necessary to verify these recommendations.*					

The above bearing capacity values contain a factor of safety of at least three (3) considering a general bearing capacity failure and the skin friction and uplift resistance values have a factor of safety of at least three (3). The allowable end bearing and skin friction values are based on center-to-center spacing of the pier foundations no closer than a horizontal distance of three (3) pier diameters (using the largest pier diameter). A closer spacing may be considered but may affect (reduce) the axial capacity of the foundation depending on the spacing pattern of the foundations.

The following reduction values should be used for piers spaced within three (3) pier diameters:

Axial Loading (Skin Friction):
3 diameters (center to center) or greater - no reduction is required.
2 diameters (center to center) will have a 25% reduction (0.75 times the value).
1 diameter (center to center) will have a 50% reduction (0.5 times the value).

Axial Loading (End Bearing):
For end bearing no reduction is required.

Note: Linear interpolate between pier diameters for pier spacing between the values given.

Uplift Force – Each pier shaft should be reinforced with suitable tension steel over its entire length to adequately resist potential uplift (tensile) forces due to potential soil swell (soil-to-pier adhesion) along the shaft, from post construction heave and other uplift forces applied by structural loadings. The magnitude of uplift adhesion due to soil swell along the pier shaft cannot be defined accurately and can vary according to the actual in-place moisture content of the soils during construction. The estimated soil uplift adhesion force from Table C can be considered to act uniformly over the pier shaft for the specified

depth from existing grade. The soil uplift force can be neglected for the portion of the shaft that penetrates through non-expansive, select fill material. The uplift force can be reduced by the dead load of the structure. The uplift resistance of each pier can be computed using the allowable uplift resistance values provided in Table C.

This calculated uplift force may be used to compute the longitudinal reinforcing steel required in the pier to resist the uplift force induced by the swelling clays. However, the cross-sectional area of the reinforcing steel should not be less than ½ percent of the gross cross-sectional area of the drilled pier shaft.

Total settlements, based on the indicated bearing pressure, should be less than 1-inch for properly designed and constructed drilled piers. Settlement beneath individual piers will be primarily elastic with most of the settlement occurring during construction. Differential settlement may also occur between adjacent piers. The amount of differential settlement could approach 50 to 75 percent of the total pier settlement. Settlement response of drilled piers is impacted more by the quality of construction than by soil-structure interaction.

6.3.3 Lateral Loading

Lateral analysis can be performed using the following design parameters provided for the site soils in Table D. The lateral resistance of the top portion of the pile shafts (portion within 6-feet of final grade) should be neglected.

TABLE D
DESIGN PARAMETERS FOR L-PILE

	Clay Soils		
	6 to 15 ft Below Final Grade	**15 ft to 35 ft Below Final Grade**	**Deeper Than 35 ft Below Grade**
L-Pile p-y Model	Soft Clay	Very Stiff Clay	Hard Clay
Effective Unit Weight (γ), pci	0.069	0.072	0.036
Undrained Cohesion (c), psi	3.4	20	35
Friction Angle (F), degrees	0	0	0

6.3.4 Grade Beams and Pier/Pile Caps

Grade beams connecting to piles or piers, pier caps, and pile caps should be formed and not cast in earthen trenches. Grade beams should be formed with a nominal 12-inch void at the bottom for a structurally suspended floor slab.

Commercially available cardboard box forms (cartons) are made for this purpose. The cardboard cartons should extend the full length and width of the grade beams. Prior to concrete placement, cartons should be inspected to verify they are firm, properly placed, and capable of supporting wet concrete. Some type of permanent soil retainer, such as

pre-cast concrete panels, must be provided to prevent soils adjacent to grade beams and caps from sloughing into the void space at the bottom of the grade beams and caps. Additionally, backfill soils placed adjacent to grade beams and caps must be compacted as outlined in Section 7.3 of this report.

6.4 Structurally Suspended Floor Slab on ACIP Piles or Drilled Piers

A structural slab suspended completely above the existing highly expansive soils could be used for the building supported on ACIP piles or piers. A structurally suspended floor slab will be required for potential movements of less than 1-inch. At least 12-inches of void space should be provided between the bottom of the floor slab (and lowest suspended fixture) and top surface of the underlying expansive clays. Utilities and other fixtures below the building should be suspended from the floor and isolated from the active clay soils, and a minimum 12-inch void space should also be provided below the bottom of the utility or fixture. Cardboard carton forms or a deeper crawl space can be used to create the minimum void space.

If a crawl space is constructed, consideration should be given to constructing a mud mat at the bottom of the crawl space. The benefits of a mud mat include (but are not limited to) resistance to moisture ingress through the suspended floor slab from the precipitation of moisture from the underlying soils and improved access below the structure for maintenance and repairs of utilities. In addition, proper ventilation should be provided to reduce the possibility that a high humidity environment could develop in the void space areas. We recommend that the crawl space subgrade be sloped (not exceeding 3 horizontal to 1 vertical) to appropriate drainage outlets to reduce the possibility of water accumulation in these areas. Flexible connections should be utilized in suspended piping, especially where it enters the ground.

6.5 Flatwork

Exterior flatwork supported on-grade could be subjected to potential seasonal movements up to about 6-inches as discussed in Section 6.1.2 of this report. Subgrade improvement, as discussed in Section 6.2 should be considered if it is desired to reduce these anticipated movements to about 1-inch and to reduce the risk of potential for differential movements between the flatwork and adjoining structural elements. Note: Select fill placed below flatwork may consist of material in compliance with "Select Fill (Flatwork)" outlined in section 7.3 of this report. Subgrade improvement below flatwork is intended to reduce the potential for large differential movements between the flatwork and the structure. However, some differential movement should be expected. Therefore, allowances should be made for differential movements between the structure and the flatwork, including flexible connections and control joints. The use of sand as a leveling course below flatwork supported on expansive clays should be avoided. *Note: UES should be contacted if additional subgrade improvement recommendations are needed for the flatwork area.*

The flatwork should be installed to ensure drainage away from the structure. A positive slope away from the structure should be maintained. The slope should be sufficient to accommodate future potential movements. The flatwork should never be allowed to reach either a level plane or negative slope back toward the structure. In addition, a moisture seal should be provided at the joint between the flatwork and the foundation.

6.6 Seismic Considerations

TABLE E SEISMIC PARAMETERS	
Description	**Values**
2021 International Building Code Site Classification (IBC) [1]	D [2]
Site Latitude (Degrees)	29.9958738
Site Longitude (Degrees)	-97.8507182
Mapped Spectral Acceleration for Short Periods (0.2-Second): (S_S) [3]	0.052 g
Mapped Spectral Acceleration for a 1-Second Period: (S_1) [3]	0.029 g
[1]	The site class definition was determined using SPT N-values in conjunction with section 1613.2.2 in the 2021 IBC and ASCE 7-16.
[2]	Section 20.1 in the 2010 ASCE-7 requires a site soil profile determination extending to a depth of 100 feet for seismic site classification. The current scope does not include the required 100-foot soil profile determination. Borings extended to a maximum depth of 35 ft, and this seismic site class definition considers that stiff soil continues below the maximum depth of the subsurface exploration. Additional exploration to deeper depths would be needed to confirm the conditions below the current depth of exploration.
[3]	The Spectral Acceleration values were determined using publicly available information provided on the United States Geological Survey (USGS) website. The spectral acceleration values can be used to determine the site coefficients using Tables 1613.2.3 (1) and 1613.2.3 (2) in the 2018 IBC.

6.7 Pavements

The soils encountered near the ground surface should be improved and prepared prior to construction of pavements at this site. To permit correlation between information from the borings and actual subgrade conditions exposed during construction, a qualified Geotechnical Engineer should be retained to provide subgrade monitoring and testing during construction. If there is any change in project criteria, the recommendations contained in this report should be reviewed by our office.

Calculations used to determine the required pavement thickness are based only on the physical and engineering properties of the materials and conventional thickness determination procedures. Pavement joining the buildings should be constructed with a curb and the joint between the building and curb should be sealed. Related civil design factors such as subgrade drainage, shoulder support, cross-sectional configurations, surface elevations, reinforcing steel, joint design and environmental factors will significantly affect the service life and must be included in preparation of the construction drawings and specifications, but were not included in the scope of this study. Normal periodic maintenance will be required for all pavements to achieve the design life of the pavement system.

Recommendations for both Portland Cement Concrete (PCC) and asphalt concrete pavements are provided below. These types of pavement are not considered equal in performance. Over the life of the pavement structure, asphalt concrete pavement should be expected to have a shorter life and higher maintenance costs. Also, pavement in dumpster areas and areas receiving heavy truck traffic should consist of PCC. The dumpster pads should be extended to include all wheels of any garbage trucks.

Based on our knowledge of the project, we anticipate that traffic loads will be produced primarily by automobile traffic and occasional delivery, trash removal trucks and school buses. For this project General Parking, Access Drives and Bus Loop pavement section alternatives are provided. General Parking is for areas expected to receive only car traffic. Access Drives assumes areas with some delivery truck traffic, trash trucks and main access drive areas. Bus Loop assumes 30 school bus trips per day and 5 trash removal truck trips per week with some car traffic. If heavier traffic loading is expected, UES should be provided with the information and allowed to review these pavement sections.

Note: The recommended pavement sections provided below are considered the minimum necessary to provide satisfactory performance based on the expected traffic loading. In some cases, City minimum standards for pavement section construction may exceed those provided below.

6.7.1 Pavement Subgrade Preparation

After final subgrade elevation has been achieved, the exposed subgrade preparation should consist of scarifying the exposed subgrade soils to a depth of at least 6-inches and then either lime treating or recompacting the scarified soils to at least 95 percent of standard Proctor maximum dry density (ASTM D 698) and within the range of -1 to +3 percentage points of the material's optimum moisture content. The pavement subgrade should be proof-rolled as described in Section 7.1 in this report. Recommendations for subgrade preparation (lime treated subgrade) are presented in Section 6.7.4.

It is recommended that subgrade preparation (lime treated subgrade) extend at least 1-foot beyond the edge of the pavement to reduce effects of seasonal shrinking and swelling upon the extreme edges of pavement. Also, the curb should be constructed such that the base of the curb extends at least 6-inches into the pavement subgrade.

Pavement will have the same potential for movement as discussed in Section 6.1.2 (up to about 6-inches). Good perimeter surface drainage with a minimum slope of 2 percent away from the pavement is recommended. The use of sand as a leveling course below pavement supported on expansive clays should be avoided. Normal maintenance of pavement should be expected over the life of the pavement structures.

Note: Based on our experience with similar projects in the general geographical area of the site, the risk of sulfate induced heave of lime-treated subgrade is considered to be low for on-site soils. We do not anticipate any special treatment of the subgrade in this regard. However, additional testing of soluble sulfates is recommended during construction once pavement grades are established.

6.7.2 Portland Cement Concrete Pavement

Subgrade preparation as described in Section 6.7.1 is required for PCC pavement. The minimum recommended PCC pavement sections to be constructed are provided in Table F:

TABLE F
PORTLAND CEMENT CONCRETE PAVEMENT SECTIONS

	General Parking 30,000 ESAL (inches)		Access Drives 100,000 ESAL (inches)		Bus Loop/Fire Lane 200,000 ESAL (inches)	
Reinforced PCC	5.0	5.5	6.0	6.5	7.0	7.5
Lime Treated Subgrade [1]	6.0	----	6.0	----	6.0	----
Recompacted Subgrade	----	6.0	----	6.0	----	6.0

A minimum of 7-inches of PCC is recommended for dumpster pads. PCC should have a minimum compressive strength of 4,000 lbs per sq inch (psi) at 28 days. Joints in concrete paving should not exceed 15-feet. Reinforcing steel should consist of No. 3 bars placed at 18 inches on-center in two directions for General Parking and No. 4 bars placed at 18-inches on-center in two directions for Access Drives and Bus Loops/Fire Lanes. *Note: Refer to ACI 330 for additional information on pavement joints and reinforcement.*

6.7.3 Asphalt Concrete Pavement

Subgrade preparation as described in Section 6.7.1 is required for asphalt concrete pavement. The minimum recommended asphalt concrete pavement sections to be constructed are provided in Table G. Pavement materials are described in Section 6.7.4.

TABLE G ASPHALT CONCRETE PAVEMENT SECTIONS						
	General Parking 18,000 ESAL (inches)		Access Drives 75,000 ESAL (inches)		Bus Loop/Fire Lane 200,000 ESAL (inches)	
HMAC Surface Course – Type C or D	2.0	2.0	2.0	2.0	2.5	2.5
Flexible Base	8.0	10.0	10.0	14.0	12.0	15.0
Lime Treated Subgrade [1,2]	6.0	----	6.0	----	6.0	----
Recompacted Subgrade	----	6.0	----	6.0	----	6.0
[1] *Geogrid may be used in lieu of the 6-inch lime treated subgrade. Criteria for geogrid can be found in Section 6.7.4.*						

6.7.4 Pavement Materials

Presented below are various materials that may be used to construct the pavement sections at this site. Submittals should be made for each pavement material. The submittals should be reviewed by the Geotechnical Engineer and appropriate members of the design team and should provide test information necessary to verify full compliance with the recommended or specified material properties.

Hot Mix Asphaltic Concrete (HMAC) Courses - The HMAC surface course should be plant mixed, hot laid Type C or D (TxDOT Standard Specifications Item 341). Each mix should meet the master specifications requirements of 2014 TxDOT Standard Specifications Item 341, Item SS 3224 (2011) and specific criteria for the job mix formula.

Flexible Base – Crushed limestone flexible base should meet TxDOT Standard Specification Item 247 Grade 1-2, Type A. Flexible base should be compacted to a minimum of 95 percent of the materials maximum Modified Proctor dry density (ASTM D1557) at a moisture content of -2 to +2 percentage points of optimum moisture.

Lime Treated Subgrade – Due to the presence of clayey soils (with a PI over 20) at this site, the pavement subgrade may be treated with hydrated lime. The subgrade should be scarified to a depth of 6 inches and mixed with a minimum 8 percent hydrated lime (by dry soil weight) in conformance with TxDOT Standard Specification Item 260. Assuming an in-place unit weight of 100 pcf for the pavement subgrade soils, this percentage of lime equates to about 36 lbs of lime per square yard of treated subgrade. The actual amount of lime required should be confirmed by additional laboratory tests (ASTM C 977 Appendix XI) prior to construction. The soil-lime mixture should be compacted to at least 95 percent of standard Proctor maximum dry density (ASTM D 698) and within the range of 0 to 4 percentage points above the mixture's optimum moisture content. In all areas where hydrated lime is used to stabilize subgrade soil, routine Atterberg-limit tests should be performed to verify the resulting

plasticity index of the soil-lime mixture is at/or below 20 percent. Subgrade preparation utilizing lime stabilization as described herein will not prevent normal seasonal movement of the underlying untreated materials.

Geogrid-Geogrid should be placed beneath the base material and on top of the compacted subgrade. Geogrid should be Tensar HX-5.5 and should be placed and overlapped in accordance with the manufacturer's recommendations. Geogrid will significantly improve the performance of the flexible pavements and reduce cracking.

If alternate geogrid products are desired for use, additional base material thickness will apply, and UES should be contacted for the specific recommendations. If a direct substitution with an alternate geogrid is proposed by the local geogrid distributor, the geogrid should come with a pavement design specific for the site that is sealed by a licensed professional engineer in the state of Texas and that pavement design shall supersede the pavement recommendations provided herein.

Recompacted Subgrade – The subgrade should be scarified to a depth of 6 inches and compacted to a dry density of at least 95 percent of standard Proctor maximum dry density (ASTM D 698) and within the range of 0 to +4 percentage points of optimum moisture content.

6.8 Drainage and Other Considerations

Adequate drainage should be provided to reduce seasonal variations in the moisture content of foundation soils. All pavement and sidewalks within 10-feet of a building should be sloped away from the building to prevent ponding of water around the building. Final grades within 10-feet of the building should be adjusted to slope away from the building at a minimum slope of 2 percent. **Maintaining positive surface drainage throughout the life of the structures is essential.**

In areas with pavement or sidewalks adjacent to the new structures, a positive seal must be maintained between the structure and the pavement or sidewalk to minimize seepage of water into the underlying supporting soils. Post-construction movement of pavement and flatwork is common. Normal maintenance should include examination of all joints in paving and sidewalks, etc. as well as resealing where necessary.

Several factors relate to civil and architectural design and/or maintenance, which can significantly affect future movements of the foundation and floor slab system:

- Preferably, a complete system of gutters and downspouts should carry runoff water a minimum of 5-feet from the completed structures.

- Large trees and shrubs should not be allowed closer to the foundations than a horizontal distance equal to roughly their mature canopy due to their significant moisture demand upon maturing. *Note: A landscape expert may be consulted to evaluate the precise extents of potential root growth for specific tree and shrub species so that root growth beneath the structures and pavements can be avoided.*

- Moisture conditions should be maintained "constant" around the edge of the slabs. Ponding of water in planters, in unpaved areas, and around joints in paving and sidewalks can cause slab movements beyond those predicted in this report.

- Planter box structures placed adjacent to the building should be provided with a means to assure concentrations of water are not available to the subsoil stratigraphy.

- The root systems from any existing trees cleared/removed at this site will have dried and desiccated the surrounding clay soils, resulting in soil with near-maximum swell potential. Clay soils surrounding tree root mats within the building areas or flatwork areas should be removed to a depth of 3-feet and compacted in-place with moisture and density control as described in Section 7.3 of this report.

Trench backfill for utilities should be properly placed and compacted as outlined in Section 7.3 of this report and in accordance with requirements of local City standards. Since granular bedding backfill is used for most utility lines, the backfilled trench should not become a conduit and allow access for surface or subsurface water to travel toward the new structures. Concrete cut-off collars or clay plugs should be provided where utility lines cross building lines to prevent water from traveling in the trench backfill and entering beneath the structures.

7.0 GENERAL CONSTRUCTION PROCEDURES AND RECOMMENDATIONS

Variations in subsurface conditions could be encountered during construction. To permit correlation between boring data and actual subsurface conditions encountered during construction, it is recommended a registered Professional Engineering firm be retained to observe construction procedures and materials.

Some construction problems, particularly degree or magnitude, cannot be anticipated until the course of construction. The recommendations offered in the following paragraphs are intended not to limit or preclude other conceivable solutions, but rather to provide our observations based on our experience and understanding of the project characteristics and subsurface conditions encountered in the borings.

7.1 Site Preparation and Grading

All areas supporting pavement, flatwork, or areas to receive new fill should be properly prepared.

- After completion of the necessary stripping, clearing, and excavating and prior to placing any required fill, the exposed soil subgrade should be carefully evaluated by probing and testing. Any undesirable material (organic material, wet, soft, or loose soil) still in place should be removed.

- The exposed soil subgrade should be further evaluated by proof-rolling with a heavy pneumatic tired roller, loaded dump truck or similar equipment weighing approximately 20 tons to check for pockets of soft or loose material hidden beneath a thin crust of possibly better soil.

- Proof-rolling procedures should be observed routinely by a Professional Engineer, or his designated representative.

- Any undesirable material (organic material, wet, soft, or loose soil) exposed during the proofroll should be removed and replaced with well-compacted select fill material as outlined in Section 7.3.

- Prior to placement of any fill, the exposed soil subgrade should then be scarified to a minimum depth of 6 inches and recompacted as outlined in Section 7.3.

Slope stability analysis of embankments (natural or constructed) was not within the scope of this study. If fill is to be placed on existing slopes (natural or constructed) steeper than six (6) horizontal to one (1) vertical (6:1), the fill materials should be benched into the existing slopes in such a manner as to provide a minimum bench width of five (5) feet. This should provide a good contact between the existing soils and new fill materials, reduce potential sliding planes and allow relatively horizontal lift placements.

The contractor is responsible for designing any excavation slopes, temporary sheeting or shoring. Design of these structures should include any imposed surface surcharges. Construction site safety is the sole responsibility of the contractor, who shall also be solely responsible for the means, methods and sequencing of construction operations. The contractor should also be aware that slope height, slope inclination or excavation depths (including utility trench excavations) should in no case exceed those specified in local, state and/or federal safety regulations, such as OSHA Health and Safety Standard for Excavations, 29 CFR Part 1926, or successor regulations. Stockpiles should be placed well away from the edge of the excavation and their heights should be controlled so they do not surcharge the sides of the excavation. Surface drainage should be carefully controlled to prevent flow of water over the slopes and/or into the excavations. Construction slopes should be closely observed for signs of mass movement, including tension cracks near the crest or bulging at the toe. If potential stability problems are observed, a geotechnical engineer should be contacted immediately. Shoring,

bracing or underpinning required for the project (if any) should be designed by a professional engineer registered in the State of Texas.

Due to the nature of the clayey and granular soils found near the surface at the borings, traffic of heavy equipment (including heavy compaction equipment) may create pumping and general deterioration of shallow soils. Therefore, some construction difficulties should be anticipated during periods when these soils are saturated.

7.2 Foundation Excavations

All foundation excavations should be monitored to verify foundations bear on suitable material. The bearing stratum exposed in the base of all foundation excavations should be protected against any detrimental change in conditions. Surface runoff water should be drained away from excavations and not allowed to collect. All concrete for foundations should be placed as soon as practical after the excavation is made. Prolonged exposure of the bearing surface to air or water will result in changes in strength and compressibility of the bearing stratum. Drilled piers should be completed the same day as excavated. All other excavations should not be left open for more than 48 hours. If delays occur, excavations for drilled piers should be slightly widened, deepened, and cleaned.

All pier shafts should be at least 1.5-feet in diameter to facilitate clean-out of the base and proper monitoring. Concrete placed in pier holes should be directed through a tremie, hopper, or equivalent. Placement of concrete should be vertical through the center of the shaft without hitting the sides of the pier or reinforcement to reduce the possibility of segregation of aggregates. Concrete placed in piers should have a minimum slump of 5-inches (but not greater than 7-inches) to avoid potential honey-combing.

Observations during pier drilling should include, but not necessarily be limited to, the following items:

- Verification of proper bearing strata and consistency of subsurface stratification with regard to boring logs,
- Confirmation the minimum required penetration into the bearing strata is achieved,
- Complete removal of cuttings from bottom of pier holes,
- Proper handling of any observed water seepage and sloughing of subsurface materials,
- No more than 2-inches of standing water should be permitted in the bottom of pier holes prior to placing concrete, and

- Verification of pier diameter, underream diameter (where applicable), and steel reinforcement.

Groundwater was encountered during drilling at depths of 16 and 31-feet below the existing ground surface in borings B-2 and B-4, during drilling at this site. From our experience, seasonal groundwater seepage could be encountered at the site during pier installation, and the risk of encountering seepage is increased during or after periods of precipitation. Immediate placement of concrete may be required to prevent sidewall collapse from impacting pier or pile construction. Temporary casing may be required to control sloughing of the subsurface soils and groundwater seepage encountered during the pier drilling. The casing should be properly seated below the depth of seepage and groundwater should be removed prior to beginning the design penetration. As casing is extracted, care should be taken to maintain a positive head of plastic concrete and minimize the potential for intrusion of sloughing of fill soils. It is recommended a separate bid item be provided for casing on the contractor's bid schedule. Underwater or slurry placement of concrete may also be necessary in conjunction with or as an alternate to casing depending on the depth to groundwater at the time of pier drilling. Pier drilling contractors experienced in similar soil and groundwater conditions should be utilized for this project. The sidewalls of the pier excavation need to be sufficiently rough or artificially roughened to mobilize skin friction. UES should be contacted for further review and evaluation if groundwater seepage and/or pier shaft sidewall collapse occurs during pier installation.

When the pier excavation depth is achieved and the bearing area has been cleaned, steel and concrete should then be placed immediately in the excavation. The concrete should be placed completely to the bottom of the excavation with a closed-end tremie in the pier excavation if more than 2-inches of water is ponded on the bearing surface or the slurry drilling technique is used. A short tremie may be used if the excavation has less than 2-inches of ponded water or if the water is pumped out prior to concrete placement. The fluid concrete should not be allowed to strike the pier reinforcement, temporary casing (if required) or excavation sidewalls during concrete placement.

7.3 Fill Material and Compaction

The following fill materials and compaction recommendations provided below are applicable for general site grading and other structural areas.

Select Fill – Materials used as select fill material should consist of a "non-expansive" material with a liquid limit less than 35 percent, a PI not less than about 5 percent or greater than 15 percent and contain no more than 0.5 percent fibrous organic materials, by weight. All select fill material should contain no deleterious material and should be compacted to a dry density of at least 95 percent standard Proctor maximum dry density (ASTM D 698) and within the range of 1 percentage point below to 3 percentage points above the material's optimum moisture content. *Note: The plasticity index and liquid limit of material used as select fill material should be routinely verified during placement using*

laboratory tests. Visual observation and classification should not be relied upon to confirm the material to be used as select fill material satisfies the above Atterberg-limit criteria.

Flexible Base – Flexible base used as non-expansive fill in the building pad should consist of material meeting the requirements of TxDOT Standard Specifications Item 247, Type A, B, C or D, Grade 1-2. The flexible base should be compacted to at least 98 percent of modified Proctor maximum dry density (ASTM D 1557) and within the range of 2 percentage points below to 2 percentage points above the material's optimum moisture content. *Note: Any flexible base used for pavement applications should meet the requirements of Section 6.7.4.*

The following fill compaction recommendations provided below are applicable for general site grading. *Note: Imported soils used as general fill should consist of material with a PI not greater than 35 percent.*

General Fill (Clay) – Clay soils should be compacted to a dry density between 95 and 100 percent of standard Proctor maximum dry density (ASTM D 698). The compacted moisture content of the clays during placement should be within the range of 0 to 4 percentage points above optimum. Clayey materials used as fill should be processed and the largest particle or clod should be less than 6-inches prior to compaction.

General Fill (Granular) – Granular materials should be compacted to a dry density between 95 and 100 percent of standard Proctor maximum dry density (ASTM D 698). The compacted moisture content of the granular soils during placement should be within the range of -2 to +2 percentage points of optimum.

Prior to placement of any fill, the subgrade should be scarified to a depth of 6-inches and recompacted to a dry density of at least 95 percent of standard Proctor maximum dry density (ASTM D 698) and within the range of -1 to +4 percentage points of the material's optimum moisture content.

In cases where mass fills outside the structure areas are more than 12-feet deep, the fill below 12-feet should be compacted to at least 100 percent of standard Proctor maximum dry density (ASTM D-698) and within 2 percentage points of the material's optimum moisture content. The portion of the fill shallower than 12-feet should be compacted as outlined above. *Note: Even if fill is properly compacted, fills in excess of about 12-feet are still subject to settlements over time of up to about 1 to 2 percent of the total fill thickness. This should be considered when designing structures and pavements on relatively deep backfill. UES should be contacted if alterative backfill recommendations are required to reduce settlement of mass fills.*

Compaction should be accomplished by placing fill in about 8-inch thick loose lifts and compacting each lift to at least the specified minimum dry density. Field density and moisture content tests should be performed on each lift. A qualified geotechnical engineering firm should

be retained to perform sufficient in-place density tests during the filling operations to evaluate that proper levels of compaction, including dry unit weight and moisture content, are being attained. Controlled, compacted fill should consist of approved materials that are free of organic matter and debris or materials exceeding 4-inches in maximum dimension. *Note: We recommend any imported fill to be used at this site be approved by UES prior to placement.*

7.4 Utilities

In cases where utility lines are more than 12-feet deep, the trench backfill below 12-feet should be compacted to at least 100 percent of standard Proctor maximum dry density (ASTM D 698) and within –2 to +2 percentage points of the material's optimum moisture content. The portion of the trench backfill shallower than 12-feet should be compacted as previously outlined. Density tests should be performed on each lift (maximum 12-inch thick) and should be performed as the trench is being backfilled. *Note: Even if the utility backfill is properly compacted, fills in excess of about 12-feet are still subject to settlements over time of up to about 1 to 2 percent of the total fill thickness. This should be considered when designing pavements or other structures over utility lines and/or other areas with deep fill. UES should be contacted if alterative backfill recommendations are required to reduce settlement of deep utility lines.*

If utility trenches or other excavations extend to or beyond a depth of 5-feet below construction grade, the contractor or others shall be required to develop an excavation safety plan to protect personnel entering the excavation or excavation vicinity. The collection of specific geotechnical data and the development of such a plan, which could include designs for sloping and benching or various types of temporary shoring, is beyond the scope of this study. Any such designs and safety plans shall be developed in accordance with current OSHA guidelines and other applicable industry standards.

7.5 Groundwater

Groundwater was encountered during drilling at depths of 16 and 31-feet below the existing ground surface in borings B-2 and B-4, during drilling at this site. From our experience with similar soils, seasonal groundwater seepage could be encountered in excavations for grade beams, foundations, utility conduits and other general excavations. The risk of encountering seepage increases with depth of excavation and during or after periods of precipitation. Standard sump pits and pumping may be adequate to control minor seepage on a local basis in relatively shallow excavations.

In any areas where cuts are made to establish final grades at the site, attention should be given to possible seasonal water seepage that could occur through natural cracks and fissures in the newly exposed stratigraphy. Subsurface drains may be required to intercept seasonal groundwater seepage. The need for these or other de-watering devices should be carefully addressed during construction. Our office could be contacted to visually observe the final grades to evaluate the need for such drains.

8.0 LIMITATIONS

Professional services provided in this geotechnical exploration were performed, findings obtained, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. The scope of services provided herein does not include an environmental assessment of the site or investigation for the presence or absence of hazardous materials in the soil, surface water or groundwater. UES, upon written request, can be retained to provide these services.

UES is not responsible for conclusions, opinions or recommendations made by others based on this data. Information contained in this report is intended for the exclusive use of the Client (and their designated design representatives), and is related solely to design of the specific structures outlined in Section 2.0. No party other than the Client (and their designated design representatives) shall use or rely upon this report in any manner whatsoever unless such party shall have obtained UES's written acceptance of such intended use. Any such third party using this report after obtaining UES's written acceptance shall be bound by the limitations and limitations of liability contained herein, including UES's liability being limited to the fee paid to it for this report. Recommendations presented in this report should not be used for design of any other structures except those specifically described in this report. In all areas of this report in which UES may provide additional services if requested to do so in writing, it is presumed that such requests have not been made if not evidenced by a written document accepted by UES. Further, subsurface conditions can change with passage of time. Recommendations contained herein are not considered applicable for an extended period of time after the completion date of this report. It is recommended our office be contacted for a review of the contents of this report for construction commencing more than one (1) year after completion of this report. Non-compliance with any of these requirements by the Client or anyone else shall release UES from any liability resulting from the use of, or reliance upon, this report.

Recommendations provided in this report are based on our understanding of information provided by the Client about characteristics of the project. If the Client notes any deviation from the facts about project characteristics, our office should be contacted immediately since this may materially alter the recommendations. Further, UES is not responsible for damages resulting from workmanship of designers or contractors. It is recommended the Owner retain qualified personnel, such as a Geotechnical Engineering firm, to verify construction is performed in accordance with plans and specifications.

APPENDIX

A-1 METHODS OF FIELD EXPLORATION

A truck-mounted, rotary drill rig equipped with continuous flight augers or air rotary augers were used to advance the boreholes. A total of five (5) borings were performed for this geotechnical exploration at the approximate locations shown on the Boring Location Plan. The boring locations were staked by using a handheld GPS device in conjunction with assessing landmarks which could be identified in the field and as shown on the site plan provided during this study. The locations of the borings shown on the Boring Location Plans are considered accurate only to the degree implied by the methods used to define them. The approximate latitude and longitude coordinates at each boring location were obtained using a handheld GPS device.

Relatively undisturbed samples of the cohesive subsurface materials were obtained by hydraulically pressing 3-inch O.D. thin-wall sampling tubes into the underlying soils at selected depths (ASTM D 1587). These samples were removed from the sampling tubes in the field and evaluated visually. One representative portion of each sample was sealed in a plastic bag for use in future visual evaluations and possible testing in the laboratory.

Samples of granular, cohesive, and rock materials were obtained using split-spoon sampling procedures in general accordance with ASTM Standard D 1586. Disturbed samples were obtained at selected depths in the borings by driving a standard 2-inch O.D. split-spoon sampler 18-inches into the subsurface material using a 140-pound hammer falling 30-inches. The number of blows required to drive the split-spoon sampler the final 12-inches of penetration (N-value) is recorded in the appropriate column on the boring logs. However, if the sampler was not driven the initial 6-inch seating increment with 50 hammer blows, refusal (i.e. “ref”) is recorded along with the inches driven on the logs.

Our field representative prepared field logs as part of the field exploration. The field logs included visual descriptions of the materials encountered during drilling and their interpretation of the subsurface conditions between samples. The Log of Boring sheets included in this report represent the engineer’s interpretation of the field logs and include modifications based on visual observations using the Unified Soil Classification System (USCS) and testing of the samples in the laboratory. **Samples not consumed by testing will be retained in our laboratory for at least 30 days and then discarded unless the Client requests otherwise.**

SITE VICINITY MAP

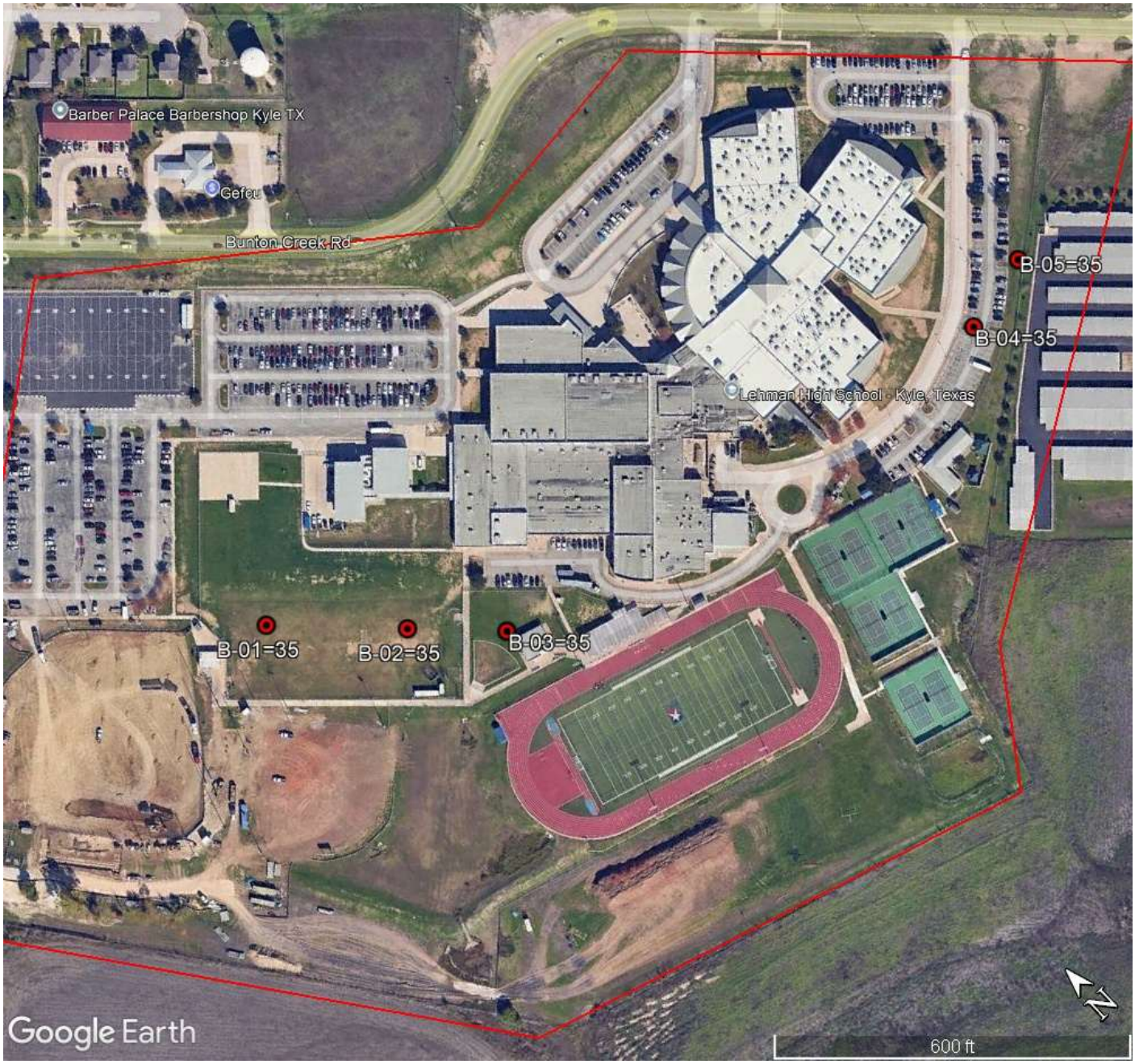
LOCATION IS APPROXIMATE

Hays CISD
UES Project No.: A251017

Lehman HS
1700 Lehman Road
Kyle, Texas

BORING LOCATION PLAN

LOCATIONS ARE APPROXIMATE



Hays CISD
UES Project No.: A251017

Lehman HS
1700 Lehman Road
Kyle, Texas

BORING LOCATION PLAN

LOCATIONS ARE APPROXIMATE

Hays CISD
UES Project No.: A251017

Lehman HS
1700 Lehman Road
Kyle, Texas

B-1 METHODS OF LABORATORY TESTING

Representative samples were inspected and classified by a qualified member of the Geotechnical Division and the boring logs were edited as necessary. To aid in classifying the subsurface materials and to determine the general engineering characteristics, natural moisture content tests (ASTM D 2216), Atterberg-limit tests (ASTM D 4318), and percent passing No. 200 Sieve (ASTM D 1140) were performed on selected samples. Results of these laboratory tests are provided on the Logs of Boring sheets.

In addition to the Atterberg-limit tests, the expansive properties of the clay soils were further analyzed by absorption swell tests (ASTM D 4546, Method B). The swell test is performed by placing a selected sample in a consolidation machine and applying the overburden pressure and then allowing the sample to absorb water. When the sample exhibits very little tendency for further expansion, the height increase is recorded and the percent swell and total moisture gain calculated. Results of the absorption swell tests are provided on the Logs of Boring sheets.

LOG OF BORING 01

UES Professional Solutions, LLC
4740 Perrin Creek, Suite 480
San Antonio, Texas 78217
Telephone: 210-249-2100
Fax: 210-249-2101

CLIENT: Hays CISD
PROJECT: Lehman HS
LOCATION: 1700 Lehman Road, Kyle, Texas
NUMBER: A251017
DATE(S) DRILLED: 3/12/2025 - 3/12/2025

DRILLING METHOD(S):
Solid Flight Auger

GROUNDWATER INFORMATION:
Groundwater was not encountered during drilling, and the boring was dry upon completion of drilling.

SURFACE ELEVATION: N/A

SOIL SYMBOL	DEPTH (FT)	SAMPLE TYPE	SAMPLE SYMBOL	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT Qc: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PL	ATTERBERG LIMITS PI	DRY DENSITY (POUNDS/CU.FT)	COMPRESSIVE STRENGTH (TONS/SQ FT)	MINUS NO. 200 SIEVE (%)	DESCRIPTION OF STRATUM
		SPT		N=9	10	50	15	35			79	**FILL: SANDY FAT CLAY**, stiff, brown and light brown. (CH)
		ST		P=4.5+	12	57	17	40			95	**FAT CLAY**, hard, dark brown. (CH)
	5	ST		P=4.5+	10							
		ST		P=4.5+	7	52	15	37	118		81	**FAT CLAY WITH SAND**, hard to very stiff, light brown, with calcareous deposits. (CH) - (swell result = 2.2%, final moisture = 16.3%)
		SPT		N=26	8							
	10											
		ST		P=4.5+	15							**FAT CLAY**, hard, yellow light brown, with gypsum deposits. (CH)
	15											
		ST		P=4.5+	14	55	16	39	110	1.0	96	
	20											
		ST		P=4.5+	11							
	25											
		ST		P=4.5+	15	60	18	42	107	6.4	94	- (confining pressure = 24.7 psi)
	30											
		ST		P=4.5+	13							
	35											Boring terminated at a depth of 35-feet.

N - STANDARD PENETRATION TEST RESISTANCE
Qc - STATIC CONE PENETROMETER TEST INDEX
P - POCKET PENETROMETER RESISTANCE

REMARKS:
Boring location determined by UES. Drilling operations performed by UES.
GPS Coordinates: N 29.9958738°, W -97.8507182°

LOG OF BORING 02

UES Professional Solutions, LLC
4740 Perrin Creek, Suite 480
San Antonio, Texas 78217
Telephone: 210-249-2100
Fax: 210-249-2101

CLIENT: Hays CISD
PROJECT: Lehman HS
LOCATION: 1700 Lehman Road, Kyle, Texas
NUMBER: A251017
DATE(S) DRILLED: 3/12/2025 - 3/12/2025

DRILLING METHOD(S):
Solid Flight Auger

GROUNDWATER INFORMATION:
Groundwater was encountered at a depth of 16-feet during drilling. Groundwater was measured at a depth of 17-feet 3- inches upon completion of the drilling.

SURFACE ELEVATION: N/A

SOIL SYMBOL	DEPTH (FT)	SAMPLE TYPE	SAMPLE SYMBOL	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT Qc: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS LIQUID LIMIT LL	ATTERBERG LIMITS PLASTIC LIMIT PL	ATTERBERG LIMITS PLASTICITY INDEX PI	DRY DENSITY (POUNDS/CU.FT)	COMPRESSIVE STRENGTH (TONS/SQ FT)	MINUS NO. 200 SIEVE (%)	DESCRIPTION OF STRATUM
		ST		P=4.5+	4	33	15	18			72	**LEAN CLAY WITH SAND**, hard to very stiff, dark brown to brown, with calcareous deposits. (CL)
		SPT		N=15	10							
	5	ST		P=3.0	8							
		ST		P=4.5+	11	31	13	18	104		91	**LEAN CLAY**, hard, light brown, with calcareous deposits. (CL) - (swell result = 1.1%, final moisture = 22.0%)
		ST		P=4.5+	15							
	10											
		ST		P=1.5	10							**CLAYEY GRAVEL WITH SAND**, dense, light brown, (41% gravel). (GC)
	15											
		SPT		N=48	7	33	15	18			18	
	20											**FAT CLAY**, hard, yellow light brown. (CH)
		ST		P=4.5+	14							
	25											
		ST		P=4.5+	14							
	30											
		ST		P=4.5+	11	57	18	39	110	8.6	93	- (confining pressure = 28.9 psi)
	35											Boring terminated at a depth of 35-feet.

N - STANDARD PENETRATION TEST RESISTANCE
Qc - STATIC CONE PENETROMETER TEST INDEX
P - POCKET PENETROMETER RESISTANCE

REMARKS:
Boring location determined by UES. Drilling operations performed by UES.
GPS Coordinates: N 29.9953581°, W -97.8500825°

LOG OF BORING 03

UES Professional Solutions, LLC
4740 Perrin Creek, Suite 480
San Antonio, Texas 78217
Telephone: 210-249-2100
Fax: 210-249-2101

CLIENT: Hays CISD
PROJECT: Lehman HS
LOCATION: 1700 Lehman Road, Kyle, Texas
NUMBER: A251017
DATE(S) DRILLED: 3/13/2025 - 3/13/2025

DRILLING METHOD(S):
Solid Flight Auger

GROUNDWATER INFORMATION:
Groundwater was not encountered during drilling, and the boring was dry upon completion of drilling.

SURFACE ELEVATION: N/A

SOIL SYMBOL	DEPTH (FT)	SAMPLE TYPE	SAMPLE SYMBOL	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT Qc: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PL	ATTERBERG LIMITS PI	DRY DENSITY (POUNDS/CU.FT)	COMPRESSIVE STRENGTH (TONS/SQ FT)	MINUS NO. 200 SIEVE (%)	DESCRIPTION OF STRATUM
		ST		P=4.5+	5	44	12	32			55	**FILL: SANDY LEAN CLAY**, hard to stiff, brown. (CL)
		ST		P=4.5+	6							
	5	ST		P=4.0	18	54	15	39			89	**FAT CLAY**, very stiff, dark brown. (CH)
		ST		P=4.5	12							**LEAN CLAY**, hard, light brown. (CL)
		ST		P=4.5	13	38	13	25	114		88	- (swell result = 0.5%, final moisture = 17.4%)
	10											
		ST		P=3.5	12							
	15											**SANDY LEAN CLAY**, hard to very stiff, light brown. (CL)
		ST		P=4.5	10	25	11	14	115	1.0	55	
	20											
		ST		P=4.5+	15							**FAT CLAY**, hard, yellow light brown. (CH)
	25											
		ST		P=4.5+	16							
	30											
		ST		P=4.5+	15	55	17	38	105	7.0	96	- (confining pressure = 28.9 psi)
	35											Boring terminated at a depth of 35-feet.

N - STANDARD PENETRATION TEST RESISTANCE
Qc - STATIC CONE PENETROMETER TEST INDEX
P - POCKET PENETROMETER RESISTANCE

REMARKS:
Boring location determined by UES. Drilling operations performed by UES.
GPS Coordinates: N 29.9950677°, W -97.8497962°

LOG_OF_BORING A251017.GPJ ROCK_ETL.GDT 3/27/25

LOG OF BORING 04

UES Professional Solutions, LLC
4740 Perrin Creek, Suite 480
San Antonio, Texas 78217
Telephone: 210-249-2100
Fax: 210-249-2101

CLIENT: Hays CISD
PROJECT: Lehman HS
LOCATION: 1700 Lehman Road, Kyle, Texas
NUMBER: A251017
DATE(S) DRILLED: 3/13/2025 - 3/13/2025

DRILLING METHOD(S):
Solid Flight Auger

GROUNDWATER INFORMATION:
Groundwater was encountered at a depth of 31-feet during drilling. Groundwater was measured at a depth of 32-feet 4-inches upon completion of the drilling.

SURFACE ELEVATION: N/A

SOIL SYMBOL	DEPTH (FT)	SAMPLE TYPE	SAMPLE SYMBOL	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT Qc: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PL	ATTERBERG LIMITS PI	DRY DENSITY (POUNDS/CU.FT)	COMPRESSIVE STRENGTH (TONS/SQ FT)	MINUS NO. 200 SIEVE (%)	DESCRIPTION OF STRATUM
		GB			4	15	11	4			24	**ASPHALT**, 2-inches.
		ST		P=3.0	13							**FILL: SILTY, CLAYEY GRAVEL WITH SAND**, 10-inches, brown and red, (47% gravel). (GC-GM)
		ST		P=3.0	14	77	17	60			94	**FAT CLAY**, very stiff, dark brown and brown, with calcareous deposits. (CH)
	5	ST		P=3.0	14							
		ST		P=3.5	17							
	10	ST		P=4.5+	10	45	18	27	104		85	**LEAN CLAY**, very stiff to hard, light brown. (CL) - (swell result = 0.2%, final moisture = 21.5%)
		ST		P=4.5+	10							
	15											
		ST		P=4.5+	7							
	20											
		ST		P=3.0	11	33	15	18	114	6.5	88	- (confining pressure = 20.4 psi)
	25											
		ST		P=2.5	8							
	30											**FAT CLAY**, very stiff to hard, yellow light brown. (CH)
		ST		P=4.5+	16	67	22	45	106	9.3	96	- (confining pressure = 28.9 psi)
	35											Boring terminated at a depth of 35-feet.

N - STANDARD PENETRATION TEST RESISTANCE
Qc - STATIC CONE PENETROMETER TEST INDEX
P - POCKET PENETROMETER RESISTANCE

REMARKS:
Boring location determined by UES. Drilling operations performed by UES.
GPS Coordinates: N 29.9946098°, W -97.8468833°

LOG OF BORING 05

UES Professional Solutions, LLC
4740 Perrin Creek, Suite 480
San Antonio, Texas 78217
Telephone: 210-249-2100
Fax: 210-249-2101

CLIENT: Hays CISD
PROJECT: Lehman HS
LOCATION: 1700 Lehman Road, Kyle, Texas
NUMBER: A251017
DATE(S) DRILLED: 3/13/2025 - 3/13/2025

DRILLING METHOD(S):
Solid Flight Auger

GROUNDWATER INFORMATION:
Groundwater was not encountered during drilling, and the boring was dry upon completion of drilling.

SURFACE ELEVATION: N/A

SOIL SYMBOL	DEPTH (FT)	SAMPLE TYPE	SAMPLE SYMBOL	N: BLOWS/FT P: TONS/SQ FT T: TONS/SQ FT Qc: TONS/SQ FT	MOISTURE CONTENT (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PL	ATTERBERG LIMITS PI	DRY DENSITY (POUNDS/CU.FT)	COMPRESSIVE STRENGTH (TONS/SQ FT)	MINUS NO. 200 SIEVE (%)	DESCRIPTION OF STRATUM
		ST		P=3.0	13							**FILL: SILTY, CLAYEY GRAVEL WITH SAND**, very stiff, brown and red. (GC-GM)
		ST		P=2.5	17	63	21	42			86	**FAT CLAY**, very stiff to hard, dark brown and brown. (CH)
	5	ST		P=4.5+	13							
		ST		P=4.5+	12	46	16	30	120		91	**LEAN CLAY**, hard, light brown, with calcareous deposits. (CL) - (swell result = 1.5%, final moisture = 15.7%)
		ST		P=4.5+	9							
	10											**FAT CLAY**, hard, yellow light brown. (CH)
		ST		P=4.5+	15	66	21	45	107	7.4	95	
	15											
		ST		P=4.5+	14							
	20											
		ST		P=4.5+	16							
	25											
		ST		P=4.5+	11	64	20	44	110	13.6	93	- (confining pressure = 24.7 psi)
	30											
		ST		P=4.5+	11							
	35											Boring terminated at a depth of 35-feet.

N - STANDARD PENETRATION TEST RESISTANCE
Qc - STATIC CONE PENETROMETER TEST INDEX
P - POCKET PENETROMETER RESISTANCE

REMARKS:
Boring location determined by UES. Drilling operations performed by UES.
GPS Coordinates: N 29.9946937°, W -97.8465158°

UES™

KEY TO SOIL CLASSIFICATION AND SYMBOLS

UNIFIED SOIL CLASSIFICATION SYSTEM

MAJOR DIVISIONS		SYMBOL	NAME
COARSE GRAINED SOILS	GRAVEL AND GRAVELLY SOILS	GW	Well Graded Gravels or Gravel-Sand mixtures, little or no fines
		GP	Poorly Graded Gravels or Gravel-Sand mixtures, little or no fines
		GM	Silty Gravels, Gravel-Sand-Silt mixtures
		GC	Clayey Gravels, Gravel-Sand-Clay Mixtures
	SAND AND SANDY SOILS	SW	Well Graded Sands or Gravelly Sands, little or no fines
		SP	Poorly Graded Sands or Gravelly Sands, little or no fines
		SM	Silty Sands, Sand-Silt Mixtures
		SC	Clayey Sands, Sand-Clay mixtures
	SILTS AND CLAYS LL < 50	ML	Inorganic Silts and very fine Sands, Rock Flour, Silty or Clayey fine Sands or Clayey Silts
		CL	Inorganic Clays of low to medium plasticity, Gravelly Clays, Sandy Clays, Silty Clays, Lean Clays
		OL	Organic Silts and Organic Silt-Clays of low plasticity
	SILTS AND CLAYS LL > 50	MH	Inorganic Silts, Micaceous or Diatomaceous fine Sandy or Silty soils, Elastic Silts
		CH	Inorganic Clays of high plasticity, Fat Clays
		OH	Organic Clays of medium to high plasticity, Organic Silts
NON USCS MATERIALS			Limestone
			Marl/Claystone
			Sandstone

TERMS CHARACTERIZING SOIL STRUCTURE

SLICKENSIDED - having inclined planes of weakness that are slick and glossy in appearance

FISSURED - containing shrinkage cracks, frequently filled with fine sand or silt; usually more or less vertical

LAMINATED (VARVED) - composed of thin layers of varying color and texture, usually grading from sand or silt at the bottom to clay at the top

CRUMBLY - cohesive soils which break into small blocks or crumbs on drying

CALCAREOUS - containing appreciable quantities of calcium carbonate, generally nodular

WELL GRADED - having wide range in grain sizes and substantial amounts of all intermediate particle sizes

POORLY GRADED - predominantly of one grain size uniformly graded) or having a range of sizes with some intermediate size missing (gap or skip graded)

SYMBOLS FOR TEST DATA

- Groundwater Level (Initial Reading)
- Groundwater Level (Final Reading)
- Shelby Tube Sample
- SPT Samples
- Auger Sample
- Rock Core
- Texas Cone Penetrometer
- Grab Sample

TERMS DESCRIBING CONSISTENCY OF SOIL

COARSE GRAINED SOILS		FINE GRAINED SOILS		
DESCRIPTIVE TERM	NO. BLOWS/FT. STANDARD PEN. TEST	DESCRIPTIVE TERM	NO. BLOWS/FT. STANDARD PEN. TEST	UNCONFINED COMPRESSION TONS PER SQ. FT.
Very Loose	0 - 4	Very Soft	< 2	< 0.25
Loose	4 - 10	Soft	2 - 4	0.25 - 0.50
Medium Dense	10 - 30	Firm	4 - 8	0.50 - 1.00
Dense	30 - 50	Stiff	8 - 15	1.00 - 2.00
Very Dense	over 50	Very Stiff	15 - 30	2.00 - 4.00
		Hard	over 30	over 4.00

Field Classification for "Consistency" of Fine Grained Soils is determined with a 0.25" diameter penetrometer

SECTION 01 2300
ALTERNATES

PART 1 GENERAL

1.01 SECTION INCLUDES

A. Description of Alternates.

B. Procedures for pricing Alternates.

1.02 RELATED REQUIREMENTS

A. Document 00 2116 - Instructions to Proposers

1.03 PROCEDURES

A. Proposers are required to submit alternate proposals to add work or to deduct work from the base proposal as described below. Failure to submit alternate amounts in spaces provided on proposal form is basis for disqualification of proposal.

B. The successful proposer shall not modify, withdraw or cancel any of the alternate proposals or any part thereof for 45 days after date of receipt of proposals, unless specifically noted otherwise.

C. Contractor shall be responsible for any changes in the work affected by acceptance of these alternates. Include within the alternative proposal prices all costs, including materials, installations, and fees.

D. Claims for additional dollars resulting from changes caused by the alternates will not be allowed.

E. Refer to the drawings and project manual for items of work affected by alternates.

F. Alternates will be exercised at the option of the Owner.

G. Coordinate related work and modify surrounding work as required to complete the Work, including changes under each alternate, when acceptance is designated in the Owner - Contractor Agreement.

1.04 ACCEPTANCE OF ALTERNATES

A. Indicate variation of proposal price for alternates described below and list on the proposal form or any supplement to it, which requests a 'difference' in proposal price by adding to or deducting from the base proposal price or by indicating "No Charge".

B. Indicating "No Bid" as an alternate is unacceptable and is reason for rejection of the proposal.

C. Alternates quoted on Bid / Proposal Forms will be reviewed and accepted or rejected at Owner's option. Accepted Alternates will be identified in the Owner-Contractor Agreement.

D. Coordinate related work and modify surrounding work to integrate the Work of each Alternate.

1.05 SCHEDULE OF ALTERNATES

A. Alternate Number 1:
   1. Construction of the Multi-Purpose Athletic Facility as specified and where shown on the drawings.

B. Alternate Number 2:
   1. Add card reader access at classroom, workrooms and other locations as specified and where shown on the drawings. Infrastructure to be in base bid.

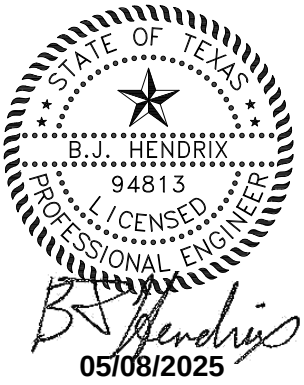
C. Alternate Number 3:
   1. Construction of the Nort Parking Lot as specified and where shown on the drawings.

PART 2 PRODUCTS - NOT USED

PART 3 EXECUTION - NOT USED

END OF SECTION

DIVISION 26 & 28
HAYS CISD
LEHMAN HIGH SCHOOL
ADDITIONS & RENOVATIONS
2025 Bond
ELECTRICAL SPECIFICATIONS



05/08/2025

CONTENTS:

SECTION 21 0000 - FIRE PROTECTION

PART 1 - DESCRIPTION

1.01 DESCRIPTION

A. Description:

1. This section describes specific requirements, products and methods of execution relating to fire protection for the project.

2. Temporary occupancy permits, or partial occupancy require alarm and life safety systems to be fully operational. Expedite installation and complete life safety requirements for temporary occupancy.

B. Scope: Provide a system of fire protection, complete in every detail and in perfect operating order, in accordance with the contract documents and applicable codes and standards to protect the areas noted, including all piping, valves, backflow preventers, sprinkler heads, heat shields, fire department connections, alarm valves, flow switches, hangers, supports, pertinent accessories, etc. for a complete turnkey job.

1. Remodel or Expansion Scope:

a. Expand existing fire sprinkler system to the remainder of the building.

b. All Additions Provide complete new system.

c. Phasing : Phasing of the sprinkler system installation will be required. All phasing shall be coordinated with General Contractor, they are in charge of all phasing. General Contractor will provide separate plan as applicable to communicate phasing. Summer work, during school year work, start and stop dates etc….

d. Coordinate all ceiling work with General Contractor.

2. New Construction : Provide Complete system.

3. This is a performance specification. Construction and installation drawings and hydraulic calculations shall be prepared according to standard practice, by a Sprinkler designer under the supervision of a State of Texas licensed RME-G or a Professional Engineer. Change in installation necessary due to lack of coordination with other trades shall be made without additional cost to the Owner. Verify final location of all exposed piping with the Architect.

4. Coordinate final location of Fire Department Connection (FDC) with local Fire Marshall. Provide all piping and materials required for installation of FDC.

5. Provide Class I standpipe system in stairways and at locations if required in accordance with IFC and NFPA 14. Fire hose threads used in connection with the standpipe systems shall be approved and shall be compatible with fire department hose threads. Final location of fire department hose connections shall be approved by Fire Marshal and coordinated with Architect. Standpipes shall have fire department valve in cabinet.

ADD #1

6. Provide Class III standpipes in stages greater than 1,000 square feet as required by IFC and NFPA 14. Fire hose threads used in connection with the standpipe systems shall be approved and shall be compatible with fire department hose threads. Final location of fire department hose connections shall be approved by Fire Marshal and coordinated with Architect.

7. Sprinkler Contractor to begin work at flanged stub 12" A.F.F. provided by site utility contractor in Fire Riser Room. Coordinate final size and stub location with site utility contractor.

8. Provide AMES Fire and Waterworks Series 1BR In-Building Riser, single piece, stainless steel, with all NFPA 24 and code required thrust blocking/rod anchoring, coordinate with soil conditions. When expansive soils are present provide 'EBAA Iron INC. "FLEX-TEND FORCE BALANCED FLEXIBLE EXPANSION JOINT". Size to match fire line size. If soil conditions are unknown at time of bid, include in base price and provide deductive alternate to be removed from scope when soil conditions are known. Installation to be by Fire Protection Contractor or Site Utility Contractor with 'SCRU' License. Installation to be per NFPA 24 "Standard for Installation of Private Fire Main.

C. Provide the following type fire protection in the areas noted:

1. Provide a wet and/or dry pipe sprinkler system (light hazard pipe schedule) to protect the entire building as required. Hazard type shall be as required by FM Global, NFPA-13, International Fire Code and as required by the Owner's Insuring Agency.

2. Provide a dry pipe system in any areas subject to freezing.

3. Remodel areas receiving new ceilings shall have new heads as defined in this specification and treated as new construction to match all additions. Existing pendant heads shall not be reused on new ceilings.

D. SPECIAL NOTE: All provisions and divisions of these specifications are a part of this section of these specifications. The Contractor shall consult these divisions and provisions in detail for instructions and include all items pertaining to this work. The Contractor shall consult all other divisions of these specifications, determine the extent of impact on the work required to complete the work required by this section of the specifications or portion thereof and related work shown on the drawings.

E. Provide floor sink in each sprinkler riser room.

F. Submit shop drawings for the fire sprinkler system to the local Fire Department for review prior to installation.

1.02 Provide all fire protection in accordance with the minimum provisions of the latest approved edition of the following codes and standards.

A. NFPA 13 - Latest approved edition, Sprinkler Systems.

B. Latest approved edition of the International Fire Code.

C. Latest approved local ordinances and amendments.

1.03 SUBMITTALS AND APPROVALS

A. Review and Approvals Required:

1. Obtain written review and approval of the entire fire protection system design and arrangement from the following authority:

a. State Fire Marshall (Approval)

b. Owner's Insuring Authority (Approval)

c. Architect (Approval)

d. Mechanical Engineer (Review)

2. Provide construction drawings, calculations, details and all other data required by the above authority for approval.

B. Submittals:

1. Provide complete fire protection systems construction drawings and calculations for the entire project.

2. Obtain and verify the high and low static water pressure and the residual pressure at full flow at the point of connection to the water utility systems or at a nearby point acceptable to the approval authority. Obtain data from flow tests or system network design calculations of reliability acceptable to the approval authority. Use this data in flow calculations and include it with submittal of calculations.

3. Include the following on the construction drawings:

a. Location of water source, routing and size of supply piping.

b. Location of key gate valves.

c. Detector check valves.

d. Fire department connections.

e. All necessary controlling equipment.

f. Location of flow alarm valves.

g. All distribution system piping and outlets.

h. Reflected ceiling plan showing proposed location of sprinkler heads and other outlets.

i. Interference control between work of other trades.

j. Indicate connection points for Fire Alarm Contractor and provide letter indicating coordination has been done with Fire Alarm Contractor.

k. Provide 1/4" shop drawing of sprinkler entrance and valve assemblies to ensure adequate space.

4. Submit six copies of fire protection drawings reviewed and approved by the Administrative Authority to the Architect for further approval. Include all correspondence with the Administrative Authority.

5. Obtain approval of Architect prior to ordering, fabricating or installing any part of the system. Head locations are subject to this review and approval.

1.04 COORDINATION REQUIRED

A. Check all dimensions indicated on the Architectural or structural drawings and verify dimensions at the site before fabricating any portion of the system. Any discrepancies in piping and head locations resulting from failure to so check shall be corrected expeditiously to provide proper coordination of all trades.

B. Coordinate work with that of other crafts to ensure that adequate space is provided for all work, including requirements for accessibility and serviceability. Locate sprinkler heads to avoid conflict with light fixtures and other installed equipment, and center location of piping and heads in field to accomplish these requirements for coordination.

C. Coordinate with Mechanical Shop Drawings to avoid ducts from RTU's and other main runs.

D. Do not install any piping over electric rooms, electric equipment, MDF/IDF/Technology rooms or racks. Only piping serving electric/technology rooms may enter electric rooms and should enter over doorway.

1.05 FINAL INSPECTION AND APPROVAL

A. After installation is complete, obtain inspection and letter of approval of system stating that sprinkler system complies with all requirements for a fully sprinklered building.

1.06 ACCEPTABLE SUPPLIERS

A. Furnish the services of a qualified and approved fire protection subcontractor to provide the work of this specification section. Unless otherwise noted, this is substantially a "performance" specification.

B. Subcontractor minimum qualifications include:

1. Maintain a complete engineering, sales, installation and service organization that has operated within the Austin/San Antonio area for at least three years prior to bid date of this project.

2. Maintain a complete stock of replacement parts.

3. Remain on 24-hour call for emergency service.

4. Demonstrate satisfactory completion of three projects of similar size and scope. Provide references if required.

5. Bids by wholesalers, Contractors or any firm whose principle business is not that of manufacturing and/or installing fire protection systems are not acceptable.

1.07 MAINTENANCE INFORMATION AND INSTRUCTION

A. Reference Section 20 00 00.

B. Include instruction charts describing operation and proper maintenance of fire protection devices.

C. Include publication entitled: “Care and Maintenance of Sprinkler Systems”, NFPA No. 13-latest approved edition.

PART 2 - PRODUCTS

2.01 General: Provide only products which are a standard product of a manufacturer regularly engaged in the fire protection application where they are used.

2.02 Labels and Approvals for Products: All products UL or FM listed, labeled and specifically approved for the fire protection application where they are used.

2.03 PIPE AND FITTINGS

A. Water Systems:

1. Piping system materials shall be currently recognized by NFPA 13 and must be listed for the intended service by UL or FM. Furnish steel pipe and fittings of domestic manufacturers only.

2.04 VALVES

A. All valves UL listed and labeled and specifically approved for the fire protection application where they are used. Minimum working pressure 175 psi non-shock cold water.

2.05 SPRINKLER HEADS; FINISHED AREAS

A. General: Heads of temperature rating required by NFPA 13, suitable for system type. **USE FLEXIBLE TYPE SPRINKLER HEAD CONNECTION ONLY.**

B. Concealed Quick Response Type: With coverplate and spring-loaded clips, color as approved by Architect. Reliable G4QR concealed with cover to be used in all ceilings unless otherwise noted.

C. Quick Response Dry Pendant Type: For use in freeze applications. Reliable Model G3FR.

D. Institutional Heads: For cell and other secure type areas. Flush style type to conceal operating parts except for sensor, Conical Escutcheon Assembly attached to sprinkler body with tamper resistant fasteners. Pendent or side wall mount as applicable. Use side wall in cell areas. Reliable Model ZX-OR-IWST.

E. Pendant Type: For the Gym and Maintenance Areas use Quick Response Type, Reliable Model F1FR. Use sprinkler guards on sprinkler heads in Gym Area.

F. Manufacturers: Chemetron, Reliable, Grinnell or approved equal.

2.06 SPRINKLER HEADS; UNFINISHED AREAS

A. General: Heads of temperature rating required by NFPA 13; plain finish suitable for system type.

B. Type: Fusible link, equal to Reliable Model A.

C. Manufacturers: Reliable, Grinnell, approved equal.

2.07 FIRE DEPARTMENT CONNECTION (FDC)

A. FDC: Cast brass body and trim having individual 2-1/2" double female snoot inlets with rigid end NPT with pin lug hose thread swivels, plugs, and chain; outlet size as required; exposed parts polished brass (chrome-plated); horizontal flush mounting; Potter Roemer 5020 Series or approved equal. Coordinate final requirements and locations of FDC with Fire Marshal.

B. Provide appropriate lettering on escutcheon plate, to identify connection.

C. Provide hose threads to match the threads of the local fire department.

D. Provide Knox Caps per Fire Department requirements.

2.08 SPRINKLER ALARM VALVE ASSEMBLY

A. Provide sprinkler alarm valve assemblies, appropriate to the system, complete with all trimmings and accessories for proper alarm initiation and interface with fire alarm system. Include inlet and discharge pressure gauges, main drain and inspectors test connection.

B. Alarm Gong: Provide a 10" diameter water powered alarm bell, confirm location with Architect.

2.09 WATER FLOW DETECTORS

A. Provide flow switches or water flow detectors as required. Provide switches compatible with fire alarm system.

B. Electrical connection by Electrical Contractor. Coordinate location and quantity with Electrical Contractor.

2.10 VALVE TAMPER SWITCHES

A. Furnish UL listed and FM approved, series NGV supervisory switch for OS & Y valve installation with two single pole, double throw microswitches. The mechanism shall be contained in a red baked enamel, weatherproof housing and shall incorporate the necessary facilities for attachment to valves. The switch mechanism shall be compatible with the fire alarm system. The entire installed assembly shall be tamperproof and arranged to cause switch operation if the housing cover is removed or if the unit is removed from its mounting. Install supervisory switches and furnish all wire and conduit required from supervisory switch to the alarm panel. Furnish a set of additional alarm contacts for additional supervisory capability.

B. Manufacturers: Potter Electric Signal Co., Model OSYS-B or approved equal.

C. Electrical connection by Electrical Contractor. Coordinate location and quantity with Electrical Contractor.

2.11 INSPECTORS TEST CONNECTIONS

A. Provide inspectors test connections for complete system testing and as required for final approval by inspecting authority.

B. Chemoplate all portions of pipe and fittings exposed outside building; provide chrome-plated set screw escutcheon.

2.12 DRY SYSTEM

A. Provide Nitrogen Generator System for all Dry Zones.

1. Basis of Design – South Tek (www.southteksystems.com)

a. FPS – xxx (sized for system)

b. Quick Check Purity Manifold (sized for system)

c. Provide with Auto Purge

d. Provide appropriately sized air compressor for system to have 30 min max charge time.

B. Provide UL approved low air pressure alarm switches on each dry system, compatible with fire alarm system.

C. Electrical connection is described in Division 26-28 work. Coordinate location and quantity with Electrical Contractor.

2.13 BACKFLOW PREVENTER

A. Provide UL and FM approved reduced pressure zone (RPZ) double check backflow flow preventer equal to Ames Silver Bullet style inside building at fire entry point. Provide shop drawing indicating space requirements. Pipe RPZ relief to nearest floor sink.

2.14 FIRE PUMP SYSTEMS

A. Furnish and install a complete fire pump system complete with pump, driver, controller, jockey pump accessories. The pumping unit shall be listed by Underwriter's Laboratories, Inc. and fully approved by the Associated Factory Mutual Fire Insurance Companies. The pumping unit shall meet all requirements of the National Fire Protection Associated Pamphlet No. 20. The fire pump shall be designed to deliver a required G.P.M. and pressure to meet NFPA flow requirements for building. Contractor to verify city pressures.

B. The driver shall be an open drip-proof (or T.E.F.C.), ball bearing type, AC, induction, squirrel cage motor, wound for 480 volts, 3 phase, 50 hertz. The motor shall be of such capacity that 115% of the full-load ampere rating shall not be exceeded at any condition of pump load for U.L. listed fire pump systems and 110% of full-load for F.M. approved fire pump systems. Locked rotor current shall not exceed the values specified in NFPA Pamphlet No. 20.

C. Pump and motor shall be mounted on a common baseplate of steel with drip rim. Pump and motor shall be checked for alignment after the pump base has been installed and grouted in place.

D. All pumps where the suction pressure is expected to average 40 P.S.I. or below, shall be

provided with lantern ring connected to the pressure side of the pump by a cored passage in the parting flange of the pump. Stuffing boxes shall be equipped with split bronze packing glands designed for easy removal for packing inspection and maintenance.

E. The fire pump service rated motor control shall be U.L. listed and F.M. approved. It shall be completely assembled, wired and tested by the control manufacturer before shipment from the factory, and shall be labeled “Fire Pump Controller”. Fire pump controller shall have over current protection to handle locked rotor amp of fire pump. The controller shall be located as close as practical and within sight of the motor. The controller shall be so located or protected that it will not be injured by water escaping from the pump or connections. The controller shall be of the combined manual and automatic across-the-line type, and shall be complete with disconnect switch – externally operable, quick break type, circuit breaker – time delay type with strips in all places set for 300% of the motor full-load current motor starter – across-the-line type capable of being energized automatically through the pressure switch or manually by means of an externally operable handle, pressure switch, running period timer – set to keep motor in operation, when started automatically, for testing as required by code. (Control equipment shall meet all requirements of NFPA No. 20.)

F. Provide ¼" scale shop drawing of fire pump room. Indicating fire pump, piping controller and jockey pump.

G. Manufacturers: Aurora, Suncroflo or equal.

H. Coordinate all final electrical requirements with Electrical Contractor.

I. Coordinate final room size with Architect.

J. Provide alternate to provide MIC Control System to aid in reducing degradation.

PART 3 - EXECUTION

3.01 Install all work in accordance with codes and recommended practices for this type of work.

3.02 Conceal all piping possible. Coordinate with the other trades to take timely advantage of available space above ceilings, in pipe and duct spaces, and elsewhere.

3.03 Provide access doors where “Fire Protection” valves, switches, or other controlling or monitoring devices are concealed. Label doors for quick location and recognition of concealed device.

3.04 Advise the Architect immediately of any conflicts between the plans and specifications. Clearly explain problem, limits of problem, and proposed solution.

3.05 Center sprinkler heads in ceiling tiles in both directions. Coordinate exact placement with diffusers, light fixtures and other ceiling mounted devices.

3.06 **Sprinkler heads shall not be painted.**

3.07 Provide as-built drawings indicating location of zones.

3.08 FLUSHING AND TESTING

A. Flush underground service piping and distribution piping before connecting underground

piping to sprinkler system.

B. Arrange for proper witnessing of all tests as required by codes and authorities.

C. Make all tests in accordance with applicable codes. Test piping at minimum 200 psig hydrostatic for two hours. Flush as required by NFPA.

D. Provide a letter of certification stating that all testing and flush has been performed in accordance with the applicable codes and standards. Itemize codes and standards complied with.

3.09 All sprinkler piping is to be routed in conditioned space to prevent freezing and concealed above ceiling, furr-outs or other Architectural features whenever possible. Coordinate changes in elevation, piping between floors and adjoining spaces with Architectural drawings to ensure that piping will not be exposed to finished spaces. **Any piping that must be exposed is to be coordinated with Architect and indicated on shop drawings.**

3.10 DRY SPRINKLER SYSTEM

A. Air Compressor: Confirm final electrical requirements with Electrical Contractor(min 20amp-120v dedicated circuit). Compressor must have adequate vibration isolation so that no vibrations are transmitted to the structure or slab.

B. Nitrogen Generator: Confirm final electrical requirements with Electrical Contractor(min 20amp-120v dedicated circuit). If large system coordinate 208volt connection.

C. Provide start-up and test report showing proper functioning of Nitrogen systems, Purity Manifold operation and monitoring, including final Nitrogen Content.

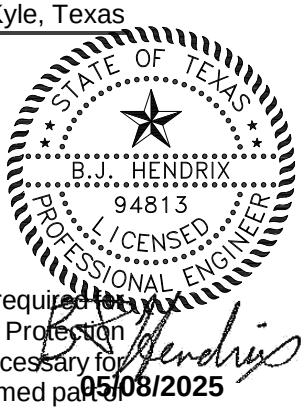
D. Provide all piping and controls associated with air compressors.

E. Arrange wiring for single point connection, for each piece of equipment.

3.11 REMODEL AREA

A. Provide new heads in ALL remodel areas to match new construction standards and aesthetics based on this specification.

END OF SECTION



SECTION 26 4113 - LIGHTNING PROTECTION SYSTEM

PART 1 - GENERAL

1.01 SCOPE OF WORK

A. The Contractor shall furnish and install all equipment, accessories, and material required for the installation of a complete Master labeled UL, AW, IRI approved Lightning Protection System in accordance with the specifications. Any material and/or equipment necessary for the proper operation of the system not specified or described herein shall be deemed part of this specification.

B. The equipment described and furnished under these specifications shall be the standard product of one manufacturer.

1.02 STANDARDS

A. Construct each item of equipment, including parts and accessories, in a workmanlike manner, using new materials or the best quality obtainable for the purpose intended. Design and build materials, wiring and equipment in accordance with the best practices of the electrical industry.

B. Furnish and install a complete Lightning Protection System which shall comply with the specifications of the Underwriters' Laboratories (UL 96A), the National Fire Protection Association (NFPA No. 78) and the Lightning Protection Institute (LPI-75). The Underwriter's Laboratories Master Label "C" shall be delivered to the Architect/Engineer for approval before installation is completed.

1.03 QUALIFICATIONS

A. Manufacturer: Company specializing in lightning protection equipment with minimum three years documented experience and member of the Lightning Protection Institute.

B. Installer: Authorized installer of manufacturer with three years documented experience and member of the Lightning Protection Institute.

1.04 SUBMITTALS

A. Furnish the Engineer Shop Drawings/Submittals for each of the following:

1. Submit shop drawings showing layout of air terminals, grounding electrodes, and bonding connections to structure and other metal objects. Include terminal, electrode, and conductor sizes, and connection and terminal details.

2. Submit product data showing dimensions and materials of each component and include indication of listing in accordance with ANSI/UL 96.

3. Complete manufacturer's installation documentation.

1.05 PROJECT RECORD DOCUMENTS

A. Accurately record actual locations of air terminals, grounding electrodes, bonding connections and routing of system conductors.

PART 2 – PRODUCTS

2.01 PRODUCTS

A. Components - Including but not limited to: (In accordance with ANSI/UL 96)

1. Air Terminals
2. Grounding Plate
3. Conductors
4. Connectors and Splicers

B. Materials shall be as required by code to suit installation requirement including materials of mounting surfaces.

PART 3 - EXECUTION

3.01 INSTALLATION

A. All metal bodies such as ventilators, stacks, pipes, gutters, downspouts, ducts, tracks, antennas, water pipes, ladders, exercise yard cover and other similar metal shall be interconnected to the main conductor system.

B. Incoming electric and telephone service shall have a common ground with the Lightning Protection System.

C. **Obtain the services of Underwriters Laboratories, Inc. to provide inspection and certification of the lightning protection system under provisions of UL 96A.**

D. No part of the system shall be concealed until inspected. In addition, the Underwriter's Laboratories Master Label "C" shall be delivered to the Architect or Owner before the completed installation is cleared for final payment. Any items found not to comply with the specification requirements shall be immediately replaced at no additional cost to the Owner.

E. The system shall be installed by an authorized installer.

F. All materials shall be copper, except where specifically prohibited by UL96A due to materials of mounting surfaces which may cause deterioration in the presence of moisture. At these locations use appropriate material.

END OF SECTION

SECTION 31 0010
EARTHWORK AND GRADING
(Sports Facility Work)

PART 1 - GENERAL

1.01 CONSIDERATIONS:

A. Earthwork consists of operations required for the excavation and/or fill of subgrade materials; stock piling materials; scarifying and compaction of sub-grades; finish grading; and other required operations

1.02 REFERENCES:

A. ASTM D 2487--Classification of Soils for Engineering Purposes.
B. ASTM D 698--Compaction Characteristics of Soil Using Standard Effort.
C. Geotechnical Exploration Report by UES, dated April 10, 2025

1

1.03 DEFINITIONS:

A. Classification: Earthwork materials are classified in accordance with definitions in this Article.
B. Subgrade: Natural soil at the established lines and grades.
C. Earthen Fill: Suitable, clean material excavated on-site or imported borrow material meeting specified characteristics.
D. Finish Grading: Operations required for smoothing disturbed areas that are not overlaid with pavement.
E. Excavation: Excavation of every description and of whatever substances encountered within the limits of the project to the lines and grades indicated.
F. Compaction: Compaction of soil materials shall be measured as a percent of Standard Proctor density as determined by ASTM D698.

1

1.04 EXISTING UTILITIES:

A. Where pipes, ducts and structures are encountered in the excavation but are not shown or specified on the drawings to be abandoned, immediately notify the Engineer.

PART 2 - PRODUCTS

2.01 EARTHEN FILL:

1

A. Select fill shall be comprised of material which is free of all organic materials and meets the requirements of the Geotech Report. Unsuitable materials shall be classified as:

1 Topsoil; frozen materials; material containing more than 0.5 percent fibrous organic material by weight; construction materials and materials subject to decomposition; clods of clay and stones larger than 75 mm (3 inches); organic material, including silts, which are unstable; and inorganic materials, including silts, too wet to be stable and any material with a liquid limit greater than 35 and a plasticity index outside the range of 5-15. Unsatisfactory soils also include satisfactory soils not maintained within -1 and +3 percent of optimum moisture content at time of compaction. 1

2.02 SOURCE QUALITY CONTROL:

A. Provide materials from same source throughout the project. Imported soils shall be clean non-expansive soils having a liquid limit less than 35 and a PI between 5 and 15. Soils shall be maintained at -1 and +3 percent of optimum moisture content at time of compaction. 1

B. A change in source requires sampling, testing, and approval by Engineer.

PART 3 - EXECUTION

3.01 SITE PREPARATION:

A. The project site shall be stripped of all vegetation and shall be rough graded and otherwise prepared, as detailed on the drawings. Additionally, the contractor shall clean subgrade soils of any organic material or trash encountered during excavation so as to maintain clean earthen materials.

3.02 TREATMENT OF SUBGRADES:

1 A. All site preparation and grading shall be performed in accordance with the Geotech Report and construction drawings.

1 B. In cut areas, over excavate and remove soils to a depth of 10-feet below finished elevations and grades indicated on the drawings. Excess clean soils shall be stockpiled in the locations designated by the Owner, if Owner agrees to utilize such, or shall otherwise be properly disposed of.

1 C. After completion of cut, the exposed subgrade shall be proof rolled with a heavy pneumatic tired roller or approved equivalent to check for pockets of soft or loose material. Proof rolling shall be observed routinely by a Profession Engineer.

1 D. Upon completion of proof rolling, subgrade shall be scarified to a minimum depth of 6-inches and thereafter compacted to a minimum density of 95 percent of maximum density per ASTM D 698 at a moisture content of between -1 and +4 percent of optimum. Subgrade surface shall be proof roll tested in the presence of the Geotechnical Engineer prior to placing new layers.

E. Examination of Subgrade: Do not place materials on prepared subgrade until the subgrade preparation has been accepted by the Engineer. Do not place fill over frozen or saturated ground.

3.03 PLACING FILL:

A. Place select fill to a depth of 10-feet in accordance with the Geotech Report. Select fill shall be placed in loose lifts no greater than 8-inches.

B. In fill areas, after placement of fill, compact material to a minimum density of 95 percent of maximum density per ASTM D 698 at a moisture content of between -1 and +3 percent of optimum.

C. Attaining Proper Bond: If the compacted surface of a layer is too smooth to bond with succeeding layers, loosen the surface by means of scarifying to a minimum depth of 3-inches before placing fill and continuing the work.

D. Place materials to lines and grades shown allowing for depth of base and concrete/asphalt. Material depths to follow depths as outlined in the geotechnical report.

E. Maintain aggregate drainage throughout construction.

F. The material shall be blended sufficiently to secure the best degree of compaction.

3.05 FINAL GRADING:

A. Upon completion of the excavation, grading and compaction process, fine grade all surfaced by means of laser grading (or grade using equivalent means) as needed to meet the meet the elevations, lines and grades indicated on the drawings.

3.06 TESTING:

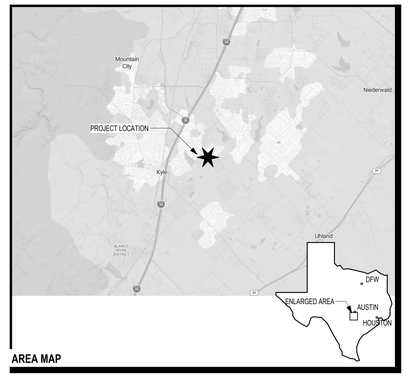
A. Compaction Testing: Conduct compaction testing for subgrade soils. Minimum spacing for compaction testing shall be one test per each ten thousand square feet of area compacted. Material shall be compacted to a density of 95 percent of maximum density per ASTM 698 at a moisture content of -1 and +3 percent of optimum. Areas of the field found not to meet compaction criteria shall be re-worked and/or re-compacted at the Contractors expense until compaction criteria are met. Contractor shall also be responsible for the costs of additional compaction testing.

B. Grading Test: For synthetic turf field area, conduct in the presence of the Engineer, a string line test on the final grades of the field prior to installation of geotextile fabric. String line test shall be conducted by pulling a string along the direction of the grade to verify positive drainage. Strings shall be pulled at a minimum interval of 15 feet. Acceptable tolerance for this test shall be 0.06 foot. Areas found to be outside of stated tolerances shall be re-graded at the Contractor's expense and re-tested until entire field meets grading tolerance.

END OF SECTION

1 ALTERNATE #1 – CONSTRUCTION OF MPAC BUILDING
ALTERNATE #2 – ADD CARD READERS AT CLASSROOMS/WORKROOMS (INFRASTRUCTURE IN BASE BID)
ALTERNATE #3 – NORTH PARKING LOT
REF SPECIFICATION AND REMAINDER OF CONSTRUCTION DOCUMENTS FOR ADDITIONAL INFORMATION.

ALTERNATES



AREA MAP

VICINITY MAP

HAYS COUNTY, TEXAS

AB	ANCHOR BOLT
AC	AIR CONDITIONER
ACC	ACCESSIBLE
ACMU	ARCHITECTURAL CONCRETE MASONRY UNIT
ADA	AMERICANS WITH DISABILITIES ACT / ADA STANDARDS FOR ACCESSIBLE DESIGN (LATEST VERSION)
ADDN'L	ADDITIONAL
ADJ	ADJUSTABLE
ADH	ADHESIVE/ADHERE
ADMIN	ADMINISTRATION
AFF	ABOVE FINISHED FLOOR
AFG	ABOVE FINISHED GRADE
AG	AGRICULTURE
AHU	AIR HANDLING UNIT
AL	ALUMINUM
ALT	ALTERNATE
ALUM	ALUMINUM
AP	ACOUSTICAL PANELS
AP	ASSISTANT PRINCIPAL
APPROX	APPROXIMATE
ARCH	ARCHITECTURAL
B	BEYOND
B/B	BACK TO BACK
BD	BOARD
BLDG	BUILDING
BLKG	BLOCKING
BOD	BOTTOM OF DECK
BOM	BOTTOM OF MASONRY
BOS	BOTTOM OF STEEL
BOT	BOTTOM
BP	BASE, PORCELAIN TILE
BR	BASE, RUBBER
BRES	BASE, RESINOUS INTEGRAL
BRG	BEARING
BRK	BRICK
BRV	BASE, RUBBER VENTED
BTC	BASE, TILE CERAMIC
BTZP	BASE, TERRAZZO PRECAST
BS	BOTH SIDES
BTWN	BETWEEN
BW	BOTH WAYS
BWD	BASE, WOOD
C/C	CENTER TO CENTER
CAB	CABINET
CCD	COILING COUNTER DOOR
CCPS	CERAMIC CLADDING PANEL SYSTEM
CDW	CUSTOM DIGITAL WALLCOVERINGS
CDAP	CUSTOM DIGITALLY PRINTED ACOUSTIC PANEL
CFMF	COLD-FORMED METAL FRAMING
CH	CHANNEL
CI	CONTINUOUS INSULATION
CJ	CONTROL JOINT
CL	CENTERLINE
CLG	CEILING
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
CNTRD	CENTERED
CO	CLEAN OUT/CASED OPENING
COL	COLUMN
CONC	CONCRETE
CONF	CONFIGURATIONS / CONFERENCE
CONN(S)	CONNECTION(S)
CONT	CONTINUOUS
COORD	COORDINATE
COP	CONCRETE (POLISHED)
CORR	CORRIDOR
COS	CONCRETE SEALED
CPT	CARPET, SHEET
CPY	CANOPY
CR	CLASSROOM
CS	CAST STONE
CSM	CONCRETE STONE MASONRY
CU	CONDENSING UNIT
CUST	CUSTODIAN
CT	CARPET, TILED
CTE	CAREER TRAINING ED
CTRL	CONTROL
CW	CURTAIN WALL
CWF	CUSTOM WINDOW FILM
DBL	DOUBLE
DET	DETAIL
DH	DOOR HARDWARE
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DN	DOWN
DS	DOWNSPOUT
DTL	DETAIL
DWG	DRAWING
DWR	DRAWER
EA	EACH
EBWS	EPOXY BASED WALL SYSTEM
ED	EDUCATION
EF	EACH FACE
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
ENGR	ENGINEER
EPNT	EPOXY PAINT
EQ	EQUAL (EQUALLY)
EQUIP	EQUIPMENT
EW	EACH WAY
EWC	ELECTRIC WATER COOLER
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
EXTNG	EXTINGUISHER
F/F	FACE TO FACE
FC	FURRING CHANNEL
FD	FLOOR DRAIN
FDC	FIRE DEPARTMENT CONNECTION
FDN	FOUNDATION
FE	FIRE EXTINGUISHER (SURFACE MOUNT)
FEC	FIRE EXTINGUISHER CABINET (SEMI-RECESSED)
FFE	FINISHED FLOOR ELEVATION
FHC	FIRE HOSE CABINET (SEMI-RECESSED)
FIN	FINISH(ED)
FIN FLR	FINISHED FLOOR
FL	FLUSH (FLUSHED)
FLR	FLOOR
FR	FRAME
FRP	FIBERGLASS REINFORCED PLASTIC PANEL SYSTEM / FIBERGLASS REINFORCED PANELING
FRT	FIRE RETARDANT TREATED
FRTW	FIRE RETARDANT TREATED WOOD
FTG	FOOTING
FV	FIELD VERIFY
GA	GAGE OR GAUGE
GALV	GALVANIZED
GB	GRAB BAR / GRADE BEAM
GBN	GENERAL
GFRC	GLASS FIBER REINFORCED CONCRETE
GRF	GROUND FACE CMU (BURNISHED)
GUT	GUTTER
GYM	GYMNASIUM
GYP	GYPSUM
HB	HOSE BIB
HDWD	HARD WOOD
HM	HOLLOW METAL
HO	HOLD OPEN
HORIZ	HORIZONTAL
HSS	HOLLOW STRUCTURAL SECTION
HT	HEIGHT
HVAC	HEATING, VENTILATION, & A/C
ICF	INSULATED CONCRETE FORMS / INSULATED CONCRETE FORMING
IDF	INTERMEDIATE DISTRIBUTION FRAME
INFO	INFORMATION
INSUL	INSULATION
INT	INTERIOR
JT	JOINT
JST(S)	JOIST(S)
L	ANGLE
LAB	LABORATORY
LAV	LAVATORY
LEPC	LOCAL EMERGENCY PLANNING COMMITTEE
LONG	LONGITUDINAL
LVT	LUXURY VINYL TILE
MAS	MASONRY
MATL	MATERIAL
MAX	MAXIMUM
MB	MARKER BOARD
MCM	METAL COMPOSITE MATERIAL
MDF	MAIN DISTRIBUTION FRAME
MECH	MECHANICAL
MEZZ	MEZZANINE
MFG	MANUFACTURER
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MP	METAL WALL PANELS
MPF	MULTI-PURPOSE FACILITY
MSK	MOP SINK
MS	METAL SOFFIT
MSL	MEAN SEA LEVEL
MTL	METAL
NB	NO BASE
NF	NO FINISH
NIC	NOT IN CONTRACT
NO	NUMBER
NS	NATURAL STONE
NTS	NOT TO SCALE
OC	ON CENTER
OCC	OCCUPANTS / OCCUPANCY
OCEW	ON CENTER EACH WAY
OCD	OVERHEAD COILING DOOR
OCG	OVERHEAD COILING GRILLE
OD	OUTSIDE DIAMETER
OF	OUTSIDE FACE
OFCI	OWNER FURNISHED CONTRACTOR INSTALLED
OFF	OFFICE
OFOI	OWNER FURNISHED OWNER INSTALLED
OH	OPPOSITE HAND
OHD	OVERHEAD
OPP	OPPOSITE
OS	OVERFLOW SCUPPER
P/C	PRECAST CONCRETE
PEMB	PRE-ENGINEERED METAL BUILDING
PERIM	PERIMETER
PERP	PERPENDICULAR
PLA	PLATE / PLASTER
PL	PLASTIC LAMINATE
PLP	PLASTIC LAMINATE PANEL SYSTEM
PNT	PAINT
PR	PRACTICE ROOM
PREFIN	PREFINISHED
PREP	PREPARATION
PROJ	PROJECTION
PS	PROJECTION SCREEN
PT	POINT
PW	PLYWOOD
QTZ	QUARTZ SURFACING
R	RISER
RAD	RADIUS
RAFF	RESILIENT ATHLETIC FLUID FLOORING
RAFP	RESILIENT ATHLETIC FLOORING POLYURETHANE
RAFS	RUBBER FLOORING SHEET
RAFV	RESILIENT ATHLETIC FLOORING VINYL
L	ANGLE
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING
RES	RESINOUS
REQ'D	REQUIRED
RM	ROOM
RO	ROUGH OPENING
RR	RESTROOM
RT	RUBBER TILE
RTR	RUBBER TREADS AND RISERS
RTU	ROOFTOP UNIT
SB	SPLASH BLOCK
SCHED	SCHEDULE
SEC	SECURITY
SECT	SECTION
SF	STOREFRONT / SQUARE FEET
SHT	SHEET
SIM	SIMILAR
SL	SLOPE
SP	SPLASH PAN (METAL)
SPEC	SPECIFICATIONS
SPF	SPLIT-FACE CONCRETE MASONRY UNIT
SPS	SOLID POLYMER SURFACE
SS	STAINLESS STEEL
SSM	SOLID SURFACE MATERIAL
STD	STANDARD
STL	STEEL
STN	STAIN
STOR	STORAGE
STRUCT	STRUCTURE
SVT	SOLID VINYL TILE
SYMM	SYMMETRICAL
T&B	TOP & BOTTOM
T&G	TONGUE & GROOVE
TAS	TEXAS ACCESSIBILITY STANDARDS (LATEST VERSION)
TB	TACK BOARD
TC	TILE, CERAMIC
TH	THRESHOLD
TLT	TOILET
TOB	TOP OF BEAM
TOC	TOP OF CURB
TOJ	TOP OF JOIST
TOM	TOP OF MASONRY
TOS	TOP OF STEEL
TOSF	TOP OF METAL STUD FRAMING
TOW	TOP OF WALL
TPC	TILE, PORCELAIN
TQ	TILE, QUARRY
TR	TREATED
TRAN	TRANSVERSE / TRANSPARENT
TRIM	METAL FLASHINGS AND COPINGS
TS	TACKABLE SURFACE
TYP	TYPICAL
TZ	TERRAZZO
TZP	TERRAZZO, PRECAST STAIR TREADS AND RISERS
TZT	TERRAZZO TILE
U	URINAL
UL	UNDERWRITERS LABORATORIES
UNO	UNLESS NOTED OTHERWISE
UP	UPHOLSTERY
VCT	VINYL COMPOSITION TILE
VERT	VERTICAL
VEST	VESTIBULE
VSF	VINYL SHEET FLOORING
W/	WITH
WIN	WITHIN
W/O	WITHOUT
WASH	WASH STATION
WC	WATER CLOSET
WD	WOOD
WH	WATER HEATER / WALL HYDRANT
WMP	WOOD (MAPLE) STRIP AND PLANK FLOORING
WMS	MASONITE WOOD FLOORING
WOM	WALK-OFF MAT
WOT	WALK-OFF TILE
WP	WORKPOINT
WT	WEIGHT
WWF	WELDED WIRE MESH (FABRIC)
XB	X-BRACING
>=	LARGER THAN OR EQUAL TO
<=	LESS THAN OR EQUAL TO

(NOT ALL ABBREVIATIONS MAY BE USED)

ABBREVIATIONS

INDEX OF SHEETS

Revision /	Date
1	05/08/25 ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

05.08.25

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GENERAL DATA

PACKAGE VOLUME

Job No. 01954-08-01	Sheet No. ISSUE FOR BID G1.02
Drawn By: VSS	
Date: 05/08/2025	

5/8/2025 2:14:31 PM
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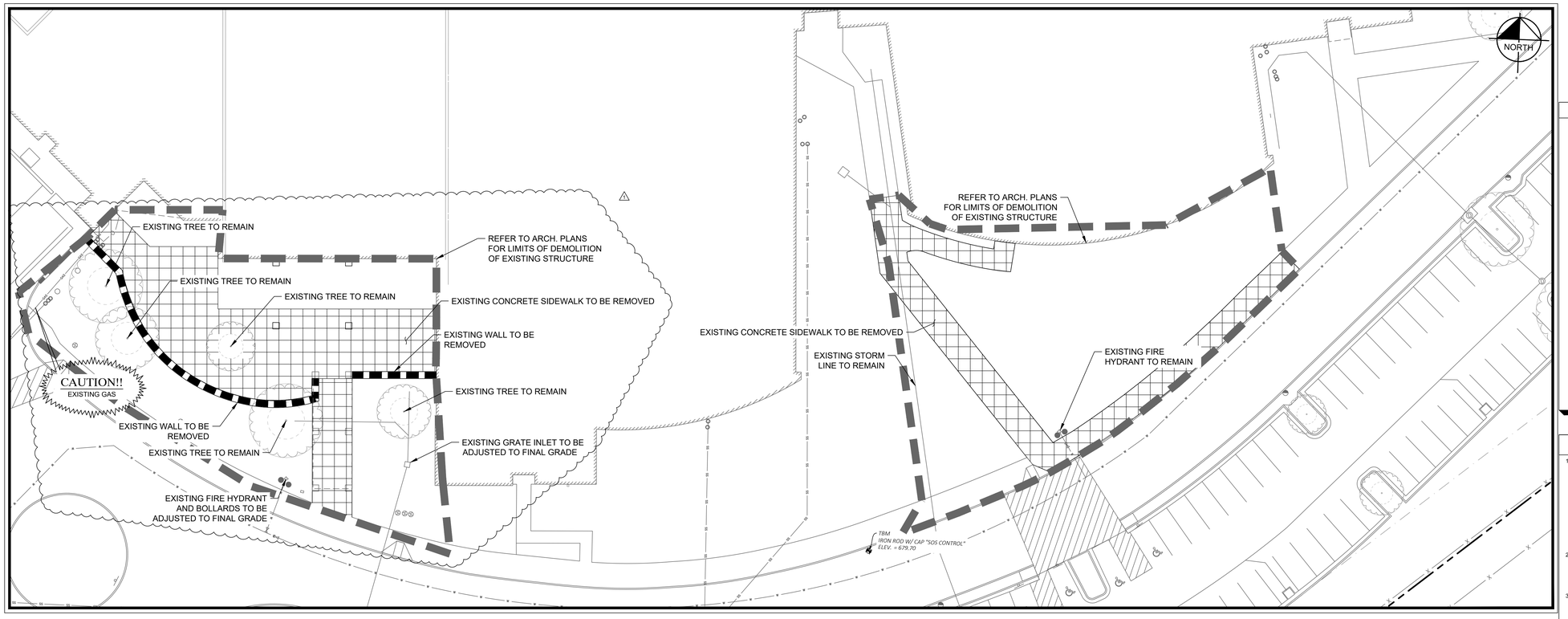


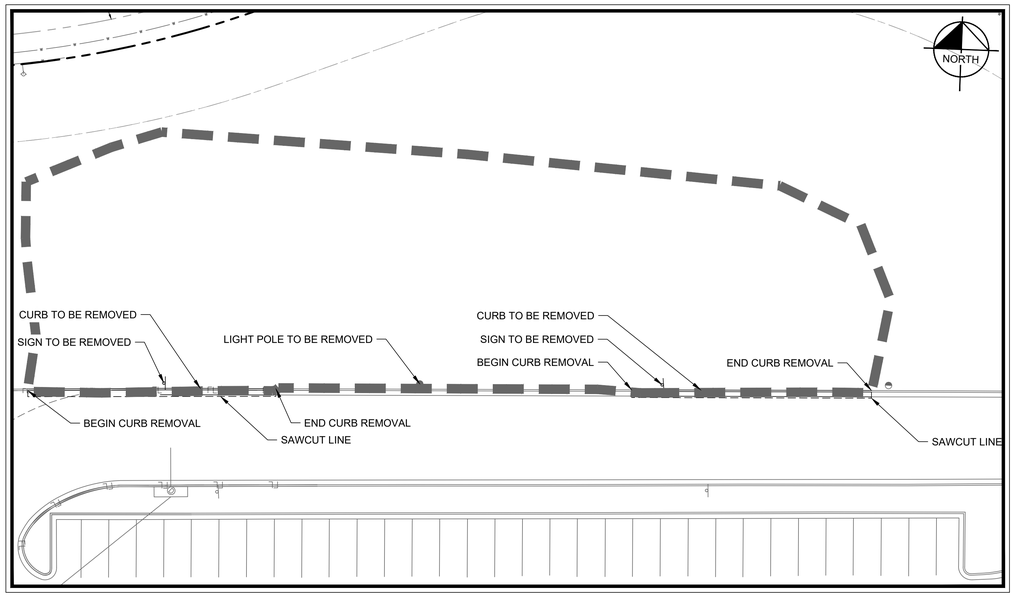
GRAPHIC SCALE IN FEET
0 10 20 40

LEGEND

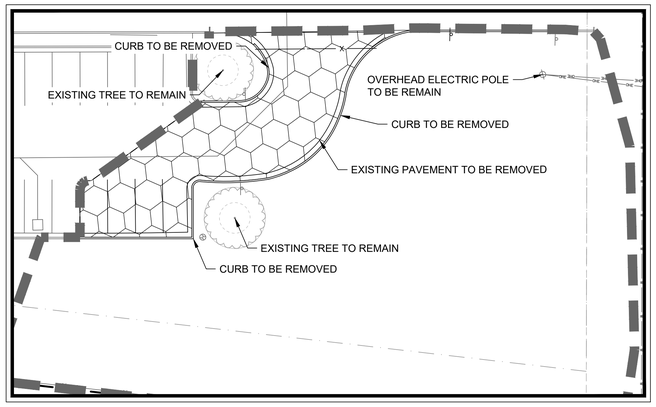
- PROPERTY BOUNDARY
- PROPOSED LIMITS OF DISTURBANCE (APPROXIMATE)
- EXISTING EDGE OF ASPHALT
- OHE — EXISTING OVERHEAD ELECTRIC TO REMAIN
- SS — EXISTING SANITARY LINE TO REMAIN
- W — EXISTING WATER LINE TO REMAIN
- GAS — EXISTING GAS LINE TO REMAIN
- W — WATER LINE TO BE REMOVED
- GAS — GAS LINE TO BE REMOVED
- EXISTING CONCRETE SIDEWALK TO BE REMOVED
- EXISTING CONCRETE PAVEMENT TO BE REMOVED
- EXISTING CONCRETE PAVEMENT TO BE REMOVED
- EXISTING DIRT ROAD TO BE REMOVED
- BENCHMARK
- PROPERTY CORNER
- EXISTING SIGN
- EXISTING SANITARY SEWER MANHOLE
- EXISTING WATER VALVE
- EXISTING FIRE HYDRANT
- EXISTING GUY WIRE
- EXISTING POWER POLE

DEMOLITION NOTES

1. THE CONTRACTOR SHALL COORDINATE WITH THE CITY OF KYLE AND FRANCHISED UTILITY COMPANIES TO MAINTAIN SERVICES AT ALL TIMES TO NEIGHBORING PROPERTIES. THE CONTRACTOR SHALL MAINTAIN COMPLETE RECORDS INDICATING HOW THE WASTE FROM THE SITE HAS BEEN HANDLED. ALL FACILITIES TO BE REMOVED SHALL BE UNDERCUT TO SUITABLE MATERIAL AND BROUGHT TO GRADE WITH SUITABLE COMPACTED FILL MATERIAL PER THE SPECIFICATIONS IN THE GEOTECHNICAL REPORT. THE SITE, AFTER DEMOLITION SHALL BE GRADED TO ELIMINATE DEPRESSIONS, HOLES, BERMS, DIRT PILES, ETC. THE SITE IS TO BE GRADED UNTIL RELATIVELY SMOOTH AND ATTRACTIVE IN APPEARANCE PRIOR TO STABILIZATION OF EARTH. ANY FILL MATERIAL/FILL AREAS SHALL BE COMPACTED TO 95% OF STANDARD PROCTOR DENSITY AT A MOISTURE AT, OR ABOVE, OPTIMUM MOISTURE CONTENT IN MAXIMUM 8" LIFTS. CONTRACTOR SHALL PROVIDE PROOF IN THE FORM OF LAB TEST KITS THAT THIS HAS BEEN ACHIEVED.
2. THE CONTRACTOR IS RESPONSIBLE FOR REMOVING ALL DEBRIS FROM THE SITE AND DISPOSING THE DEBRIS IN A LAWFUL MANNER. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED FOR DEMOLITION AND DISPOSAL. CONTRACTOR SHALL BE RESPONSIBLE FOR REVIEWING THE PHASE I ENVIRONMENTAL SITE ASSESMENT.
3. LOCATIONS OF PUBLIC AND PRIVATE UTILITIES SHOWN ARE APPROXIMATE AND MAY NOT BE COMPLETE. CONTRACTOR SHALL CALL 811 AT LEAST 48 HOURS PRIOR TO COMMENCING DEMOLITION OR CONSTRUCTION ACTIVITIES. CONTRACTOR SHALL CONTACT ANY OTHER UTILITY COMPANIES WHO DO NOT SUBSCRIBE TO THE TESS PROGRAM FOR LINE MARKINGS. THE CONTRACTOR BEARS SOLE RESPONSIBILITY FOR VERIFYING LOCATIONS OF EXISTING UTILITIES, SHOWN OR NOT SHOWN, AND FOR ANY DAMAGE DONE TO THESE FACILITIES.
4. ALL EXISTING UTILITIES SHOWN ARE LOCATED ACCORDING TO THE INFORMATION AVAILABLE TO THE ENGINEER AT THE TIME THE DRAWINGS WERE PREPARED AND HAVE NOT BEEN INDEPENDENTLY VERIFIED BY THE OWNER OR THE ENGINEER. GUARANTEE IS NOT MADE THAT ALL EXISTING UNDERGROUND UTILITIES ARE SHOWN OR THAT THE LOCATION OF THOSE SHOWN ARE ACCURATE. FINDING THE ACTUAL LOCATION OF ANY EXISTING UTILITIES IS THE CONTRACTORS RESPONSIBILITY AND SHALL BE DONE BEFORE THEY COMMENCE ANY WORK, IN THE VICINITY. FURTHERMORE, THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ANY AND ALL DAMAGE DUE TO THE CONTRACTORS FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES. THE OWNER OR ENGINEER WILL ASSUME NO LIABILITY FOR ANY DAMAGES SUSTAINED OR COST INCURRED BECAUSE OF THE OPERATIONS IN THE VICINITY OF EXISTING UTILITIES OR STRUCTURES, NOR FOR TEMPORARY BRACING AND SHORING OF SAME. IF IT IS NECESSARY TO SHORE, BRACE, SWING OR RELOCATE A UTILITY, THE UTILITY COMPANY OR DEPARTMENT AFFECTED SHALL BE CONTACTED BY THE CONTRACTOR AND THEIR PERMISSION OBTAINED REGARDING THE METHOD TO USE FOR SUCH WORK.
5. IT IS THE CONTRACTORS RESPONSIBILITY TO CONTACT THE VARIOUS UTILITY COMPANIES WHICH MAY HAVE BURIED OR AERIAL UTILITIES WITHIN OR NEAR THE CONSTRUCTION AREA BEFORE COMMENCING WORK. THE CONTRACTOR SHALL PROVIDE 72 HOURS MINIMUM NOTICE TO ALL UTILITY COMPANIES PRIOR TO BEGINNING CONSTRUCTION.
6. THE CONTRACTOR SHALL HAVE AVAILABLE AT THE JOB SITE AT ALL TIMES ONE COPY OF THE CONTRACT DOCUMENTS INCLUDING PLANS, SPECIFICATIONS AND SPECIAL CONDITIONS, COPIES OF ANY REQUIRED CONSTRUCTION PERMITS, AND EROSION CONTROL PLANS.
7. ANY DISCREPANCIES ON THE DRAWINGS SHALL BE IMMEDIATELY BROUGHT TO THE ATTENTION OF THE OWNER BEFORE COMMENCING WORK. NO FIELD CHANGES OR DEVIATIONS FROM DESIGN ARE TO BE MADE WITHOUT PRIOR APPROVAL OF THE OWNER AND NOTIFICATION TO THE ENGINEER. NO CONSIDERATION WILL BE GIVEN TO CHANGE ORDERS FOR WHICH THE OWNER WAS NOT CONTACTED PRIOR TO CONSTRUCTION OF THE AFFECTED ITEM.
8. CONTRACTOR IS RESPONSIBLE FOR TRAFFIC CONTROL DEVICES FOR ANY STREET WORK.
9. THE CONTRACTOR SHALL COORDINATE WITH RESPECTIVE UTILITY COMPANIES PRIOR TO THE REMOVAL AND/OR RELOCATION OF UTILITIES. THE CONTRACTOR SHALL COORDINATE WITH THE UTILITY COMPANY CONCERNING PORTIONS OF WORK WHICH MAY BE PERFORMED BY THE UTILITY COMPANY'S FORCES AND ANY FEES WHICH ARE TO BE PAID TO THE UTILITY COMPANY FOR THEIR SERVICES. THE DEVELOPER IS RESPONSIBLE FOR PAYING ALL FEES AND CHARGES.
10. CONTRACTOR MUST PROTECT THE PUBLIC AT ALL TIMES WITH FENCING, BARRICADES, ENCLOSURES, ETC., ACCORDING TO STANDARD BEST PRACTICES.
11. PRIOR TO DEMOLITION OCCURRING, ALL EROSION CONTROL DEVICES AROUND THE SITE PERIMETER ARE TO BE INSTALLED.
12. DAMAGE TO ALL EXISTING CONDITIONS TO REMAIN WILL BE REPLACED AT CONTRACTOR'S EXPENSE.
13. CONTRACTOR SHALL COMPLY TO THE FULLEST EXTENT WITH ALL REGULATIONS GOVERNING THE DEMOLITION, REMOVAL, TRANSPORTATION AND DISPOSAL OF ALL DEMOLITION DEBRIS.
14. CONTRACTOR SHALL COMPLY TO THE FULLEST EXTENT WITH THE LATEST OSHA STANDARDS FOR EXCAVATION AND TRENCHING PROCEDURES. CONTRACTOR SHALL USE SUPPORT SYSTEMS, SLOPING, BENCHING, ETC. AS NECESSARY FOR THESE OPERATIONS, AND SHALL COMPLY WITH ALL OSHA PERFORMANCE CRITERIA.
15. ANY RECYCLED MATERIAL TO BE STOCKPILED ON THE SITE SHALL BE STORED IN AS SMALL AN AREA AS PRACTICABLE AND THE LOCATION OF ANY STOCKPILE SHALL BE WELL CLEAR OF THE BUILDING PAD AREA AND THE LOCATION MUST BE PRE-APPROVED BY THE OWNER PRIOR TO STOCKPILING.
16. FILL MATERIAL SHALL BE PLACED IN ACCORDANCE WITH THE GEOTECH REPORT.



INSET B



INSET C

ADVISORY NOTES

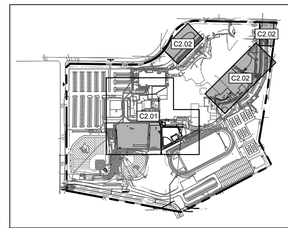
1. KIMLEY-HORN AND ASSOCIATES, INC. IS NOT RESPONSIBLE FOR THE MEANS AND METHODS EMPLOYED BY THE CONTRACTOR TO IMPLEMENT THIS DEMOLITION PLAN. THIS DEMOLITION PLAN SIMPLY INDICATES THE KNOWN OBJECTS ON THE SUBJECT TRACTS THAT ARE TO BE DEMOLISHED AND REMOVED FROM THE SITE. KIMLEY-HORN AND ASSOCIATES, INC. DOES NOT WARRANT OR REPRESENT THAT THE PLAN, WHICH WAS PREPARED BASED ON SURVEY AND UTILITY INFORMATION PROVIDED BY OTHERS, SHOWS ALL IMPROVEMENTS AND UTILITIES, THAT THE IMPROVEMENTS AND UTILITIES ARE SHOWN ACCURATELY, OR THAT THE UTILITIES SHOWN CAN BE REMOVED. THE CONTRACTOR IS RESPONSIBLE FOR PERFORMING HIS OWN SITE RECONNAISSANCE TO SCOPE HIS WORK AND TO CONFIRM WITH THE OWNERS OF IMPROVEMENTS AND UTILITIES THE ABILITY AND PROCESS FOR THE REMOVAL OF THEIR FACILITIES. THIS PLAN IS INTENDED TO GIVE A GENERAL GUIDE TO THE CONTRACTOR, NOTHING MORE. THE GOAL OF THE DEMOLITION IS TO LEAVE THE SITE IN A STATE SUITABLE FOR THE CONSTRUCTION OF THE PROPOSED DEVELOPMENT. REMOVAL OR PRESERVATION OF IMPROVEMENTS, UTILITIES, ETC. TO ACCOMPLISH THIS GOAL ARE THE RESPONSIBILITY OF THE CONTRACTOR.
2. THE CONTRACTOR IS STRONGLY CAUTIONED TO REVIEW ANY AVAILABLE REPORTS DESCRIBING SITE CONDITIONS PRIOR TO BIDDING AND IMPLEMENTING THE DEMOLITION PLAN.
3. CONTRACTOR SHALL COMPLY WITH ALL LOCAL, STATE, AND FEDERAL REGULATIONS REGARDING THE DEMOLITION OF OBJECTS ON THE SITE AND THE DISPOSAL OF THE DEMOLISHED MATERIALS OFF-SITE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO REVIEW THE SITE, DETERMINE THE APPLICABLE REGULATIONS, RECEIVE THE REQUIRED PERMITS AND AUTHORIZATIONS, AND COMPLY.
4. KIMLEY-HORN AND ASSOCIATES, INC. DOES NOT WARRANT OR REPRESENT THAT THE REPORTS AND SURVEYS REFERENCED ABOVE ARE ACCURATE, COMPLETE, OR COMPREHENSIVE.

BENCHMARK LIST

LCRA BENCHMARK PID NUMBER A490
NAVD 88 DATUM
ELEVATION: 633.05'

811 Know what's below. Call before you dig.

CAUTION!!
EXISTING UNDERGROUND UTILITIES IN THE AREA CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE HORIZONTAL AND VERTICAL LOCATION OF ALL UTILITIES PRIOR TO CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY REPAIRS TO EXISTING UTILITIES DUE TO DAMAGE INCURRED DURING CONSTRUCTION. CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES ON THE PLANS.



KEY MAP
N.T.S.

REVISION	DATE	ADDENDUM #1
△	05.05.25	

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS C.I.S.D.
KYLE, TEXAS

Kimley»Horn
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TBPE FIRM NO. 928

STATE OF TEXAS
RICHARD J. UNDERWOOD
117843
REGISTERED PROFESSIONAL ENGINEER
05/08/2025

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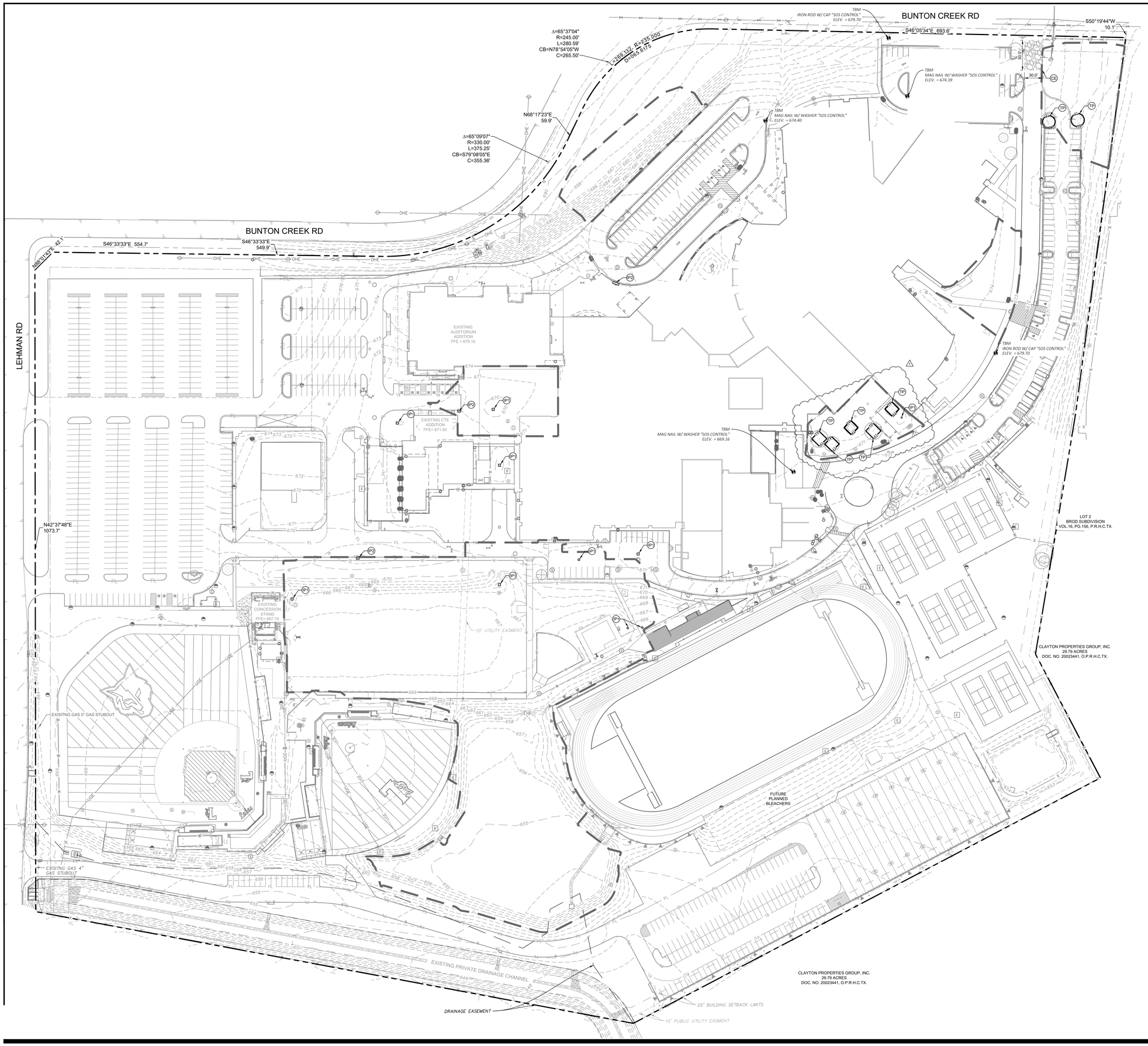
DEMOLITION PLAN (2 OF 2)

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID
Drawn By: RAU	C2.02
Date: 05/08/2025	



NORTH

GRAPHIC SCALE IN FEET
0 30 60 120

LEGEND

- PROPERTY BOUNDARY
- 670 EXISTING CONTOUR
- 670 PROPOSED CONTOUR
- PROPOSED LIMITS OF DISTURBANCE (APPROXIMATE)
- DRAINAGE BOUNDARY
- PROPOSED RETAINING WALL (TRIANGLE INDICATES FACE OF WALL)
- DIRECTION OF OVERLAND FLOW W/GRADE
- SF (SF) SILT FENCE (SEE DETAIL SHEET C3.02)
- TP (TP) TREE PROTECTION (SEE DETAIL SHEET C3.02)
- (CE) CONSTRUCTION EXIT (SEE DETAIL SHEET C3.02)
- (PS) PERMANENT STABILIZATION
- (RF) ROCK FILTER DAM (SEE DETAIL SHEET C3.02)
- (SD) SILT FENCE STONE OVERFLOW (SEE DETAIL SHEET C3.02)
- (IP1) INLET PROTECTION TYPE 1 - GRATE INLET (SEE DETAIL SHEET C3.02)
- (IP2) INLET PROTECTION TYPE 2 - CURB INLET (SEE DETAIL SHEET C3.02)
- (CD) ROCK CHECK DAM (SEE DETAIL SHEET C3.03)
- (IS) INTERCEPTOR SWALE (SEE DETAIL SHEET C3.03)
- TREE TO REMAIN
- (ST) SEDIMENT TRAP (SEE DETAIL SHEET C3.03)

NOTES

1. AREAS CONTAINED WITHIN THE PROPERTY BOUNDARIES WILL BE AREAS OF DISTURBANCE AND SOIL STABILIZATION. ALL SOILS WITHIN THESE LIMITS SHALL BE STABILIZED BY VEGETATION OR STRUCTURE.
2. REFERENCE LANDSCAPE PLANS, BY OTHERS, FOR THE TREE PRESERVATION AND MITIGATION PLAN.

EROSION CONTROL IMPLEMENTATION SEQUENCE

1. UPON IMPLEMENTATION AND INSTALLATION OF THE FOLLOWING AREAS: TRAILER, PARKING, LAY DOWN, PORTA-POTTY, WHEEL WASH, CONCRETE WASHOUT, MASON'S AREA, FUEL AND MATERIAL STORAGE AREAS/CONTAINERS, SOLID WASTE CONTAINERS, ETC., IMMEDIATELY DENOTE THEM ON THE SITE MAPS AND NOTE ANY CHANGES IN LOCATION AS THEY OCCUR THROUGHOUT THE CONSTRUCTION PROCESS. IN ADDITION, NOTE ALL AREAS WHERE FILL IS IMPORTED FROM OR SOIL IS EXPORTED TO ON THE SITE MAPS.
2. DOWN SLOPE PROTECTIVE MEASURES MUST ALWAYS BE IN PLACE BEFORE SOIL IS DISTURBED. ACTIVITIES ARE PRESENTED IN THE ORDER OR SEQUENCE IN WHICH THEY ARE REQUIRED TO BE COMPLETED.

PHASE 1

1. CONTRACTOR SHALL FILE THE NOTICE OF INTENT ("NOI") AS PRIMARY OPERATOR AND SIGN ALL REQUIRED STATE CERTIFICATIONS AND DOCUMENTATION AND OBTAIN LOCAL PERMITS FROM THE CITY. CONTRACTOR SHALL INSTALL THE SWPPP INFORMATION SIGN AND POST REQUIRED DOCUMENTS NEAR THE PLANNED CONSTRUCTION EXIT AND WITHIN EASY ACCESS TO THE GENERAL PUBLIC WITHOUT ENTERING THE SITE.
2. STAKE/FLAG THE LIMITS OF DISTURBANCE ("LOD") AND TREE SAVE AREAS (WHERE STAKING IS NOT POSSIBLE/PRACTICAL, THE LOD MUST BE CONSPICUOUSLY, AND PROMINENTLY, MARKED TO DENOTE THE BOUNDARY). CONSTRUCTION FENCING MAY BE USED TO MARK THE LOD WHERE THE CONSTRUCTION FENCING IS IMMEDIATELY ADJACENT TO THE LOD. LOD MUST REMAIN CONSPICUOUSLY MARKED THROUGHOUT THE ENTIRE CONSTRUCTION PROJECT.
3. CONTRACTOR SHALL SCHEDULE AND CONDUCT, AS NEEDED, AN ON-SITE MEETING WITH THE CONTRACTOR, DESIGN ENGINEER/PERMIT APPLICANT AND ENVIRONMENTAL INSPECTOR AFTER INSTALLATION OF THE EROSION/SEDIMENTATION CONTROLS AND TREE/NATURAL AREA PROTECTION MEASURES AND PRIOR TO BEGINNING ANY SITE PREPARATION WORK. THE CONTRACTOR SHALL NOTIFY THE CITY AT LEAST THREE (3) DAYS PRIOR TO THE MEETING DATE.
4. INSTALL PERIMETER SEDIMENT CONTROL BMPS IN THE VICINITY OF, AND DOWN GRADIENT FROM, THE LOCATION OF THE PLANNED CONSTRUCTION EXIT, CONSTRUCTION OFFICE TRAILER, AND TEMPORARY PARKING AND STORAGE AREAS. CLEAR ONLY THE MINIMUM AREA ABSOLUTELY NECESSARY TO INSTALL THESE PERIMETER CONTROL BMPS.
5. INSTALL STABILIZED CONSTRUCTION EXIT AND SET THE PROJECT OFFICE TRAILER.
6. INSTALL REMAINING PERIMETER SEDIMENT CONTROL BMPS INCLUDING CONSTRUCTION FENCING, PERIMETER SILT FENCE, STONE OVERFLOWS, INLET PROTECTION, ETC., AS SHOWN ON THE SITE MAPS. CLEAR ONLY THE MINIMUM AREA NECESSARY TO INSTALL PERIMETER CONTROL BMPS.
7. PREPARE TEMPORARY PARKING AND STORAGE AREA.
8. GENERAL CONTRACTOR, AS REQUIRED, SHALL SCHEDULE AND CONDUCT THE STORMWATER PRE-CONSTRUCTION MEETING WITH THE CEC, OWNER'S CONSTRUCTION MANAGER, AGENCY(IES) AND SUBCONTRACTORS BEFORE PROCEEDING WITH CONSTRUCTION.
9. CLEAR ONLY THE MINIMUM AREA ABSOLUTELY NECESSARY TO INSTALL SEDIMENT BASINS AND TRAPS, INCLUDING STOCKPILE AREAS NECESSARY FOR SPOIL FROM THESE REQUIRED STRUCTURAL SEDIMENT CONTROL BMPS.
10. CONSTRUCT AND STABILIZE THE SEDIMENT BASIN 2 WITH APPROPRIATE OUTFALL STRUCTURES (TEMPORARY OR PERMANENT), AS SPECIFIED ON THE SITE MAPS. CONTRACTOR SHALL FOLLOW THE BUILDING PAD FOUNDATION PREPARATION, PER THE GEOTECHNICAL REPORT, AS IT PERTAINS TO THE EARTHWORK FOR THE BUILDING PAD.
11. STABILIZE SIDE SLOPES, BOTTOM AND ALL SIDES OF EMBANKMENTS OR SLOPES OF THE SEDIMENT BASINS AND TRAPS IMMEDIATELY UPON COMPLETION, AS SPECIFIED IN THE SWPPP.
12. INSTALL HYDRAULIC CONTROL STRUCTURES (DIVERSION DIKES, DIVERSION SWALES, CHECK DAMS, ETC.), AS SPECIFIED ON THE SITE MAPS.
13. STABILIZE SIDE SLOPES AND FLOW LINE OF HYDRAULIC CONTROL STRUCTURES (DIVERSION DIKES AND SWALES) WITH SEED, FERTILIZER AND ROLLED EROSION CONTROL PRODUCTS OR OTHER EROSION RESISTANT LINING, AS SPECIFIED IN THE SWPPP.

NOTE: THE SEQUENCE OF CONSTRUCTION SHOWN ABOVE IS A GENERAL OVERVIEW AND IS INTENDED TO CONVEY THE GENERAL CONCEPTS OF THE EROSION CONTROL DESIGN AND SHOULD NOT BE RELIED UPON FOR CONSTRUCTION PURPOSES. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR DETAILED PHASING AND CONSTRUCTION SEQUENCING NECESSARY TO CONSTRUCT THE PROPOSED IMPROVEMENTS INCLUDED IN THESE PLANS. THE CONTRACTOR SHALL NOTIFY ENGINEER IN WRITING IMMEDIATELY, PRIOR TO AND/OR DURING CONSTRUCTION IF ANY ADDITIONAL INFORMATION ON THE CONSTRUCTION SEQUENCE IS NECESSARY. CONTRACTOR IS SOLELY RESPONSIBLE FOR COMPLYING WITH THE REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION AND ALL OTHER APPLICABLE LAWS.

PHASE 2 SEQUENCE CONTINUED ON SHEET C3.01

BENCHMARK LIST

LCRA BENCHMARK PID NUMBER A490
NAVD 88 DATUM
ELEVATION: 633.05'

811 Know what's below. Call before you dig.

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REVISION	DATE	
	05.08.25	ADDENDUM #1

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS C.I.S.D.
KYLE, TEXAS

Kimley»Horn
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TBPE FIRM NO. 928

STATE OF TEXAS
RICHARD J. UNDERWOOD
117843
REGISTERED
PROFESSIONAL ENGINEER
05/08/2025

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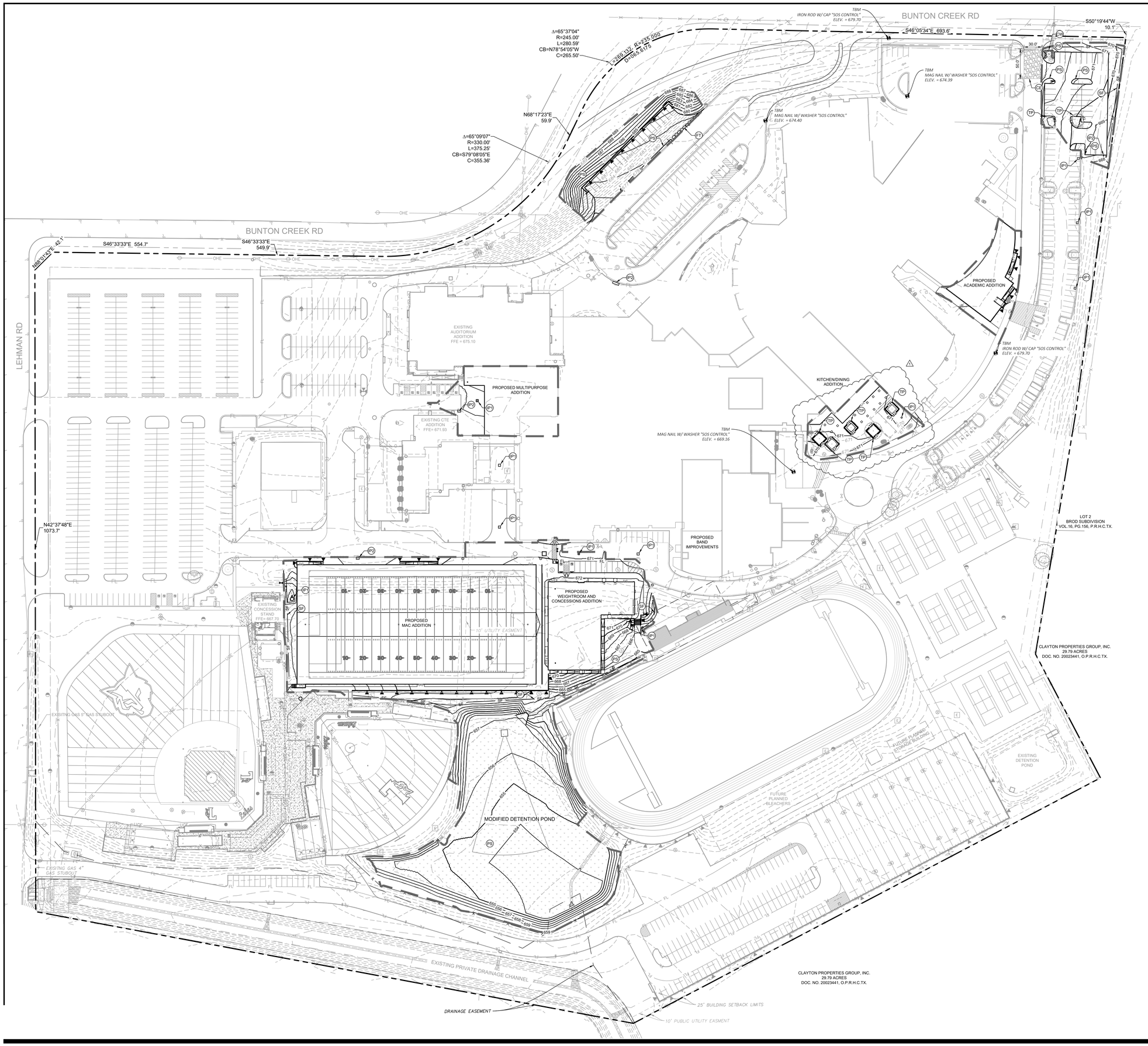
EROSION CONTROL PLAN PHASE 1

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID
Drawn By: RAU	C3.00
Date: 05/08/2025	



NORTH

GRAPHIC SCALE IN FEET
0 30 60 120

LEGEND

- PROPERTY BOUNDARY
- EXISTING CONTOUR
- PROPOSED CONTOUR
- PROPOSED LIMITS OF DISTURBANCE (APPROXIMATE)
- PROPOSED RETAINING WALL (TRIANGLE INDICATES FACE OF WALL)
- DIRECTION OF OVERLAND FLOW W/GRADE
- SF — SILT FENCE (SEE DETAIL SHEET C3.02)
- TP — TREE PROTECTION (SEE DETAIL SHEET C3.02)
- CE — CONSTRUCTION EXIT (SEE DETAIL SHEET C3.02)
- PS — PERMANENT STABILIZATION
- RF — ROCK FILTER DAM (SEE DETAIL SHEET C3.02)
- SO — SILT FENCE STONE OVERFLOW (SEE DETAIL SHEET C3.02)
- IP1 — INLET PROTECTION TYPE 1 - GRATE INLET (SEE DETAIL SHEET C3.02)
- IP2 — INLET PROTECTION TYPE 2 - CURB INLET (SEE DETAIL SHEET C3.02)
- TREE TO REMAIN
- CW — CONCRETE WASH-OUT PIT (SEE DETAIL SHEET C3.02)
- FT — FILTREXX SILT SOXX. (SEE DETAIL SHEET C3.03)

NOTES

1. AREAS CONTAINED WITHIN THE PROPERTY BOUNDARIES WILL BE AREAS OF DISTURBANCE AND SOIL STABILIZATION. ALL SOILS WITHIN THESE LIMITS SHALL BE STABILIZED BY VEGETATION OR STRUCTURE.
2. REFERENCE LANDSCAPE PLANS, BY OTHERS, FOR THE TREE PRESERVATION AND MITIGATION PLAN.

EROSION CONTROL IMPLEMENTATION SEQUENCE

PHASE 2

1. BEGIN CLEARING, GRUBBING, AND STRIPPING THE SITE. (PHASE CLEARING, GRUBBING, AND STRIPPING TO THE EXTENT PRACTICAL TO MINIMIZE THE AMOUNT OF AREA DISTURBED AT ANY POINT IN TIME)
2. BEGIN GRADING THE SITE AND BEGIN RETAINING WALL CONSTRUCTION.
3. START CONSTRUCTION OF BUILDING PAD AND STRUCTURES. AT ANY POINT, INSTALL OFF-SITE BMPS AND BEGIN THE UTILITY SERVICE AND CITY RIGHT TURN LANE CONSTRUCTION, AFTER CONTACTING SAWS AND THE CITY THAT WORK HAS COMMENCED.
4. TEMPORARILY STABILIZE, THROUGHOUT CONSTRUCTION IMMEDIATELY FOLLOWING THE COMPLETION OF THE MOST RECENT LAND DISTURBING/GRADING ACTIVITY, ANY DISTURBED AREAS, INCLUDING MATERIAL STOCKPILES THAT ARE SCHEDULED OR LIKELY TO REMAIN INACTIVE FOR 14 DAYS OR MORE.
5. IMMEDIATELY PERMANENTLY STABILIZE AREAS TO BE VEGETATED AS THEY ARE BROUGHT TO FINAL GRADE.
6. BEGIN INSTALLING UTILITIES, UNDERDRAINS, STORM SEWERS, CURBS AND GUTTERS.
7. INSTALL RIP RAP AROUND OUTLET STRUCTURES AS EACH OUTLET STRUCTURE IS INSTALLED.
8. INSTALL INLET PROTECTION AT ALL STORM SEWER STRUCTURES AS EACH INLET STRUCTURE IS INSTALLED.
9. PREPARE SITE FOR PAVING.
10. BEGIN PAVING SITE.
11. WHEN THE AREAS UP-GRADIENT OF THE SEDIMENTATION BASIN HAVE BEEN STABILIZED WITH STONE BASE, THE CONTRACTOR MUST OBTAIN CONCURRENCE FROM THE ENGINEER, OWNER, AND LOCAL SITE EROSION AND SEDIMENT CONTROL INSPECTOR THAT THE MEASURES CAN BE REMOVED. PRIOR TO BACKFILLING, SEDIMENT IN THE BASIN AND TRAPS MUST BE REMOVED AND LEGALLY DISPOSED OF.
12. FINALIZE CONSTRUCTION, LANDSCAPING, AND SITE STABILIZATION. OBTAIN CONCURRENCE FROM THE OWNER, ENGINEER, AND LOCAL INSPECTOR, THAT ALL SITE AREAS HAVE BEEN FULLY STABILIZED AND ALL CONSTRUCTION HAS BEEN COMPLETED, THEN:
   - A. REMOVE ALL REMAINING TEMPORARY EROSION AND SEDIMENT CONTROL BEST MANAGEMENT PRACTICES (BMPS).
   - B. STABILIZE ANY AREAS DISTURBED BY THE REMOVAL OF TEMPORARY BMPS, AND
   - C. FILE THE NOTICE OF TERMINATION WITH TCEQ AND FOLLOW CLOSE-OUT PROCEDURES WITH THE CITY.

NOTE: THE SEQUENCE OF CONSTRUCTION SHOWN ABOVE IS A GENERAL OVERVIEW AND IS INTENDED TO CONVEY THE GENERAL CONCEPTS OF THE EROSION CONTROL DESIGN AND SHOULD NOT BE RELIED UPON FOR CONSTRUCTION PURPOSES. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR DETAILED PHASING AND CONSTRUCTION SEQUENCING NECESSARY TO CONSTRUCT THE PROPOSED IMPROVEMENTS INCLUDED IN THESE PLANS. THE CONTRACTOR SHALL NOTIFY ENGINEER IN WRITING IMMEDIATELY, PRIOR TO AND/OR DURING CONSTRUCTION IF ANY ADDITIONAL INFORMATION ON THE CONSTRUCTION SEQUENCE IS NECESSARY. CONTRACTOR IS SOLELY RESPONSIBLE FOR COMPLYING WITH THE REQUIREMENTS OF THE AUTHORITY HAVING JURISDICTION AND ALL OTHER APPLICABLE LAWS.

SITE DATA

TOTAL LOT AREA	53.577 AC	2,333,814 SF
TOTAL AREA DISTURBED *	18.651 AC	812,424 SF
NEW PAVED AREA	5.186 AC	225,890 SF
NEW ROOFED AREA	0.819 AC	35,668 SF
NEW LANDSCAPED AREA	8.726 AC	380,104 SF

* DOES NOT INCLUDE ANY OFF-SITE DISPOSAL OR BORROW AREAS - CONTRACTOR TO UPDATE AS NECESSARY DURING CONSTRUCTION.

* NO SINGLE DRAINAGE AREA EXCEEDS 10 ACRES, THEREFORE SEDIMENTATION BASIN IS NOT REQUIRED.

BENCHMARK LIST

LCRA BENCHMARK PID NUMBER A490
NAVD 88 DATUM
ELEVATION: 633.05'

811 Know what's below. Call before you dig.

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REVISION	DATE	ADDENDUM #1
1	05.08.25	

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS C.I.S.D.
KYLE, TEXAS

Kimley»Horn
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STATE OF TEXAS
RICHARD J. UNDERWOOD
117843
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05/08/2025

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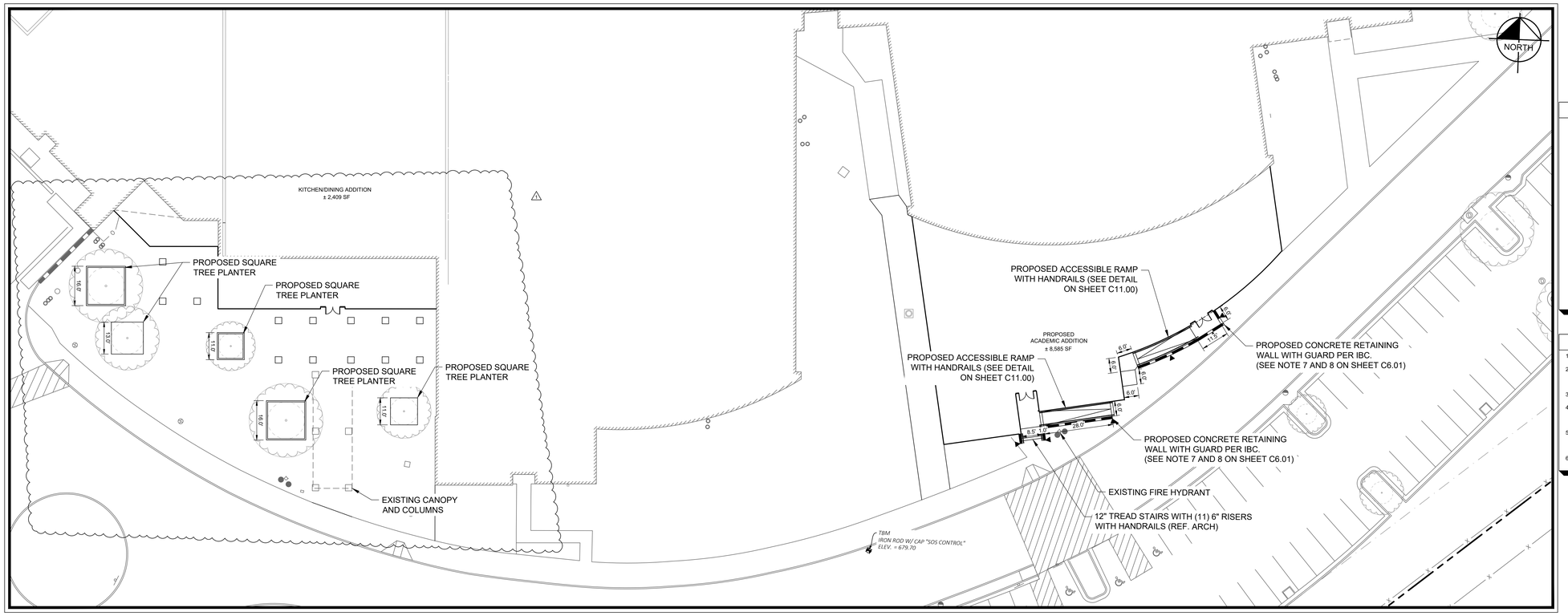
EROSION CONTROL PLAN PHASE 2

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID C3.01
Drawn By: RAU	
Date: 05/08/2025	



GRAPHIC SCALE IN FEET
0 10 20 40

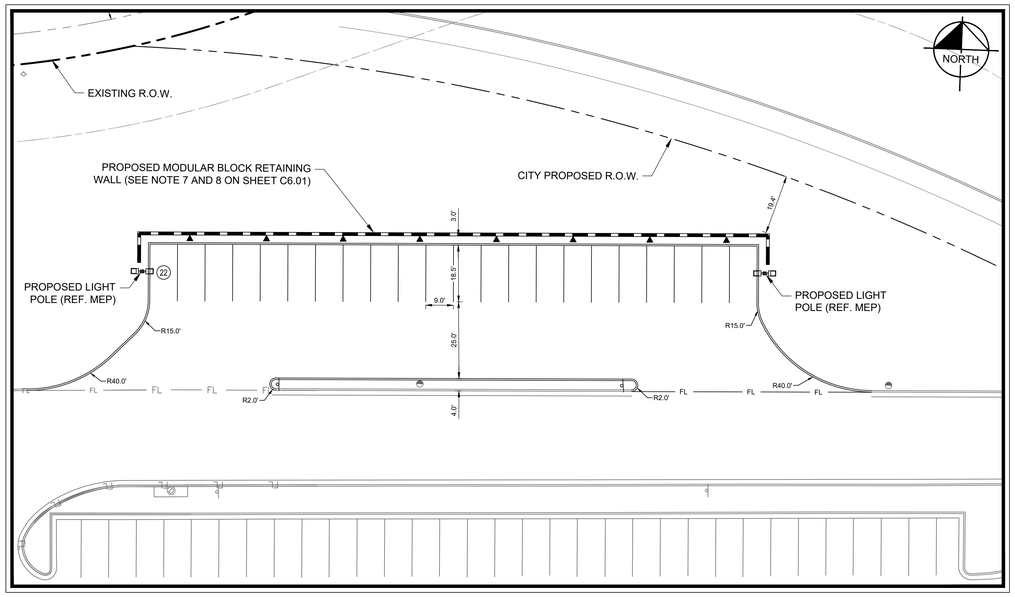
LEGEND

- PROPERTY BOUNDARY
- PROPOSED SAWCUT LINE
- FL — PROPOSED FIRE LANE
- X — PROPOSED FENCE
- PROPOSED RETAINING WALL (TRIANGLE INDICATE FACE OF WALL)
- PROPOSED PARKING COUNT
- PROPOSED ACCESSIBLE PARKING SPACE
- PROPOSED BARRIER FREE RAMP
- PROPOSED SANITARY SEWER MANHOLE
- CI — PROPOSED CURB INLET
- EXISTING SANITARY SEWER MANHOLE
- EXISTING FIRE HYDRANT
- EXISTING POWER POLE
- EXISTING LIGHT POLE

NOTES

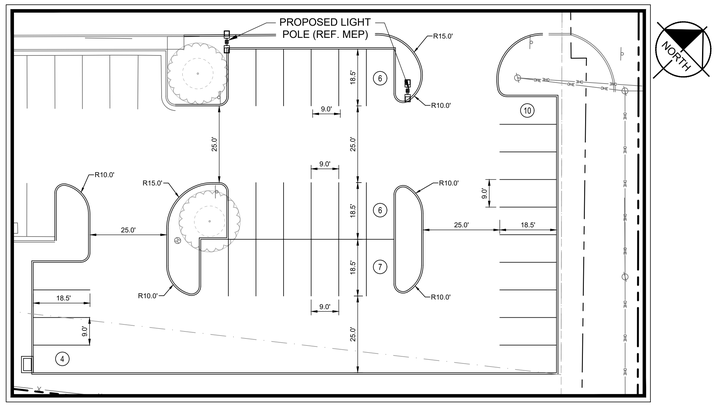
1. ALL DIMENSIONS ARE TO FACE OF CURB UNLESS OTHERWISE NOTED.
2. REFER TO ARCHITECTURAL CONSTRUCTION DRAWINGS FOR EXACT BUILDING DIMENSIONS. REFER TO LANDSCAPE ARCHITECT'S PLANS FOR DIMENSIONS AND DETAIL OF HARDSCAPE.
3. ALL CURB RADII ARE 3 FEET UNLESS DIMENSIONED OTHERWISE.
4. BUILDING, MECHANICAL EQUIPMENT AND SIGNS ARE SHOWN HEREON FOR REFERENCE ONLY. REFER TO CONSTRUCTION PLANS OF THOSE ITEMS FOR LOCATIONS AND DIMENSIONS.
5. ALL CONSTRUCTION SPECIFICATIONS WITHIN CITY RIGHT-OF-WAY AND EASEMENTS SHALL COMPLY WITH CITY OF KYLE STANDARDS. PRIOR APPROVAL TO USE ANY NON-STANDARD MATERIAL IS REQUIRED.
6. PROPOSED RETAINING WALLS TO BE STRUCTURALLY DESIGNED AND PERMITTED BY CONTRACTOR.

INSET A



INSET B

INSET C



BENCHMARK LIST
LCRA BENCHMARK PID NUMBER A490 NAVD 88 DATUM ELEVATION: 633.05'

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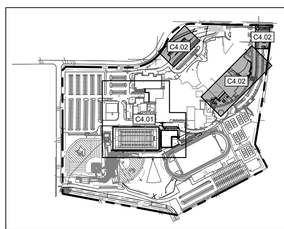

KEY MAP
N.T.S.

ADDENDUM #1
DATE 05.08.25
REVISION

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS C.I.S.D.
KYLE, TEXAS

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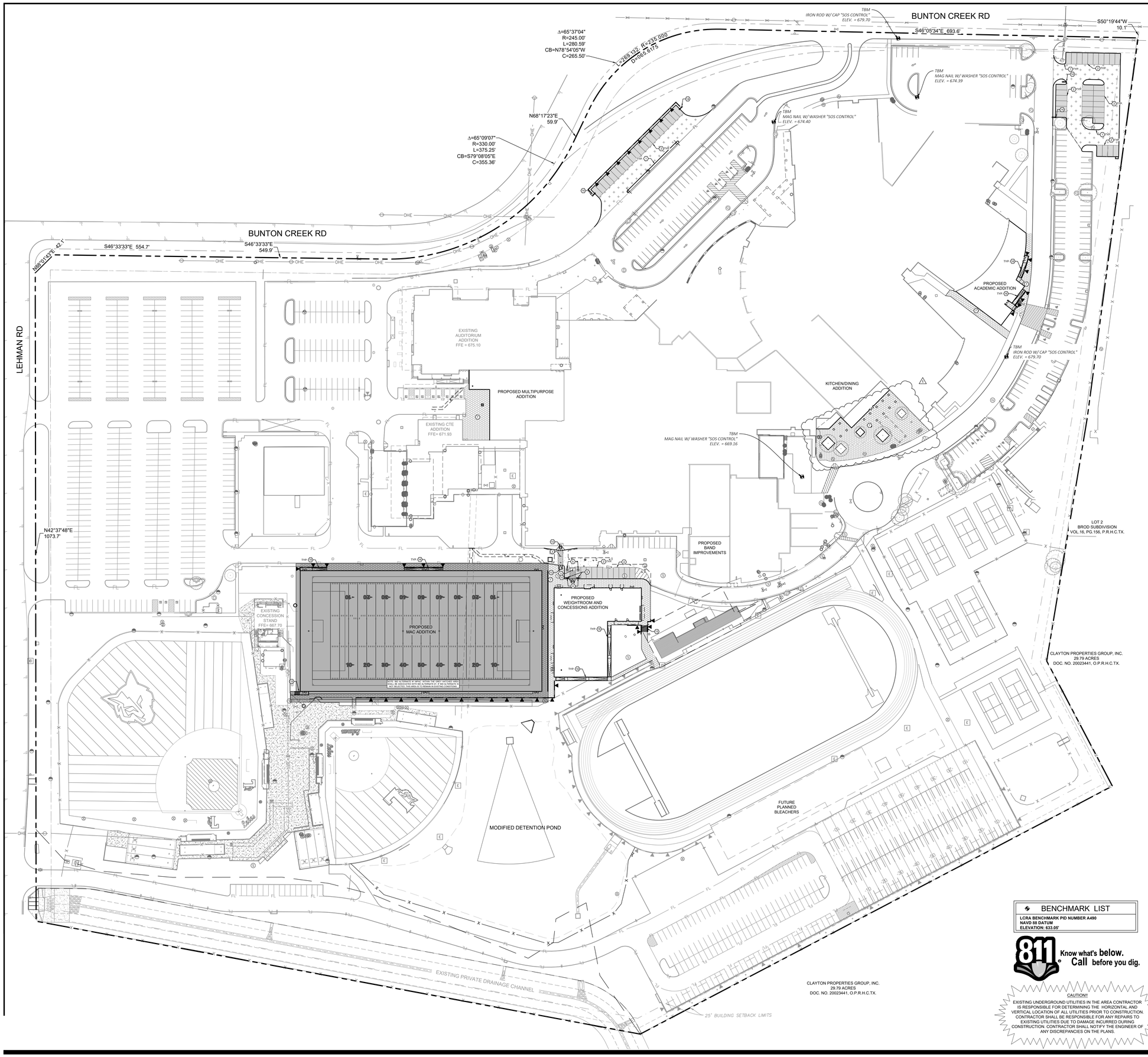
DIMENSION CONTROL PLAN (2 OF 2)

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID C4.02
Drawn By: RAU	
Date: 05/08/2025	



GRAPHIC SCALE IN FEET
0 30 60 120

LEGEND

1	PROPOSED 6" CONCRETE CURB (REF. SHEET C11.00 FOR DETAILS)
2	4" PAINTED STRIPE (TYP.)
3	4" PAINTED STRIPING, 2' O.C. @ 45°
4	FIRE LANE STRIPING (REF. DETAIL, SHEET C11.00)
5	ACCESSIBLE PARKING SYMBOL (REF. DETAIL, SHEET C11.00)
6	ACCESSIBLE PARKING SIGN (REF. DETAIL, SHEET C11.00)
7	CONSTRUCT ON-SITE CONCRETE SIDEWALK (REF. DETAIL, SHEET C11.00)
8	CONSTRUCT PUBLIC CONCRETE SIDEWALK (REF. DETAIL, SHEET C11.00)
9	CONCRETE WHEELSTOP (REF. DETAIL, SHEET C11.00)
10	CONSTRUCT ON-SITE BARRIER FREE RAMP (REF. DETAIL, SHEET C11.00)
11	CONSTRUCT PUBLIC BARRIER FREE RAMP (REF. DETAIL, SHEET C11.00)
12	MOWER ACCESS
13	PROPOSED DIRECTIONAL ARROW STRIPING. CONTRACTOR TO PROVIDE SHOP DRAWING OF PAVEMENT MARKINGS FOR APPROVAL PRIOR TO INSTALLATION
14	PROPOSED LIGHT POLE (REF. MEP)
15	CONCRETE STAIRS WITH HANDRAILS (REF. DETAIL, SHEET C11.00)
16	PROPOSED RAMP WITH HANDRAILS (REF. ARCH)
	PROPOSED LIGHT-DUTY CONCRETE PAVEMENT (GENERAL PARKING PAVEMENT PER GEOTECH REPORT)
	PROPOSED MEDIUM-DUTY CONCRETE PAVEMENT (ACCESS DRIVE PAVEMENT PER GEOTECH REPORT)
	PROPOSED HEAVY-DUTY CONCRETE PAVEMENT (BUS LOOP/FIRE PAVEMENT PER GEOTECH REPORT)
	PROPOSED ON-SITE (PRIVATE) SIDEWALK
	PROPOSED AREA OF RESTRIPING

NOTES

1. REFERENCE GEOTECHNICAL REPORT FOR ADDITIONAL PAVING AND SOIL PREPARATION NOTES.
2. REFERENCE DIMENSION CONTROL PLAN ON SHEET C4.00 FOR CURB RADII AND LAYOUT INFORMATION.
3. REFERENCE IRRIGATION AND MEP PLANS FOR CONDUIT SIZES AND LOCATIONS UNLESS OTHERWISE NOTED ON THIS SHEET.
4. EXPANSION JOINTS SHOULD BE USED WHEREVER THE PAVEMENT WILL ABUT A STRUCTURAL ELEMENT SUBJECT TO DIFFERENT MAGNITUDE OF MOVEMENT, E.G., LIGHT POLES, RETAINING WALLS, EXISTING PAVEMENT, STAIRWAYS, ENTRYWAY PIERS, BUILDING WALLS, OR MANHOLES.
5. EXISTING MANHOLE TOPS, VALVE BOXES, ETC. ARE TO BE ADJUSTED AS REQUIRED TO MATCH PROPOSED GRADES. IF NECESSARY, ADJUSTMENTS SHALL BE PERFORMED UPON COMPLETION OF PAVING AND FINE GRADING TO ENSURE A SMOOTH TRANSITION.
6. MOWER ACCESS CONSISTS OF 6' OF LAY DOWN CURB WITH 2' TRANSITION ON EACH SIDE
7. ALL SIDEWALKS AND CURB IS REQUIRED TO BE DOWELED INTO CURB AND ENTRIES.

TAS NOTE

GROUND AND FLOOR SURFACES ALONG ACCESSIBLE ROUTES AND IN ACCESSIBLE ROOMS AND SPACES INCLUDING FLOORS, WALKS, RAMPS, STAIRS, AND CURB RAMPS, SHALL BE STABLE, FIRM, SLIP-RESISTANT, AND SHALL COMPLY WITH SECTION 302 OF THE TEXAS ACCESSIBILITY STANDARDS.

BENCHMARK LIST

LCRA BENCHMARK PID NUMBER A490
NAVD 88 DATUM
ELEVATION: 633.05'

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REVISION	DATE	ADDENDUM #1
1	05.09.25	

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS C.I.S.D.
KYLE, TEXAS

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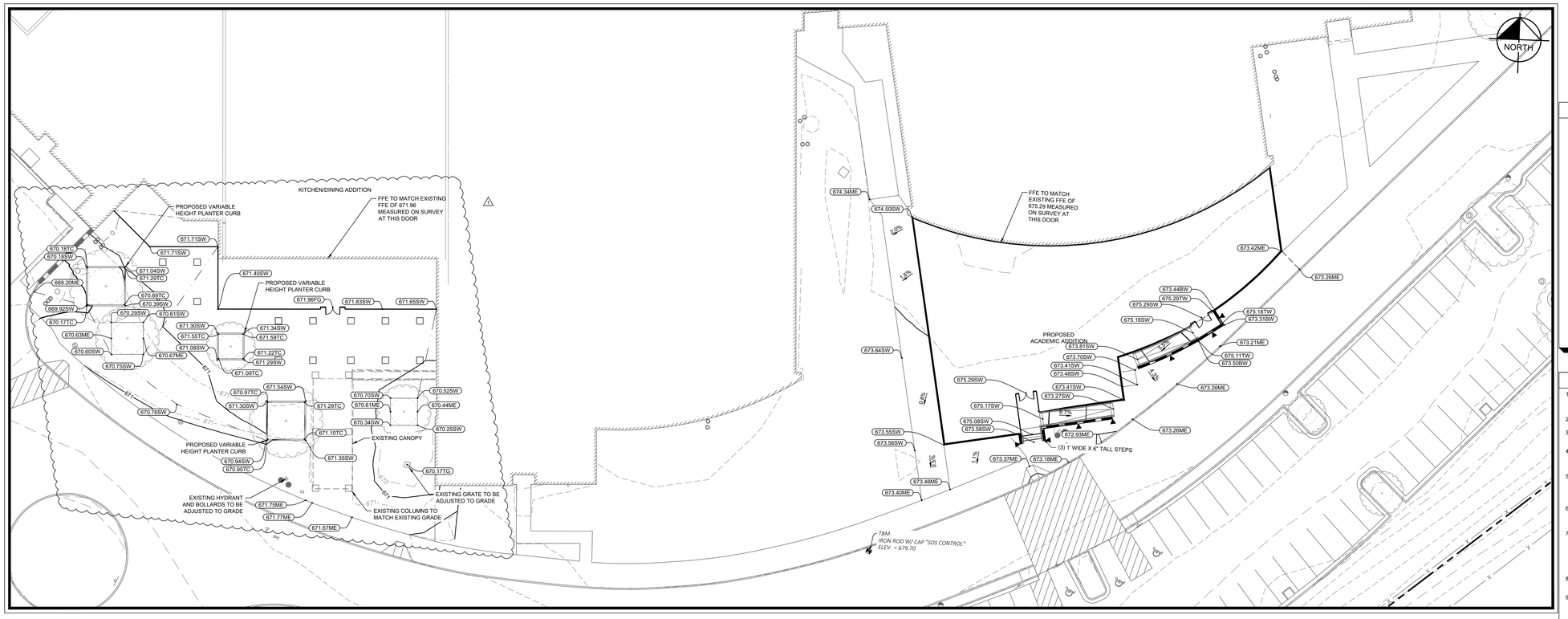
PAVING PLAN

PACKAGE	VOLUME

Job No.	Sheet No.
019544-08-01	ISSUE FOR BID
Drawn By: RAU	C5.01
Date: 05/08/2025	



GRAPHIC SCALE IN FEET
0 10 20 40

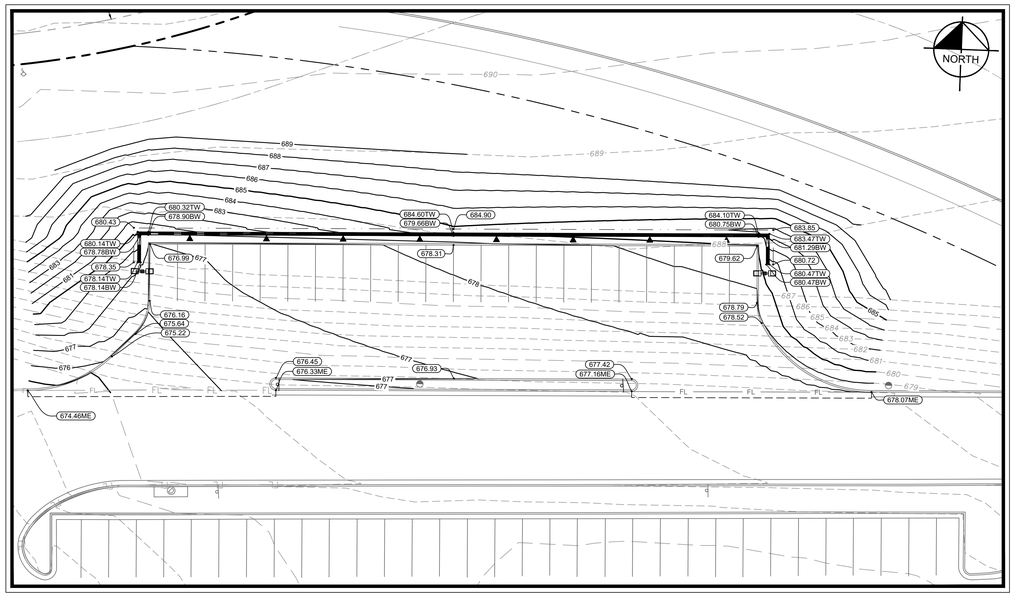
LEGEND

XXX.XX	PROPOSED SPOT ELEVATION
SW	SIDEWALK
TS	TOP OF STEP
FG	FINISHED GRADE
TW	TOP OF WALL
BW	FINISHED GRADE AT BASE OF WALL
TG	TOP OF GRATE
EX	EXISTING SPOT ELEVATION
ME	MATCH EXISTING
910	PROPOSED CONTOURS
905	EXISTING CONTOURS
HP	PROPOSED HIGH POINT
	PROPOSED SWALE
	PROPOSED RETAINING WALL (TRIANGLE INDICATES FACE OF WALL)
	DIRECTION OF INTENDED FLOW
	PROPOSED CURB RAMP
	PROPOSED RAMP WITH HANDRAIL (SEE NOTE 7)

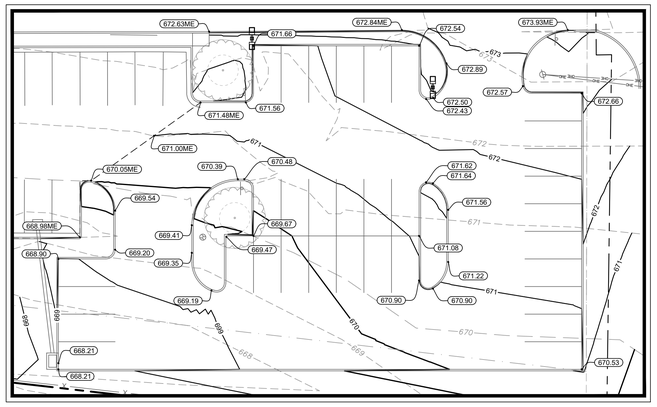
NOTES

1. ALL SPOT GRADES ARE TO TOP OF PAVEMENT (TP) OR TOP OF GRATE (TG), UNLESS OTHERWISE NOTED AS TC (TOP OF CURB). CONTRACTOR TO ADD 6" FOR TOP OF CURB AS NECESSARY.
2. NO EARTHEN SLOPE SHALL BE GREATER THAN 4:1, UNLESS OTHERWISE NOTED.
3. MAXIMUM SLOPE IN ACCESSIBLE PARKING SPACES, LOADING ZONES AND SIDEWALK LANDINGS SHALL NOT EXCEED 2.0% IN ALL DIRECTIONS.
4. MAXIMUM RUNNING SLOPE SHALL NOT EXCEED 5% AND CROSS SLOPE SHALL NOT EXCEED 2% ON ALL SIDEWALKS UNLESS OTHERWISE NOTED. RUNNING SLOPE MAY EXCEED 5% IN PUBLIC R.O.W. IF EXISTING ROAD SLOPE EXCEEDS 5%.
5. GENERAL CONTRACTOR TO REFERENCE NOTE 1 REGARDING SPOT ELEVATIONS, COORDINATE WITH DIRT AND LANDSCAPE SUBCONTRACTORS REGARDING PROPOSED SOD AND HYDROMULCH LOCATIONS TO ENSURE ADEQUATE CUT FOR FUTURE VEGETATION.
6. EXISTING MANHOLE TOPS, VALVE BOXES, ETC. ARE TO BE ADJUSTED AS REQUIRED TO MATCH PROPOSED GRADES. IF NECESSARY, READJUSTMENTS SHALL BE PERFORMED UPON COMPLETION OF PAVING AND FINE GRADING TO ENSURE A SMOOTH TRANSITION.
7. RETAINING WALL DESIGN BY CONTRACTOR SHALL TAKE INTO CONSIDERATION THE SURROUNDING PROPOSED IMPROVEMENTS, SUCH AS LIGHT POLES AND PARKING. CONTRACTOR SHALL PROVIDE CONSTRUCTION PLANS, INCLUDING STRUCTURAL DESIGN AND HANDRAIL, FOR THE RETAINING WALL IN CONFORMANCE WITH CITY STANDARDS. CONTRACTOR SHALL SUBMIT THE PLANS FOR OWNER, ARCHITECT, AND ENGINEER REVIEW AND CONTRACTOR SHALL OBTAIN CITY PERMIT
8. PROPOSED RETAINING WALLS TO BE STRUCTURALLY DESIGNED AND PERMITTED BY CONTRACTOR.
9. 10. PER SECTION 1804.4 OF THE INTERNATIONAL BUILDING CODE, THE GROUND IMMEDIATELY ADJACENT TO THE FOUNDATION SHALL BE SLOPED AWAY FROM THE BUILDING AT A 5% SLOPE FOR A MINIMUM DISTANCE OF 10 FEET MEASURED PERPENDICULAR TO THE FACE OF WALL UNLESS AN APPROVED ALTERNATIVE METHOD OF DIVERTING WATER AWAY FROM THE FOUNDATION IS IMPLEMENTED. WHERE THIS PLAN DOES NOT COMPLY WITH THIS REQUIREMENT, IT IS UNDERSTOOD THAT THE BUILDING OFFICIAL'S APPROVAL OF THIS PLAN WILL BE CONSIDERED AN APPROVED ALTERNATE METHOD OF DIVERTING WATER AWAY FROM THE FOUNDATION.

INSET A



INSET B



INSET C

BENCHMARK LIST
LCRA BENCHMARK PID NUMBER A490 NAVD 88 DATUM ELEVATION: 633.05'

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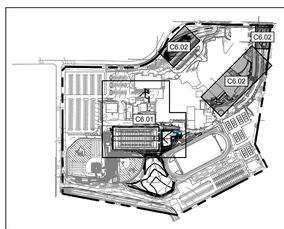

KEY MAP
N.T.S.

REVISION	DATE
ADDENDUM #1	05.08.25

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS C.I.S.D.
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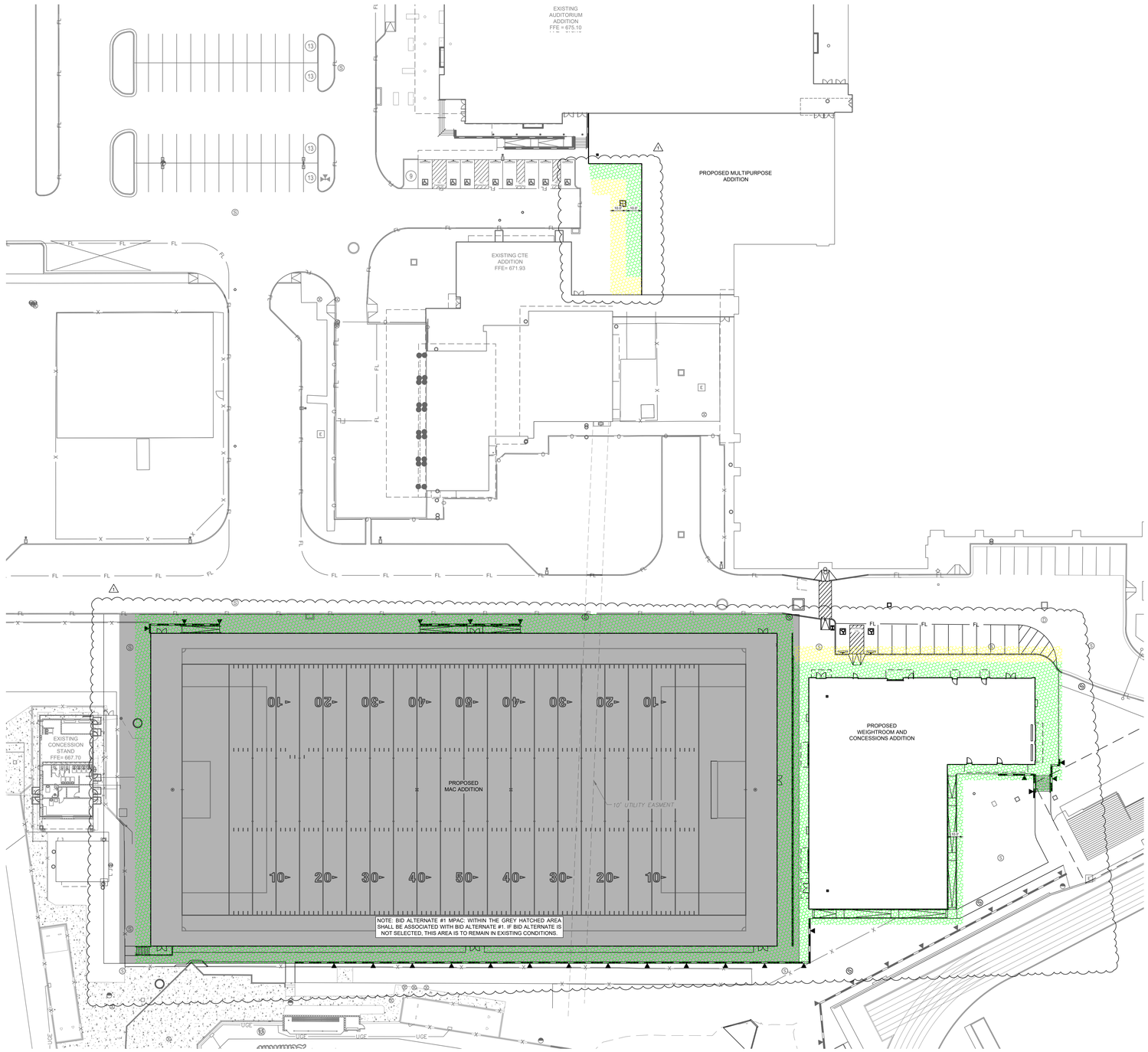
GRADING PLAN
(2 OF 2)

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID
Drawn By: RAU	C6.02
Date: 05/08/2025	



GRAPHIC SCALE IN FEET
0 15 30 60

LEGEND

PROPERTY BOUNDARY	

PVR TABLE

	DESIRED PVR	THICKNESS OF LIME-TREATED ON-SITE SOIL
	1 INCH	10 FT
	2 INCHES	9 FT

REVISION	DATE	
1	05.09.25	ADDENDUM #1

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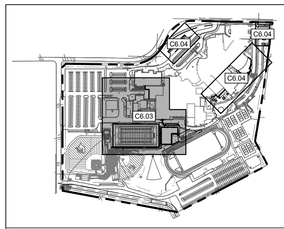

KEY MAP
N.T.S.

PVR PLAN (SHEET 1 OF 2)

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID C6.03
Drawn By: RAU	
Date: 05/08/2025	

GRAPHIC SCALE IN FEET
0 10 20 40

LEGEND

	PROPERTY BOUNDARY

PVR TABLE

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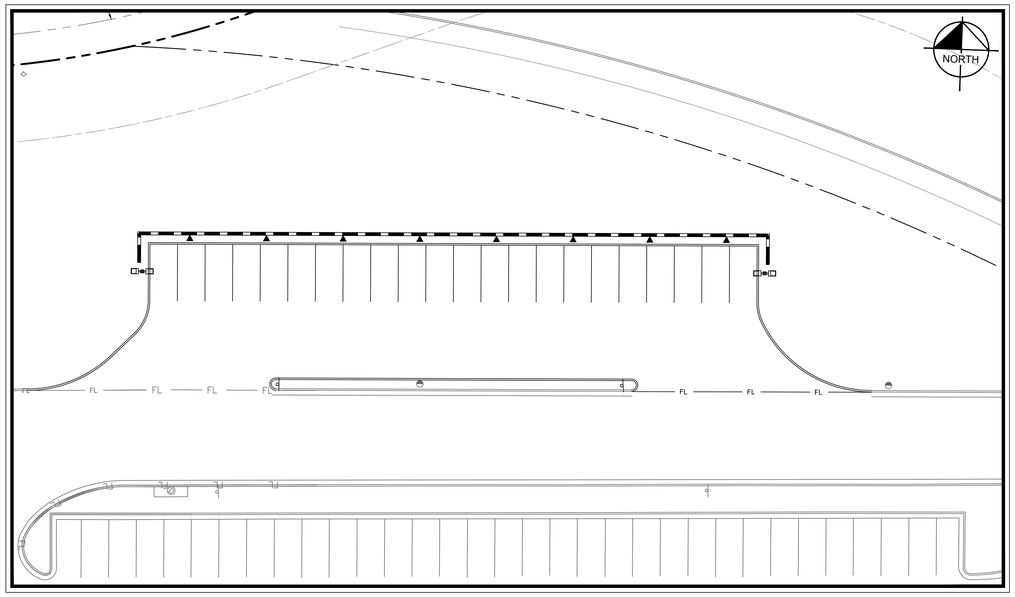
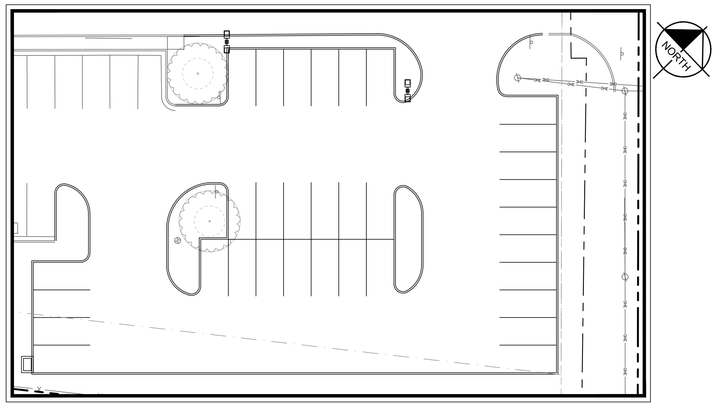
	1 INCH	10 FT
	2 INCHES	9 FT

REVISION
DATE
ADDENDUM #1
05.08.25

INSET A



INSET B



INSET C

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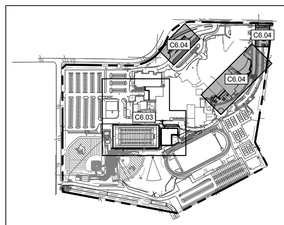
ELEVATION: 633.05'

811 Know what's below. Call before you dig.

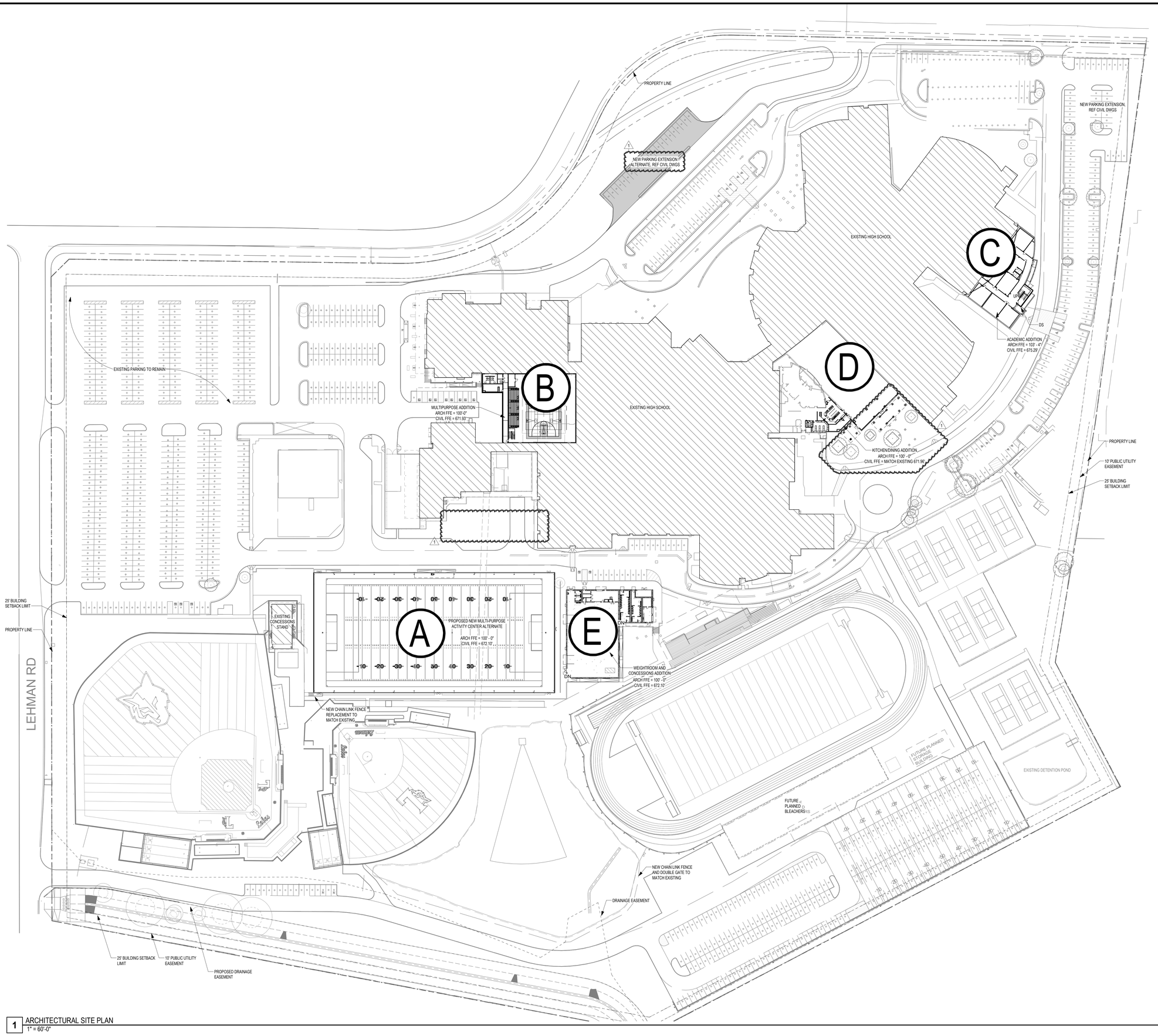
CAUTION!
EXISTING UNDERGROUND UTILITIES IN THE AREA CONTRACTOR IS RESPONSIBLE FOR DETERMINING THE HORIZONTAL AND VERTICAL LOCATION OF ALL UTILITIES PRIOR TO CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY REPAIRS TO EXISTING UTILITIES DUE TO DAMAGE INCURRED DURING CONSTRUCTION. CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES ON THE PLANS.



KEY MAP
N.T.S.

PVR PLAN (SHEET 2 OF 2)

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID
Drawn By: RAU	C6.04
Date: 05/08/2025	



1 ARCHITECTURAL SITE PLAN
1" = 60'-0"

1. ALL SITE INFORMATION INCLUDING, BUT NOT LIMITED TO, PROPERTY LINES, METES & BOUNDS, CONTOURS, EXISTING BUILDINGS, EASEMENTS, TREES, WALKS, DRIVES, FENCES, ETC., TAKEN FROM SURVEY BY:

   SPOT ON SURVEYING
   614 JERRYS LANE
   BUDA, TX 78610
   PHONE (512) 523-8092

2. HUCKABEE AND ASSOCIATES, INC. ALONG WITH THEIR CONSULTANTS HAVE LOCATED, SHOWN , AND NOTED EXISTING SITE FEATURES INCLUDING BUT NOT LIMITED TO PROPERTY LINES, METES AND BOUNDS, GRADE CONTOURS, EXISTING STRUCTURES, EASEMENTS, TREES, FENCES, ROADWAYS, OVERHEAD AND UNDERGROUND UTILITIES, AND OTHER EXISTING FEATURES TO THE BEST OF OUR ABILITY BASED UPON OWNER PROVIDED SURVEYS, EXISTING CONSTRUCTION DOCUMENTS AND OTHER EXISTING DATA COLLECTION IN REGARDS TO THE SPECIFIED SITE. HUCKABEE AND ASSOCIATES, INC. NOR THEIR CONSULTANTS SHALL ASSUME ANY RESPONSIBILITY OR LIABILITIES IN REGARD TO THE ACCURACY OF THE ABOVE MENTIONED OWNER PROVIDED DATA.

3. IF THE CONTRACTOR FINDS ANY DISCREPANCY BETWEEN EXISTING CONDITIONS AND THOSE WHICH ARE SHOWN ON THE PLANS, HE SHALL IMMEDIATELY CONTACT THE ARCHITECT.

4. THE CONTRACTOR SHALL MAKE ARRANGEMENTS WITH ALL UTILITY ENTITIES TO LOCATE AND MARK ALL UNDERGROUND UTILITIES PRIOR TO CONSTRUCTION.

5. THE CONTRACTOR SHALL VISIT THE SITE AND BECOME FAMILIAR WITH ALL EXISTING CONDITIONS.

6. REF. SHEET AS2.01 FOR DETAILS AND OTHER SITE IMPROVEMENT DETAILS.

7. FINISH GRADE AT PERIMETER OF THE BUILDING SHALL BE AS SHOWN ON SITE GRADING PLAN. REF. CIVIL DRAWINGS. SITE GRADES SHALL SLOPE UNIFORMLY AWAY FROM THE BUILDING AND INTERSECT WITH ELEVATIONS SHOWN.

8. REF. CIVIL DRAWINGS FOR ALL SITE DIMENSIONS

9. MAKE ALL SLOPES OF WALKS AND DRIVES COMPLY WITH FINISH ELEVATION SHOWN ON CIVIL DRAWINGS. THE LINE OF SLOPE SHALL FORM STRAIGHT LINES BETWEEN FINISH ELEVATIONS WHEN POSSIBLE AND GRADING CONTOURS WHEN STRAIGHT LINES ARE NOT POSSIBLE. VERIFY ALL ELEVATIONS AND DIMENSIONS WITH ARCHITECT ON JOB. REF. CIVIL.

10. ALL WALKS AND PLAZAS SHALL BE REINFORCED CONCRETE - REF. CIVIL FOR DETAILS. PROVIDE 1/2" EXPANSION JOINT AS SHOWN BY CIVIL OR AT 25'-0" O.C. MAX. AND AT ALL INSIDE CORNERS. PROVIDE TOOLED CONTROL JOINTS AT 5'-0" O.C.B.W. MAX OR AS SHOWN. SEE CIVIL FOR TYPICAL WALK AND JOINT DETAILS.

11. PROVIDE 1/2" EXPANSION JOINT ALONG BUILDING FOUNDATION OR BRICK WHERE CONCRETE WALK IS PRESENT. REF. CIVIL DRAWINGS FOR TYPICAL DETAIL.

12. PROVIDE 1/2" EXPANSION JOINT ALONG BACK OF CONCRETE CURB WHERE WALK IS PRESENT. REF. CIVIL DRAWINGS FOR CURB DETAILS

13. CONTRACTOR SHALL REMOVE ALL EXISTING OBSTRUCTIONS TO NEW CONSTRUCTION, PAVING, WALKS, AND ALL OTHER SITE IMPROVEMENTS INCLUDING BUT NOT LIMITED TO EXISTING TREES, FENCES, CONCRETE, ROCKS, DEBRIS, POSTS, VEGETATION, UTILITIES (AS NOTED), AND OTHER ITEMS AS NOTED FOR CONSTRUCTION. PRIOR TO REMOVING TREES, CONTRACTOR SHALL OBTAIN WRITTEN APPROVAL FROM OWNER.

14. REF. ELECTRICAL DRAWINGS FOR OUTDOOR LIGHTING REQUIREMENTS.

15. REF. CIVIL DRAWINGS FOR SITE GRADING PLAN, SITE UTILITY PLAN (INCLUDING FIRE HYDRANT LOCATIONS), EROSION CONTROL PLANS, AND CURB AND PAVING REQUIREMENTS AND DETAILS.

16. REF. CIVIL SHEETS FOR SITE GRADING CONTOURS. CONTRACTOR SHALL MAKE ALL CUTS AND FILLS REQUIRED TO ACCOMPLISH ALL WORK SHOWN. GRADE SITE TO TOLERANCES SPECIFIED.

17. REF. CIVIL DRAWINGS FOR THICKNESS AND LOCATIONS OF ALL TYPES OF PAVING.

18. FIRE LANE STRIPING SHALL COMPLY WITH CITY OF KYLE REQUIREMENTS AND NFPA.

19. PROVIDE DEDICATED STRUCTURAL CONCRETE FOUNDATION PAD FOR ALL ON-SITE ELECTRICAL TRANSFORMERS PER THE ELECTRICAL SERVICE PROVIDER REQUIREMENTS AND/OR SPECIFICATIONS.

20. PROVIDE DEDICATED STRUCTURAL CONCRETE FOUNDATION PAD FOR ALL ON-SITE GENERATORS AND ANY ASSOCIATED GENERATOR ENCLOSURES.

GENERAL SITE NOTES

PROPERTY DESCRIPTION

1) OWNER INFORMATION

   A. NAME : HAYS C.I.S.D.
   B. ADDRESS: 1700 LEHMAN RD, KYLE, TX 78640

2) PROPERTY INFORMATION

   A. NAME: LEHMAN HIGH SCHOOL
   B. ADDRESS: 1700 LEHMAN RD, KYLE, TX 78640

3) LEGAL DESCRIPTION

   S4826 - LEHMAN HIGH SCHOOL, BLOCK A, LOT PT 1

4) ZONING INFORMATION

   A. CATEGORY: AG - AGRICULTURAL
   B. TRANSLATED CATEGORY: XXXXXX
   C. ORDINANCE NUMBER: 438

PARKING REQUIREMENTS

AS OBTAINED FROM CITY ORDINANCE FOR CITY OF KYLE, THE REQUIRED NUMBER OF OFF STREET PARKING SPACES IS DETERMINED AS FOLLOWS:

1.5 SPACES FOR EACH CLASSROOM OR TEACHING STATION PLUS 1 ADDITIONAL SPACE FOR EACH 4 SEATS IN ANY AUDITORIUM, GYMNASIUM, OR OTHER PLACE OF ASSEMBLY, WHICHEVER IS GREATER.

1. EXISTING SPACES
   904 SPACES
   27 ACCESSIBLE SPACES
   931 TOTAL EXISTING SPACES

2. NEW SPACES
   67 SPACES ADDED
   2 ACCESSIBLE SPACES ADDED
   69 TOTAL SPACES ADDED

3. TOTAL SPACES
   971 SPACES
   29 ACCESSIBLE SPACES
   1000 TOTAL SPACES

PROPERTY & PARKING NOTES

Revision /	Date	
1	05/08/25	ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT
JASON R. HUCKABEE
STATE OF TEXAS

05.08.25

Huckabee
www.huckabee-inc.com
800.687.1229

ARCHITECTURAL SITE PLAN

Job No. 01954-08-01	Sheet No. ISSUE FOR BID AS1.01
Drawn By: YR/AB	
Date: 05/08/2025	

PLAN
TRUE
N

GENERAL NOTES

SECTION 1 - GENERAL INFORMATION AND DESIGN CRITERIA

SECTION 1.1- DOCUMENTS

1.1.1 Structural Drawings are not stand-alone documents and are augmented by technical specifications and must be coordinated with the complete set of contract documents.

1.1.2 Structural documents are protected by Copyright Law of the United States and are not to be used for any purpose other than construction of the building structure described in the contract documents at the geographic location shown.

1.1.3 General Notes and Typical Details apply throughout the project wherever conditions similar to those depicted exist and are not necessarily specifically referenced in the documents.

1.1.4 The Geotechnical Report referenced herein is not part of the Structural Documents. However, a copy should be obtained for reference during installation of foundations and subgrade preparation.

COORDINATION

1.1.5 Contractor is responsible for coordinating Structural Documents with other trades and disciplines in the contract documents. Some requirements are not known prior to issue and may change as layout and fabrication drawings are developed. Promptly report deviations and interferences with structural components for resolution by the Architect.

1.1.6 Contractor to verify dimensional location and depth of slab recesses and offsets with Architectural Drawings.

1.1.7 Contractor to verify size, weights, location, and details of structurally supported equipment and associated openings prior to fabrication of the supporting structure.

1.1.8 Contractor to verify size and location of floor and roof penetrations shown on structural drawings with other disciplines.

1.1.9 Submit for approval a composite drawing showing all proposed openings and sleeves through structural members for engineering review prior to or simultaneous with shop drawings for affected framing.

1.1.10 Contractor to verify dimensions, details, plumbness and squareness of existing structures meeting or tying into new construction.

1.1.11 Do not scale plans, details and sections for quantity, length or fit of materials.

REFERENCE ELEVATIONS

1.1.12 Heights of floor and roof decks and various framing components are given on the drawings relative to a reference elevation that is equivalent to a Mean Sea Level Elevation noted below. Contractor to verify against Civil grading plans and report discrepancies to Architect for resolution prior to construction.

Area A FFE = 100' - 0" / Civil 672.10
Area B FFE = 100' - 0" / Civil 671.67 (1)
Area C FFE = 103' - 4" / Civil 675.29
Area D FFE = 100' - 0" / Civil 671.96'
Area E FFE = 100' - 0" / Civil 672.10'

TEMPORARY BRACING

1.1.13 Structural systems are designed for final, in-place conditions only. Provide temporary bracing of structural components for conditions that will exist during construction and to meet all regulatory requirements for safety of workers.

1.1.14 Maintain temporary frame bracing until installation of permanent structural bracing elements, member connections and floor and roof diaphragms are complete.

SECTION 1.2- CODES AND STANDARDS

1.2.1 Building Code of jurisdiction - 2021 International Building Code

1.2.2 Structural Concrete Code - American Concrete Institute (ACI) 318

1.2.3 Structural Masonry Code – The Masonry Society (TMS) 402

1.2.4 Structural Steel Code - American Institute of Steel Construction (AISC) 360 (and 341 where applicable)

1.2.5 Structural Cold-Formed Steel Code - American Iron and Steel Institute (AISI) S100

SECTION 1.3- DESIGN CRITERIA

1.3.1 Structure Risk Category III

1.3.2 Live Loads

Occupancy or Use	Uniform (psf)	Concentrated (lbf)	Notes (1)
Ground Level, Typical	100		
Roof, Typical	20		
Schools, Upper Levels	80	1,000	
Stairs and Exitways	100	300	
Stair and Elevator Lobbies	100		

Notes:
(1) Typical concentrated loads applied over 2.5-foot square area to structural members. (1)

1.3.3 Roof Snow Loads

Ground Snow Load, Pg	5	psf

1.3.4 Superimposed Dead Loads

Typical Structured Level	15	psf
Typical Roof	30	psf

Notes:
Superimposed dead loads do not include self-weight of members shown in structural drawings.

1.3.5 Wind Loads

Ultimate design wind speed, Vult	115 mph
Allowable design wind speed, Vasd	89 mph
Serviceability wind speed (25 Year)	80 mph
Exposure Classification	C
Internal Pressure Coefficient	0.18

See component and cladding wind load diagram

1.3.6 Seismic Loads

Seismic Importance Factor, Ie	1.25
Mapped Spectral Acceleration, Ss	0.052
Mapped Spectral Acceleration, S1	0.029
Site Class	D
Design Spectral Acceleration, Sds	0.055
Design Spectral Acceleration, Sd1	0.046
Seismic Design Category	A
Analysis Procedure Used: Equivalent Lateral Force	
Basic Seismic Force Resisting System: Steel Systems Not Detailed for Seismic	
Response Modification Coefficient, R	3.0
Seismic Response Coeff, Cs	0.01

1.3.7 Rain Loads

Rain Intensity, i	5.36	in/hr

1.3.8 Other Concentrated Loads

Location	Load-pounds	Area
Steel Roof Deck	200	1 sq ft
Stair Treads	300	4 sq in

Notes:
Concentrated loads apply to any location on supporting structure, separately from (not in addition to) uniform live loads, except as noted otherwise.

1.3.9 Assumed weights and locations of structurally supported equipment are indicated on the framing plans.

1.3.10 Pedestrian Guardrail - 50 lbf/ft horizontal and vertical, or 200 lbf concentrated at top, any direction.

STRUCTURAL DEFLECTIONS

1.3.11 Live Load - Floor and roof systems are designed to limit vertical deflections due to live loads to (Clear Span)/360 or less. Attachments of architectural and mechanical components to or between floor and roof structures do not allow for live load deflections of this magnitude to occur without causing distress or deformity to the components.

1.3.12 Dead Load - Floor and roof systems are designed to limit vertical deflections due to total loads to (Clear Span)/240 or less. Some deflections may occur incrementally as loads are placed on the structure, and in the case of concrete structures, may occur over an extended time period. Attachments of architectural and mechanical components do not allow for dead load deflections that may occur after installation. For example, significant deflections may occur when mechanical systems are charged with water or other coolants.

1.3.13 Structural cambers, where shown on the drawings, are generally for estimated dead load deflections. Components attached to cambered beams or trusses should not be connected in a manner that would restrict vertical deflection prior to the placement of dead loads. Where steel beams are connected with self-tensioning devices, final tensioning must be delayed until structural dead loads are in place.

1.3.14 Panelized Wall Systems- Attachments of curtainwall and other wall panel systems must allow for differential vertical deflection of 0.375 inches, and horizontal deflection of H/400 between adjacent floors. Wall cladding attachments do not transfer lateral reactions to bottom flanges of steel beams, joists, or trusses except, 1) where specifically shown on the Structural Drawings, or 2) special bracing is provided by the wall supplier/installer to transfer lateral reactions to the floor slab.

SECTION 2 - FOUNDATIONS AND RELATED EARTHWORK

SECTION 2.1- GEOTECHNICAL REPORT

2.1.1 Design of foundations and structural components in contact with soil is based on recommendations given in the following:

Report Author : UES Professional Solutions
Report Number : A251017
Date of Report : April 10, 2025

2.1.2 Refer to the Geotechnical Report for subgrade conditions that may be encountered during foundation installation and site preparation.

SUBGRADE CRITERIA UNDER BUILDING SLABS

2.1.3 Coordinate under-floor drainage and waterproofing requirements with architectural and plumbing drawings and recommendations of the geotechnical report.

EARTH RETENTION SYSTEMS

2.1.4 Design of earth retention systems is not included in Structural Documents. Refer to Geotechnical Report for requirements.

SECTION 2.2- STRAIGHT SHAFT PIERS

2.2.1 Design Criteria:

Bearing Stratum	: NA
Top of Stratum Elevation (for Bidding Purposes Only)	: 15'-0"
Allowable End Bearing	: 0 psf below 30 ft penet
Positive Side Friction	: 1200 psf at 15 - 25 ft 1500 psf at 25 - 35 ft
Upheaval Side Friction	: 1500 psf
Upheaval Design Depth	: 15 ft
Negative Side Friction	: 1000 psf at 15 - 25 ft 1200 psf at 25 - 35 ft

2.2.2 Pier depths indicated are for bidding purposes only. Actual pier depths may vary depending on depth to bearing stratum.

2.2.3 Remove overpour at tops of piers ("mushrooms") to the required diameter.

SECTION 2.3- FOUNDATION WALLS

2.3.1 Do not backfill walls until lateral bracing structures at top and bottom of each wall are constructed and have attained specified design strength.

2.3.2 Do not backfill perimeter below grade walls over 3 ft until temporary lateral bracing structure at top of each wall is constructed and the wall has attained specified design strength. Walls shall remain braced until permanent lateral bracing structure has attained specified design strength.

SECTION 3 - STRUCTURAL CONCRETE

SECTION 3.1 - CONCRETE FORMS

3.1.1 Formed Voids - Provide retained void spaces between bottom of structural members and subgrade as follows:

Grade Beams and Pilasters	12"
Slab on Void	12"

3.1.2 Form vertical faces of grade beams, pilasters, pier caps, and other vertical foundation element.

SECTION 3.2- STEEL REINFORCING

3.2.1 Reinforcing bars shall be deformed. Strength of bars shall be as follows:

Deformed Bar Anchors	70 ksi
Other bars, UNO	Grade 60

SPLICING OF REINFORCING BARS

3.2.2 Top bars in beams or slabs shall be spliced at midspan between supports, unless noted otherwise.

3.2.3 Bottom and middle bars in beams or slabs shall be spliced at supports, unless noted otherwise.

3.2.4 Vertical bars in walls shall be spliced at top of concrete above floors, unless noted otherwise.

LAPPED SPLICE LENGTHS

3.2.5 Lap reinforcing 30 bar diameters at splices of slab-on-grade and temperature and shrinkage reinforcing unless noted or detailed otherwise.

3.2.6 Tension splice lengths shall be calculated in accordance with ACI 318. Use Class B splices unless noted otherwise.

3.2.7 Welded Wire Reinforcement splice length (overlap), measured between outermost cross wires of each fabric sheet, shall be at least one spacing of cross wires plus 2 inches, but in no case less than 6 inches.

CONCRETE COVER TO REINFORCING

3.2.8 Clearance from face of concrete to face of reinforcing:

Piers	3"
Formed Grade Beams,	2" top, 2" sides, 3" bottom
Pier Caps, Pilasters	2"
Slabs	3/4" interior, 1 1/2" exterior exposure

Notes: Above dimensions apply unless noted otherwise in details

PLACEMENT OF REINFORCING

3.2.9 Place first bar of slab reinforcing parallel to side 2 inches from a free edge or half of required bar spacing from face of edge beam.

3.2.10 Single layer reinforcing in walls shall be placed at center of walls unless noted otherwise.

3.2.11 Place reinforcing in toppings or in slabs poured on steel deck at center of slab unless noted otherwise.

SECTION 3.3- CONCRETE MIX DESIGNS

3.3.1 Concrete Mix Schedule:

a. Concrete type is NWC unless noted otherwise. NWC refers to normalweight concrete having maximum cured density of 145 PCF.
b. Where w/c ratio is not indicated in the Concrete Mix Schedule, it shall be as necessary to meet strength requirements.
c. Where the w/c ratio is shown, it shall be adhered to regardless of strength requirements.
d. "Strength" is required compressive cylinder strength at an age of 28 days.
e. See specification for additional information at auger cast in place piles.
f. Exposure classes are noted as defined in ACI 318. Exposure classes for concrete mixes are F0, S0, W0, and C0 unless noted otherwise.

Description of Use	Strength psi	Agg Size	Max w/c	Air Content	Exposure Class	Notes
ACIP Piles	5000	---	---	---	--	---
Grade Beams, Pilasters, Foundation Walls	4000	1"	0.55	4.5%	F1	---
Structural Beams and Slabs	4000	1"	0.45	---	--	---
Slab on Steel Composite Deck	3500 (1)	3/4"	0.45	---	--	
Housekeeping Pads	3000	3/4"	---	---	--	
Light Pole Base	5000	1"	0.45	5%	F2	---
Exterior Slabs	5000	3/4"	0.40	5%	F3	---

SECTION 3.4- CONCRETE SLABS

3.4.1 Slab Placed on Carton Form

Location	Thickness	Reinforcing
Typical	8 inches UNO	Per details (a)

a) Reinforcement shall be placed in accordance with typical details unless shown otherwise.

3.4.2 Slabs on Composite Steel Deck

Composite Slab Schedule:

Type Mark	Overall Thickness	Typ Slab Reinf	Notes/ Addl Top Reinf
CA	6.5"	WWR 6x6-W2.5xW2.5	#5(10-0)@12 over girders

Notes:
1. See typical details for reinforcing placement and additional reinforcing over girders. "Girders" refers to interior beams oriented parallel to deck.
(2) Slab types correspond to deck type (see Composite Steel Deck).

3.4.3 Housekeeping Pads

Pad Thickness:	4.0 inches
Pad Reinforcing:	WWF6x6-W2.1xW2.1
Pad Thickness:	6.0 inches
Pad Reinforcing:	WWF6x6-W3.5xW3.5

Reinforcing shall be centered in the pad. Refer to mechanical drawings for pad locations, plan dimensions and thickness required at specific locations.

3.4.4 Slabs on Geofoam

Location	Thickness	Reinforcing
Raised seating & Ramps	4 inches	#3 @ 12 EW

a) Reinforcement shall be centered in slab.
b) EPS Geofoam Insulation shall be type EPS15 meeting the requirements of ASTM D6817. Geofoam blocks to be adhered to base slab and to one another with a non-solvent based adhesive.

SECTION 3.5- DRILLED IN ANCHORS

3.5.1 Drill holes with rotary impact hammer drill using carbide tipped bits. Drill bits shall be of the diameter as specified by the anchor manufacturer. All holes shall be drilled perpendicular to the concrete or masonry surface.

3.5.2 Embedded items: Identify position of reinforcing steel and other embedded items prior to drilling holes for anchors. Exercise care in drilling to avoid damaging existing reinforcing or embedded items. Notify the Engineer if reinforcing steel or other embedded items are encountered during drilling. Take precautions as necessary to avoid damaging electrical and telecommunications conduit, and gas lines.

3.5.3 Base Material Strength: Unless otherwise specified, do not drill holes in concrete or masonry until concrete, mortar, or grout has achieved full design strength. Do not install adhesive anchors in concrete that is placed less than 21 days prior. (from ACI 318 requirement)

TESTING

3.5.4 Continuous special inspection is required for adhesive anchors. Remove and replace misplaced or malfunctioning anchors. Clean and fill empty anchor holes and patch failed anchor locations with high-strength nonshrink, nonmetallic grout. Anchors that fail to meet proof load or installation torque requirements shall be regarded as malfunctioning.

EXPANSION, UNDERCUT, SCREW AND ADHESIVE ANCHORS

3.5.5 Concrete base material: provide anchors of size and type shown with ICC-ES or IAPMO-UES compliance required

Expansion Anchors:	Hilti KWIK Bolt TZ2 (ICC-ES ESR-4266)
Undercut Anchors:	Hilti HDA Undercut Anchors (ICC-ES ESR-1546)
Screw Anchors:	Hilti Kwik HUS-EZ (ICC-ES ESR-3027)
Adhesive Anchors:	Hilti HIT-HY 200 Safe Set System (ICC-ES ESR-3187) for use with Hilti HIT-Z Rod, HAS-E Rod, & Hollow Drill Bit Hilti HIT-RE 500-V3 Safe Set System (ICC-ES ESR-3814) for use with Hilti HAS-E Rod, Hollow Drill Bit & Hilti Roughening Tool Hilti HIT HY-200 (ICC-ES ESR 3187)

3.5.6 Grout filled CMU (Concrete Masonry Unit) base material: provide anchors of size and type shown with ICC-ES or IAPMO-UES compliance required

Screw Anchors:	Hilti Kwik HUS EZ (ICC-ES ESR-3056)
Adhesive Anchors:	Hilti HIT-HY 270 (ICC-ES ESR-4143)

INSTALLATION

3.5.7 Perform anchor installation in accordance with manufacturer's printed installation instructions (MPII).

3.5.8 Protect threads from damage during anchor installation.

3.5.9 Contractor to arrange for a manufacturer's field representative to provide installation training for all products to be used prior to commencement of work. Only trained installers shall perform post-installed anchor installation. A record of training shall be kept on site and made available upon request.

3.5.10 Adhesive anchors installed horizontally or upwardly inclined shall be qualified in accordance with ACI 355.4 requirements for sensitivity to installation direction.

SECTION 4 - STRUCTURAL MASONRY

SECTION 4.1- GENERAL

4.1.1 See Architectural Drawings and Specifications for details and dimensions of masonry work.

4.1.2 Grout lifts at reinforced masonry walls shall be accomplished in accordance with TMS 402/602.

SECTION 4.2- STRUCTURAL PROPERTIES

4.2.1 Required compressive strength of structural assembly = 2000 psi

4.2.2 Load-bearing Concrete Masonry Units: ASTM C90 Normal-weight
Required net area compressive strength = 2000 psi

4.2.3 Mortar: ASTM C270 Type S

4.2.4 Grout: ASTM C476
Required 28-day compressive strength of grout 2000 psi

SECTION 4.3- REINFORCING

JOINT REINFORCEMENT

4.3.1 Horizontal joint reinforcing shall be "Ladder Type" 9 gage welded wires spaced 16 inches on center vertically.

4.3.2 Provide prefabricated "L" and "T" shaped sections at wall intersections.

4.3.3 Lap horizontal wires at least 8" at splices.

BAR REINFORCEMENT

4.3.4 Reinforcing bars shall conform to ASTM A615 Grade 60.

4.3.5 Bar reinforcing shall be lapped at splices per schedule in typical details. Stagger splices in adjacent horizontal bars at least 4'-0".

4.3.6 Vertical reinforcing in cells to be grouted shall be placed using fabricated bar positioners to maintain location within cell.

4.3.7 Grout solid cells below adjacent grade or finish floor elevation and cells with vertical or horizontal bar reinforcement.

STRUCTURAL WALLS

4.3.8 Typical wall reinforcing for load-bearing, structural CMU walls is noted in structural wall elevations.

NON-STRUCTURAL WALLS

4.3.9 Unless shown otherwise on plans or details, reinforcing for CMU walls not shown in the structural drawings shall be as follows:

Wall Thickness	Vert Reinf	Dowels	Max Height
6 inches	1 #4 @ 48 max	1 #4(0-10/4-0) @ 48 max	18' - 0"
8 inches	1 #5 @ 48 max	1 #5(0-10/4-0) @ 48 max	24' - 0"

Notes:
a) Align and lap dowels with vertical wall reinforcing.
b) At wall openings, see wall opening reinforcing schedule in typical details for reinforcing of jambs and lintels.
c) Post-installed dowels are acceptable at non-structural CMU. Drill & embed dowels 9 bar diameters minimum with adhesive.

4.3.10 Grout and reinforce the first cell at corners, ends of walls, and each side of a control joint with 1 vertical bar for 6- or 8-inch CMU walls or 2 vertical bars for 12-inch CMU walls. Jambs adjacent to openings in structural masonry are to be grouted and reinforced per applicable details.

4.3.11 Install single course depth bond beam with at least one horizontal bar at the top of CMU walls.

SECTION 4.4- CONTROL JOINTS

4.4.1 Do not locate vertical control joints in CMU walls through an opening or within the jamb or lintel bearing adjacent to an opening. Control joints must be vertical from the wall foundation to the top of wall.

4.4.2 See plans for control joint locations in load-bearing CMU walls.

SECTION 4.5- REQUIRED SUBMITTALS

4.5.1 Prior to construction, contractor is to submit CMU reinforcing layout and fabrication drawings for review. Submittal shall contain the following information:
a) CMU wall thickness
b) Material properties
c) Plans and wall elevations that show wall reinforcing details, openings, beam pockets, and lintels
d) Control joint locations

SECTION 5 - STRUCTURAL STEEL

SECTION 5.1- STRUCTURAL FRAME

5.1.1 Structural Steel Properties:

High Strength Steel Use for W Shapes and WT's	ASTM A992 Grade 50
Structural Steel (Normal Strength) Use for Angles, Channels, and Plates, UNO	ASTM A36
Steel Pipes	ASTM A53, Grade B
Hollow Structural Sections (HSS)	ASTM A500, Grade C
Erection Bolts	ASTM A307
High Strength Bolts	ASTM F3125, A325N UNO ASTM F3125, A490N where shown in drawings
Anchor Rods	ASTM F1554 Gr. 36 UNO
High Strength Anchor Rods	ASTM F1554 Gr. 105 where shown in drawings
Headed Stud Anchors	ASTM A29 Gr. 1010-1020, Type B

5.1.2 Continuity Plates (Full Depth column stiffeners aligned with beam flanges, or Full Depth beam stiffeners aligned with column flanges) shall match the steel grade of the base member.

WELDING

5.1.3 Unless otherwise noted, angles, plates, rods, and miscellaneous framing shall be welded at contact joints and supports. Weld sizes shall conform to AWS D1.1 minimums, except where noted otherwise.

5.1.4 Where fillet weld sizes are not indicated on weld symbols, fillet size shall be 1/16th inch smaller than thickness of thinner of materials being joined.

5.1.5 Complete penetration welds are indicated by notation "CJP" on weld symbols, partial penetration by "PJP".

STRUCTURAL BOLTS

5.1.6 Bolts indicated on details shall be 3/4 inch diameter, unless noted otherwise.

5.1.7 Bolts shall be tightened by the AISC "Snug Tight" method unless noted otherwise.

MISCELLANEOUS

5.1.8 Edge angles at perimeters of floors and roofs shall be continuous and spliced per typical details.

5.1.9 Unless noted otherwise, steel members shall be hot dip galvanized at exterior conditions. Field welds to be repaired in accordance with ASTM A780.

COMPOSITE STEEL BEAMS

5.1.10 Beams shall have shear studs spaced at 2 feet maximum on center, unless specifically indicated to have zero studs.

5.1.11 Composite steel beams do not require shoring during placement of concrete slab, unless noted otherwise.

SHEAR STUDS

5.1.12 Shear studs shall be fusion-welded, headed studs of high strength steel.

5.1.13 Unless noted otherwise, studs shall have a shank diameter of 3/4-inch. See details for length of studs measured after welding.

SECTION 5.2- STEEL JOISTS

5.2.1 Joist Legend:
22K6 - SJI K-SERIES JOIST.
24LH8 - SJI LH-SERIES JOIST.
22KSP - SPECIAL DESIGN FOR SPECIFIED LOADING.

5.2.2 Unless noted or detailed otherwise, typical seat depths shall be:
K or KCS Series - 2-1/2 inches
LH or DLH Series - 5 inches

5.2.3 Joists shall be designed for concentrated dead or live load in addition to required uniform dead and live loads, as follows at top and bottom chords:
Joists: 250 lb. placed at any panel point.

5.2.4 Design joists supporting mechanical units to support a concentrated load equal to 60% of the weight shown on plan at any joist panel point. Design joists supporting more than one mechanical unit to support a concentrated load equal to 60% of the sum of the weights shown on plan at any joist panel point. These concentrated loads are in addition to the loads noted above.

5.2.5 See loading diagram for net uplift requirements due to wind load.

5.2.6 Joist loads shown on drawings are nominal Loads per building code and have not been multiplied by ASD (Allowable) nor LRFD (Strength) load multipliers unless specifically noted otherwise.

5.2.7 Deflection shall not exceed L/240 for total load or L/360 for short term loads (live, snow, or wind).

SECTION 5.3- COMPOSITE STEEL DECK

5.3.1 Basis of design assumes the deck is continuous over three spans. Contractor shall review deck properties at conditions that do not meet this assumption.

5.3.2 Contractor shall provide composite metal decking to meet the following criteria:
1. Decking alone shall be capable of supporting the wet weight of concrete plus construction loads without requiring intermediate shoring for all span conditions on the project, unless noted otherwise.
2. Composite slab and deck system shall be capable of supporting design loads indicated on the drawings for all span conditions on the project.
3. Deck thickness, indicated by gage in Composite Steel Deck Schedule, is a minimum and shall be increased as necessary to meet these requirements, at no additional cost to the Contract.

5.3.3 Composite Steel Deck Schedule:
a. See framing plans for location of deck types.
b. Deck yield strength: Fy = 40 ksi minimum

5.3.4

Type Mark	Deck Height	Minimum Gage	Minimum Ip-in4	Minimum In-in4	Minimum Sp-in3	Minimum Sn-in3
CA	3.0"	20	.921	.931	.524	.573

5.3.5 Composite floor system minimum load capacity requirements:

Type	Superimposed Uniform Load (psf)	Concentrated Load (lbs) *
CA	353 psf (8' - 6" span)	See Section 1

*Concentrated load acting on area 2.5 ft x 2.5 ft; not acting simultaneously with uniform load.

5.3.6 See technical specifications for composite deck connection to supporting structure.

SECTION 5.5- STEEL ROOF DECK

5.5.1 Steel Roof Deck Schedule:
a. See framing plans for location of deck types.
b. Typical deck yield strength: Fy = 40 ksi minimum

Deck Type Mark	Deck Gage	SDI Profile	Deck Height	Min. Ip in4	Min. In in4	Min. Sp in4	Min. Sn in3	Deck Finish
RA	22	WR	1.5"	.147	.170	.171	.179	G-60
RB	20	DR	3.0"	.725	.946	.464	.506	G-60

5.5.2 Steel Roof Deck Connection Schedule:
a. Shear Capacity listed is allowable (0.6W, 0.7E) and is considered acting in combination with wind uplift pressures.
b. W/N = sheet width/no. connections each sheet.
c. Deck Connections are noted on Plans.

Conn Type Mark	Conn @ Supports (W/N)	Parallel Edges (In)	Sidelap Conn No./Span	Reqd Shear Capacity (PLF)
I	24/4	8	7	336 @ 10'-0" span
II	36/7	6	4	457 @ 6'-0" span

5.5.3 Support and parallel edge connections shall be 5/8" visible diameter arc spot welds. Sidelap connections shall be no. 10 hex head screws.

SECTION 6 - DEFERRED APPROVALS

6.1.1 The following items require deferred approval from the enforcement agency. See specifications for additional design services to be provided by Contractor.
1. Cold formed metal framing
2. Stairs and railings
3. Steel connection design
4. Pre-engineered Metal Building (PEMB)

6.1.2 The design of the above items is by the Contractor/Manufacturer. Contractor/Manufacturer must prepare all necessary calculations and drawings per the Building Code of Jurisdiction under the supervision of a Structural Engineer, registered in the state in which the project is located, and obtain all necessary plan check approvals from the enforcement agency.

6.1.3 Fabrication and installation of the above items shall not be started until detailed plans, specifications and engineering calculations have been accepted and signed by the Architect or Structural Engineer of Record and the signature of the Architect or Professional Engineer who has been delegated responsibility covering the work shown on a particular plan or specification, and approved by the enforcement agency.

6.1.4 Submittal documents for deferred items shall be submitted to the registered design professional in responsible charge, who shall review them and forward them to the building official with a notation indicating that the deferred documents have been reviewed and that they have been found to be in general conformance with the design of the building. The deferred items shall NOT be installed until their design and submittal documents have been approved by the building official.

Notes continue on S1.2

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Date 05/07/25

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

Project:

CHERYL R. STEWART 117468 LICENSED PROFESSIONAL ENGINEER 5-7-2025

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GENERAL NOTES

PACKAGE VOLUME

Job No. 01954-08-01
Drawn By: LAFP
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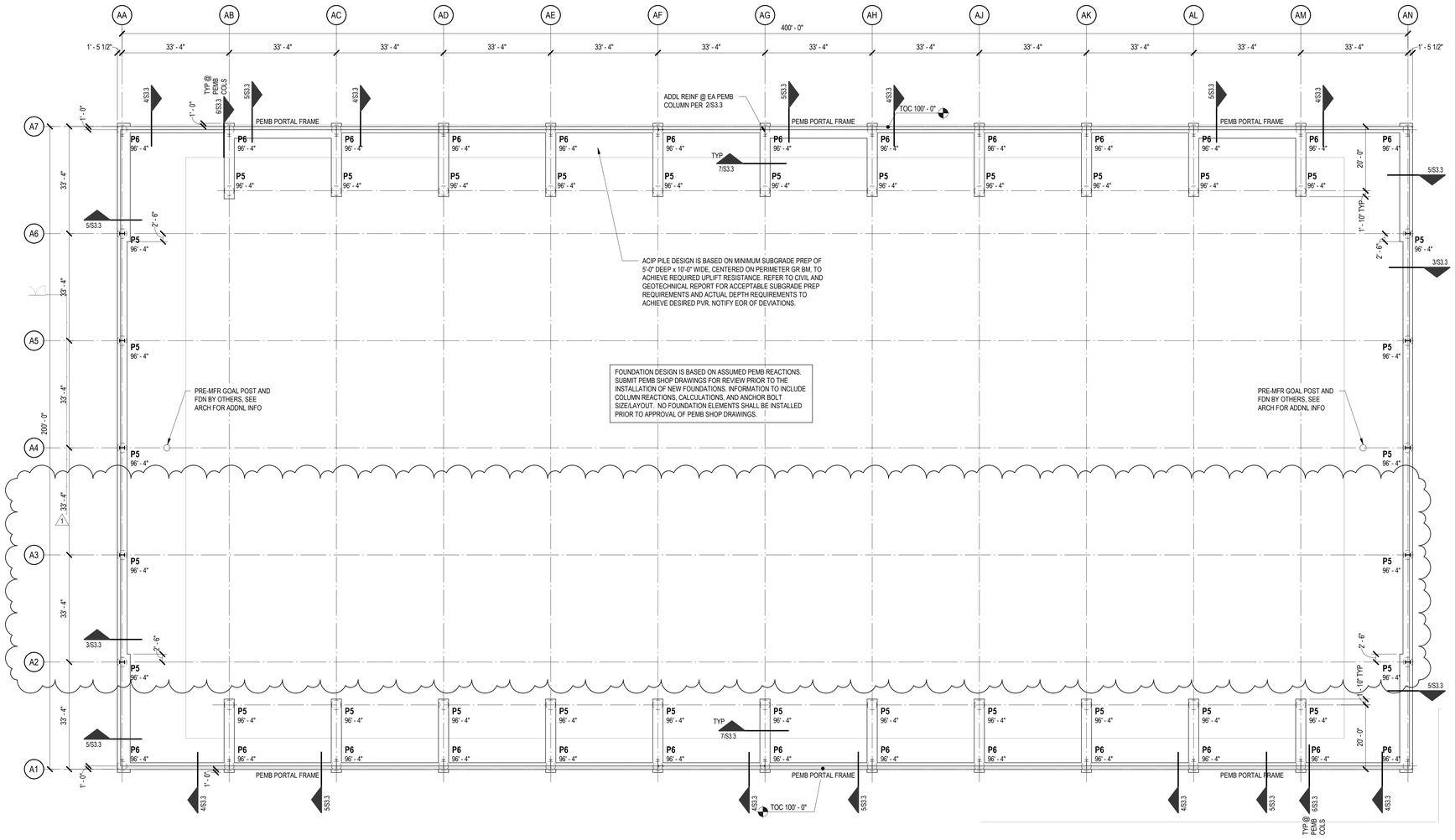


1 FOUNDATION PLAN - AREA A

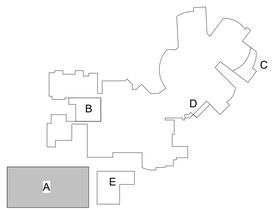
1/16" = 1'-0"

NORTH

FOUNDATION PLAN NOTES

1. SEE PLAN FOR TOP OF CURB ELEVATION (RELATIVE TO DATUM 100'-0").
2. SHEET INDEX:

GENERAL NOTES	S1.1
TYPICAL CONC DETAILS	S3.1, S3.2
PIER SCHEDULE	S3.1
STEEL COLUMN SCHEDULE	S5.1
VERTICAL BRACES	S6.1



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FOUNDATION PLAN - AREA A

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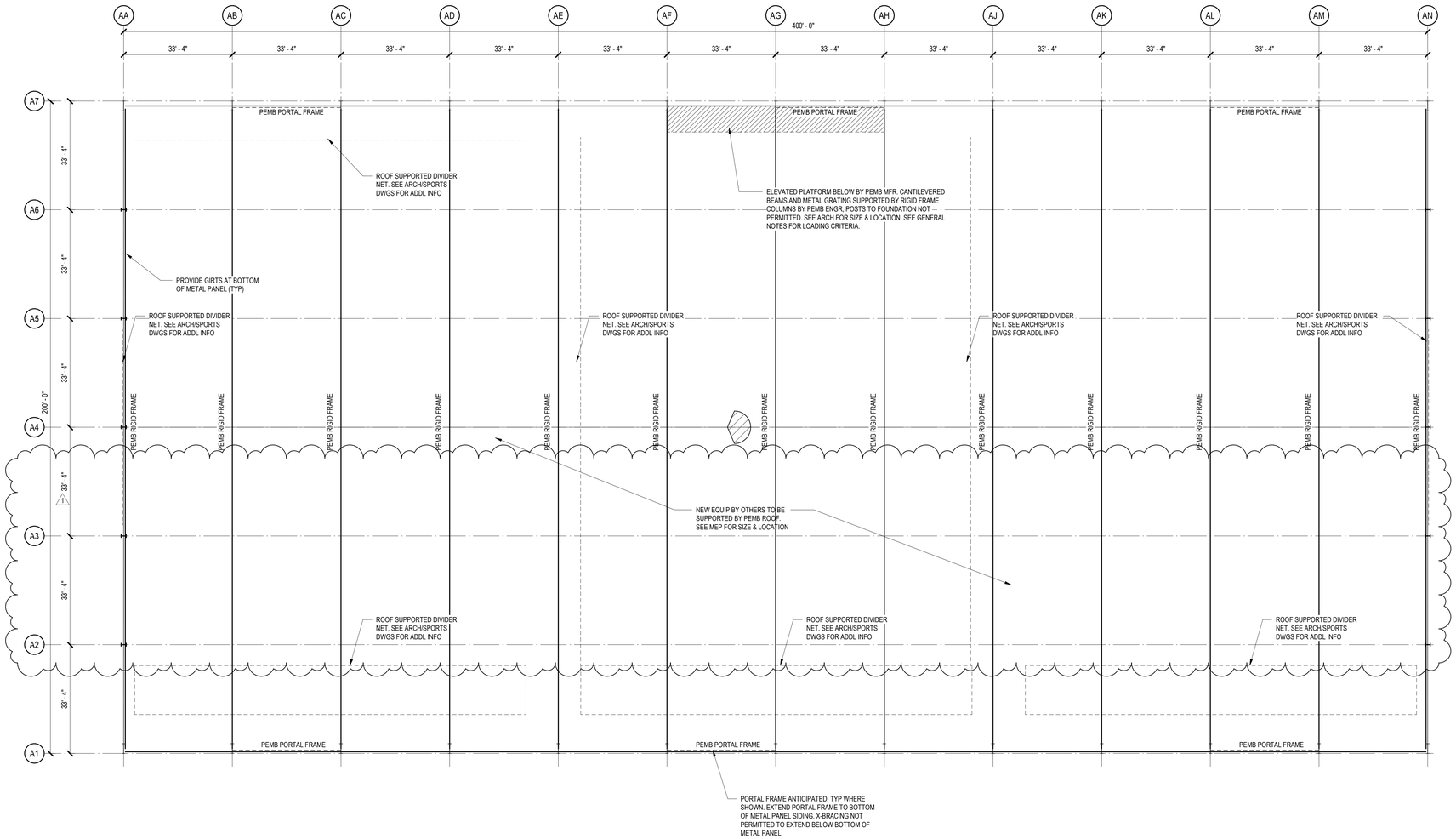


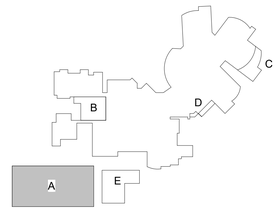
1 ROOF FRAMING PLAN - AREA A

1/16" = 1'-0"

NORTH

PEMB PLAN NOTES

1. PEMB SUPPLIER SHALL BE RESPONSIBLE FOR THE ENTIRE DESIGN OF THE STEEL SUPERSTRUCTURE INCLUDING FLOORS ABOVE GRADE, ROOFING SUPPORT, FASCIAS, FACADE SUPPORT, ANCHOR BOLT LAYOUT & DESIGN, TEMPORARY BRACING, LATERAL ANALYSIS AND RELATED WORK.
2. REFER TO INCLUDED STRUCTURAL NARRATIVE FOR ADDITIONAL INFORMATION REGARDING PEMB DESIGN CRITERIA.



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ROOF FRAMING PLAN - AREA A

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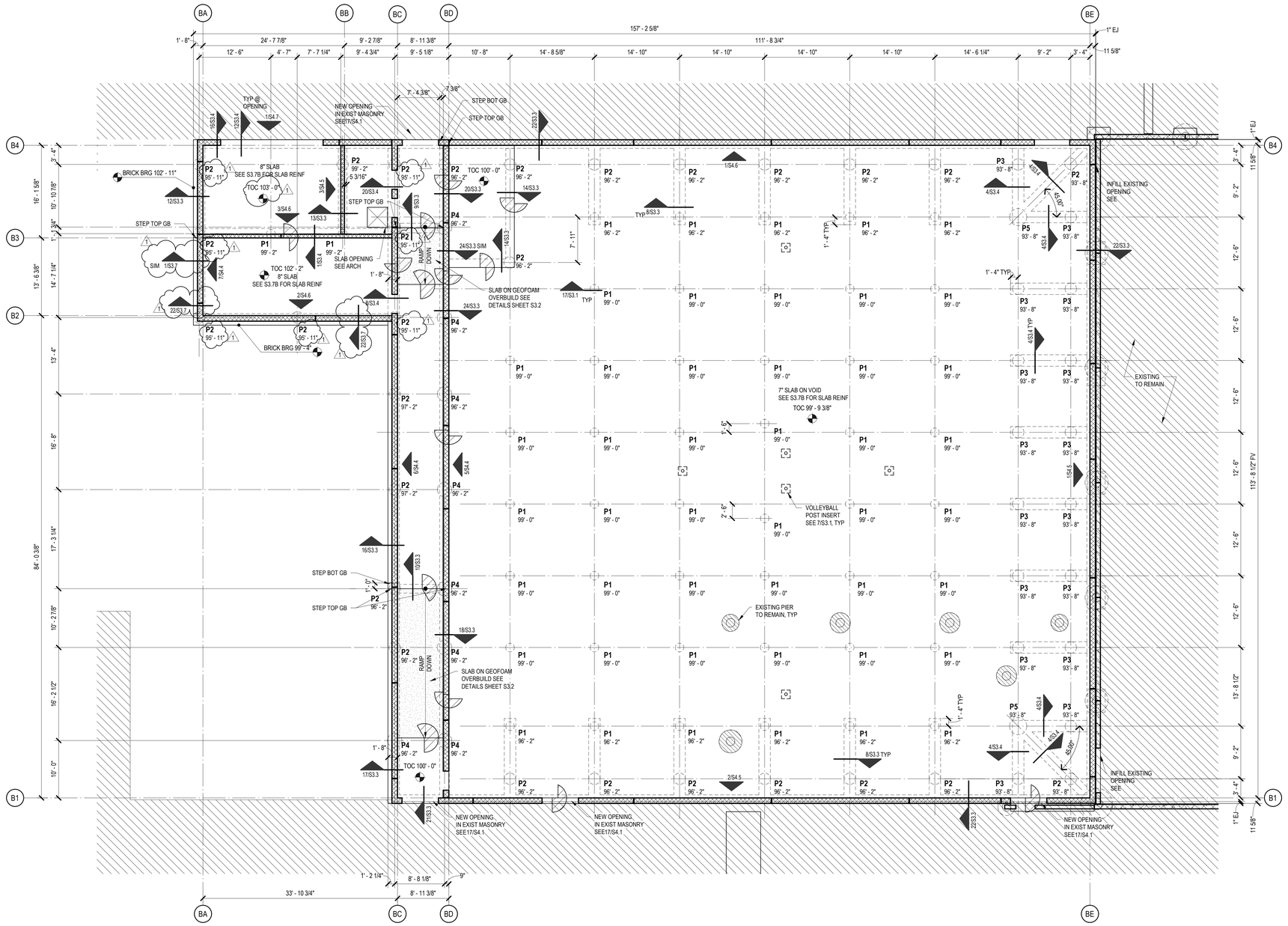


1 FOUNDATION PLAN - AREA B

1/8" = 1'-0"

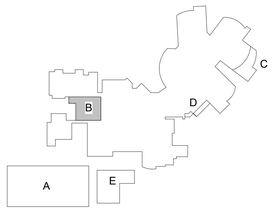
NORTH

FOUNDATION PLAN NOTES

1. FINISH FLOOR ELEVATION IS SHOWN ON PLAN (RELATIVE TO DATUM 100'-0").
2. TOP OF CONCRETE SLAB IS FINISH FLOOR UNLESS SHOWN OTHERWISE.
3. SHEET INDEX:

GENERAL NOTES	S1.1
TYPICAL CONC DETAILS	S3.1, S3.2
PIER SCHEDULE	S3.1
STEEL COLUMN SCHEDULE	S5.1
VERTICAL BRACES	S6.1

4. TYPICAL CONCRETE SLAB THICKNESS IS 8" (OVERALL) UNLESS NOTED OTHERWISE.
5. BRICK LEDGE ELEVATION IS 8" BELOW FINISH FLOOR UNLESS SHOWN OTHERWISE.
6. TOP AND BOTTOM REINFORCING IN FLAT SLAB SHALL BE PLACED IN PROPER SEQUENCE - SEE SLAB REINFORCEMENT PLANS AND DETAIL.



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FOUNDATION PLAN - AREA B

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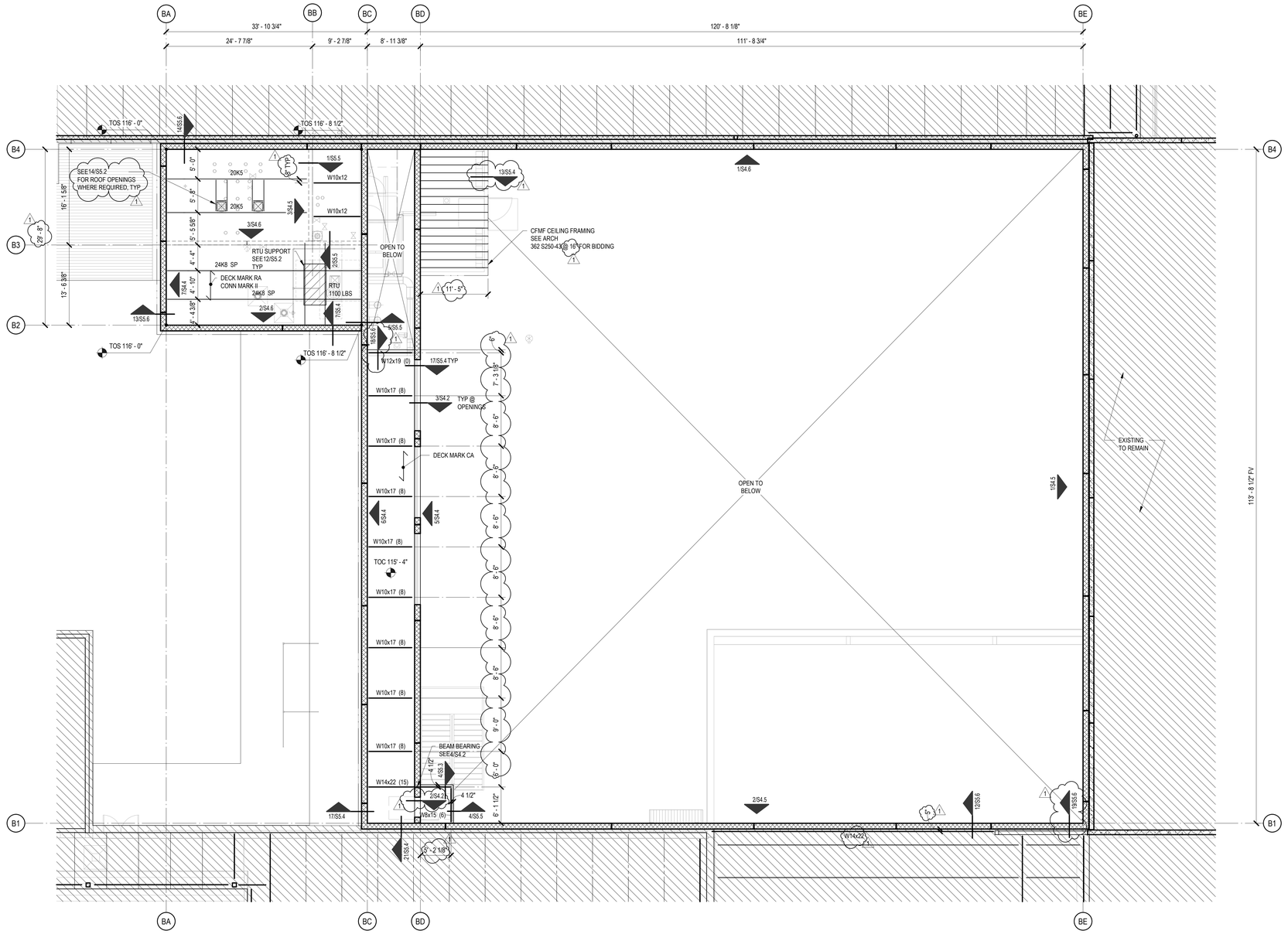


1 LEVEL 2 FRAMING PLAN - AREA B

1/8" = 1'-0"

NORTH

LEVEL 2 FRAMING PLAN NOTES

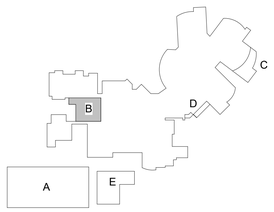
1. FINISH FLOOR ELEVATION IS SHOWN ON PLAN (RELATIVE TO DATUM 100'-0").
2. TOP OF CONCRETE SLAB IS FINISH FLOOR UNLESS SHOWN OTHERWISE.
3. SHEET INDEX:

GENERAL NOTES	S1.1
STEEL COLUMN SCHEDULE	S5.1
MASONRY TYPICAL DETAILS	S4.1, S4.2
MASONRY WALL ELEVATIONS	S4.3, S4.4, S4.5, S4.6, S4.7
STEEL TYPICAL DETAILS	S5.1, S5.2, S5.3

4. UNLESS SHOWN OTHERWISE, STEEL BEAMS ARE CENTERED ON AND EQUALLY SPACED BETWEEN COLUMN CENTERLINES AND WALLS.
5. NUMBER OF SHEAR STUDS IS NOTED IN PARENTHESES ( ) ADJACENT TO BEAM SIZES. SEE TYPICAL DETAILS FOR LAYOUT REQUIREMENTS OF STUDS.

ROOF PLAN NOTES

1. TOP OF ROOF STRUCTURE IS SLOPED FOR DRAINAGE. SEE TOP OF STEEL ELEVATIONS NOTED ON FRAMING PLANS, SLOPES SHALL BE UNIFORM BETWEEN COLUMN CENTERLINES AND WALLS UNO.
2. TOP OF STEEL ELEVATIONS SHOWN ON PLAN ARE BOTTOM OF ROOF DECK (TOP OF BEAM OR JOIST). ELEVATIONS ARE SHOWN RELATIVE TO DATUM 100'-0" UNO. SEE GENERAL NOTES FOR MORE INFO.
3. UNLESS NOTED OTHERWISE, STEEL JOISTS/BEAMS SHALL BE CENTERED ON AND EQUALLY SPACED BETWEEN COLUMN CENTERLINES.
4. JOISTS SUPPORTING MECHANICAL EQUIPMENT SHALL BE DESIGNED FOR TYPICAL ROOF LOADING PLUS A CONCENTRATED LOAD OF 60% OF INDICATED EQUIPMENT WEIGHT PLACED AT ANY PANEL POINT.
5. ADDITIONAL LOADS FOR ROOF JOISTS ARE ALSO SPECIFICED IN THE STRUCTURAL DETAILS AND ARE IN ADDITION TO THE DESIGN LOADS AND ANY ADDITIONAL LOADS NOTED ON THE STRUCTURAL FRAMING PLANS.
6. SEE S1.5 FOR COMPONENTS AND CLADDING WIND PRESSURES, INCLUDING JOIST NET UPLIFT DESIGN REQUIREMENTS



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LEVEL 2 FRAMING PLAN - AREA B

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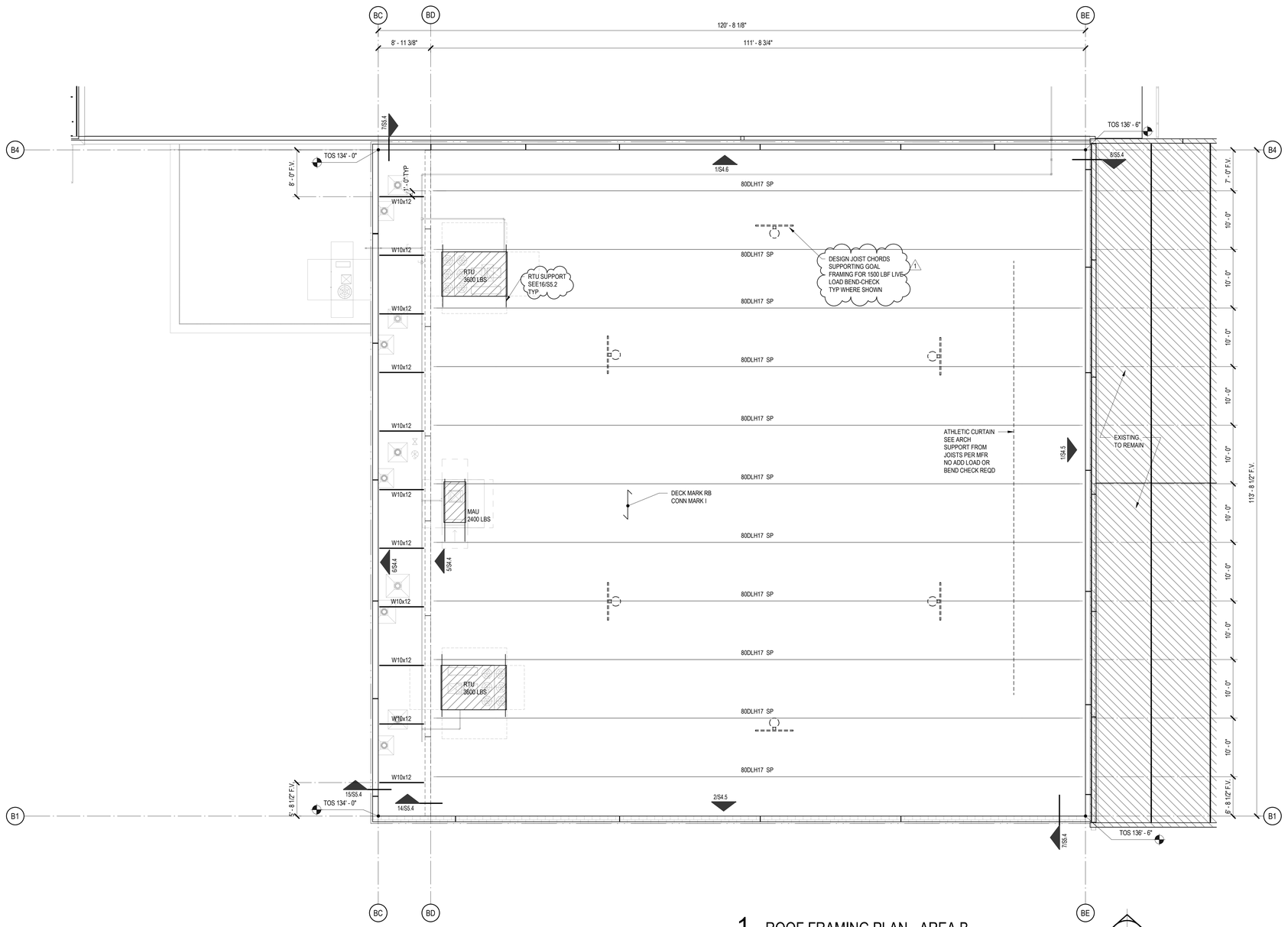

1 ROOF FRAMING PLAN - AREA B

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NORTH

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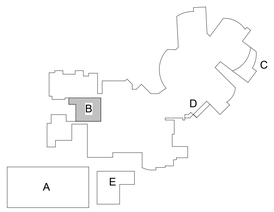
5. ADDITIONAL LOADS FOR ROOF JOISTS ARE ALSO SPECIFICED IN THE STRUCTURAL DETAILS AND ARE IN ADDITION TO THE DESIGN LOADS AND ANY ADDITIONAL LOADS NOTED ON THE STRUCTURAL FRAMING PLANS.
6. SEE S1.5 FOR COMPONENTS AND CLADDING WIND PRESSURES, INCLUDING JOIST NET UPLIFT DESIGN REQUIREMENTS
7. DO NOT CAMBER JOISTS FOR LIVE LOAD DEFLECTION. 1
8. CAMBER OF JOISTS SHALL NOT EXCEED 60% OF DEAD LOAD DEFLECTION.



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ROOF FRAMING PLAN - AREA B

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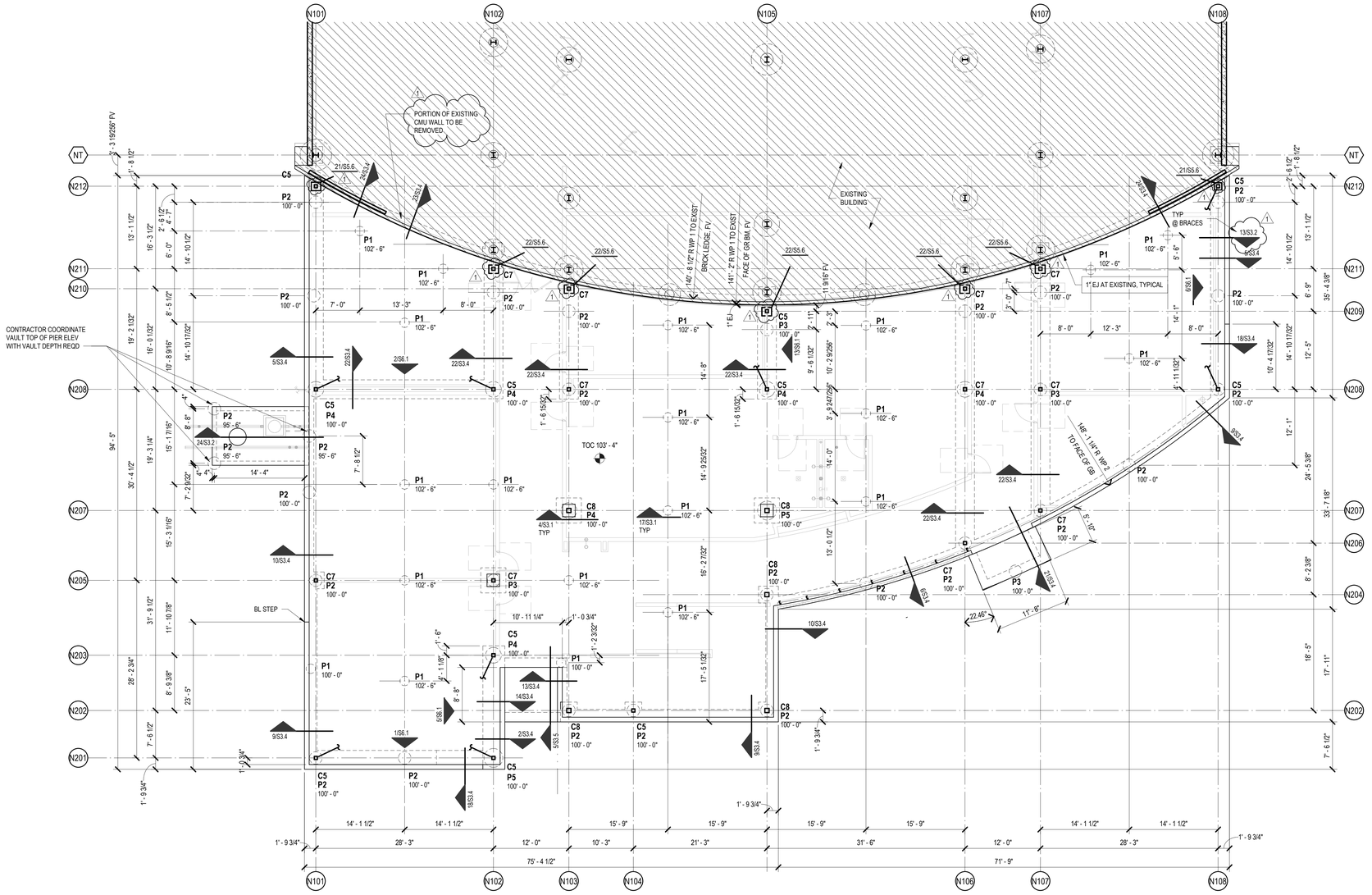

1 FOUNDATION PLAN - AREA C

1/8" = 1'-0"

NORTH

FOUNDATION PLAN NOTES

1. FINISH FLOOR ELEVATION IS SHOWN ON PLAN (RELATIVE TO DATUM 100'-0").
2. TOP OF CONCRETE SLAB IS FINISH FLOOR UNLESS SHOWN OTHERWISE.
3. SHEET INDEX:

GENERAL NOTES	S1.1
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PIER SCHEDULE	S3.1
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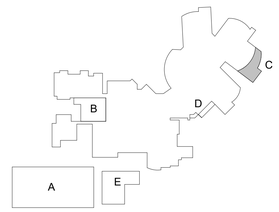

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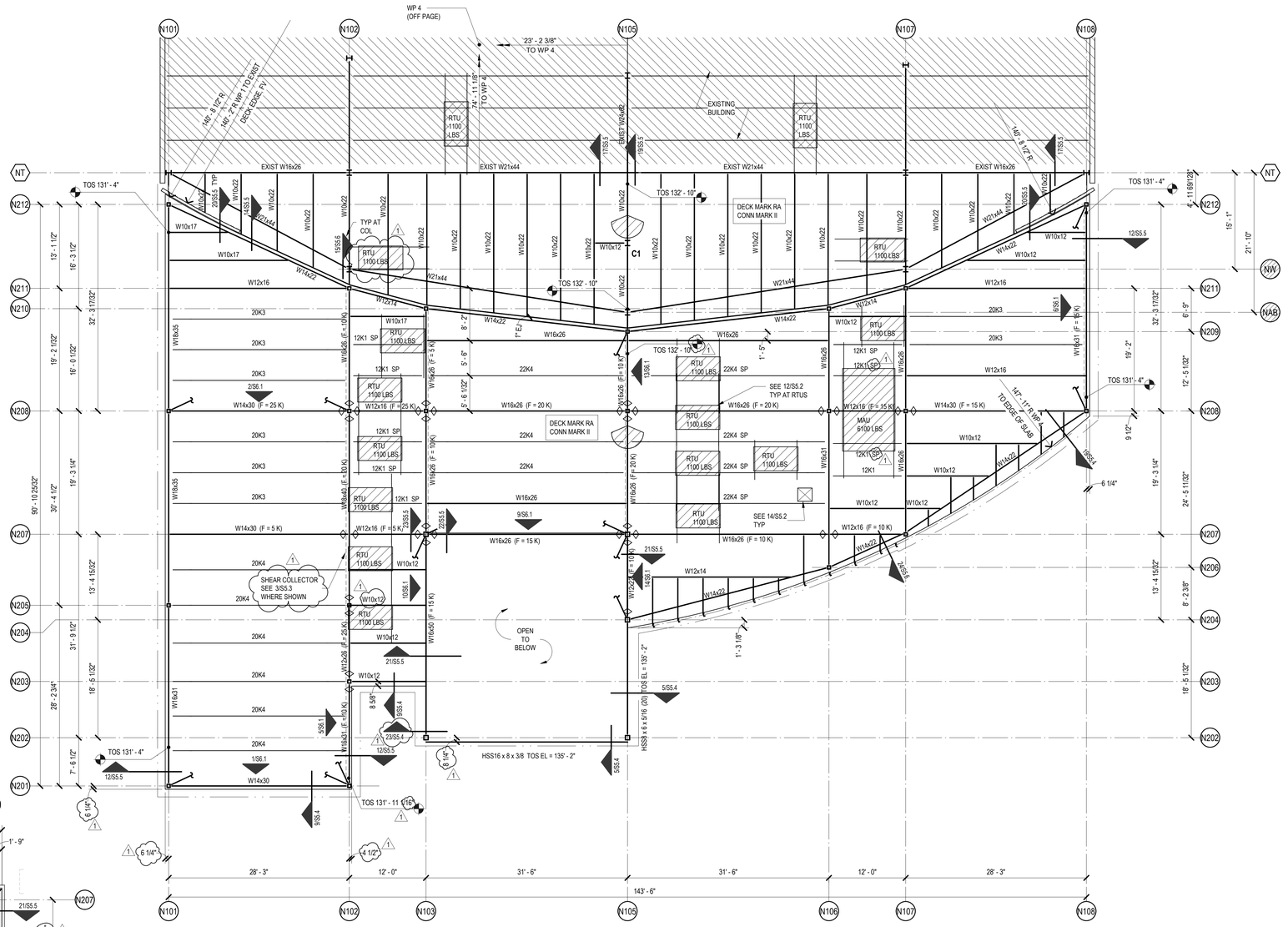

1 ROOF FRAMING PLAN - AREA C

1/8" = 1'-0"

NORTH

ROOF PLAN NOTES

1. TOP OF ROOF STRUCTURE IS SLOPED FOR DRAINAGE. SEE TOP OF STEEL ELEVATIONS NOTED ON FRAMING PLANS, SLOPES SHALL BE UNIFORM BETWEEN COLUMN CENTERLINES AND WALLS UNO.
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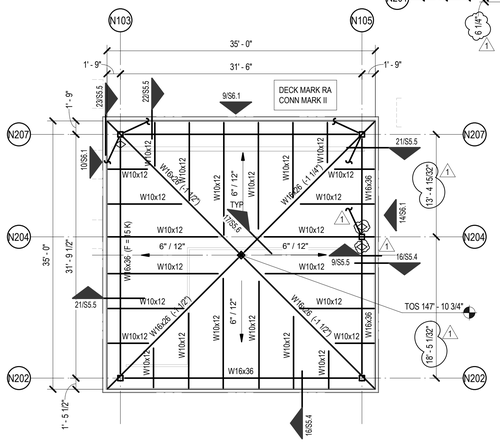
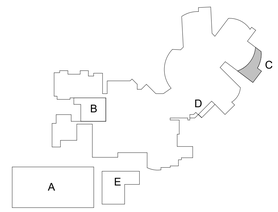
6. SEE S1.5 FOR COMPONENTS AND CLADDING WIND PRESSURES, INCLUDING JOIST NET UPLIFT DESIGN REQUIREMENTS



2 HIGH ROOF FRAMING PLAN - AREA C

1/8" = 1'-0"



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ROOF FRAMING PLAN - AREA C

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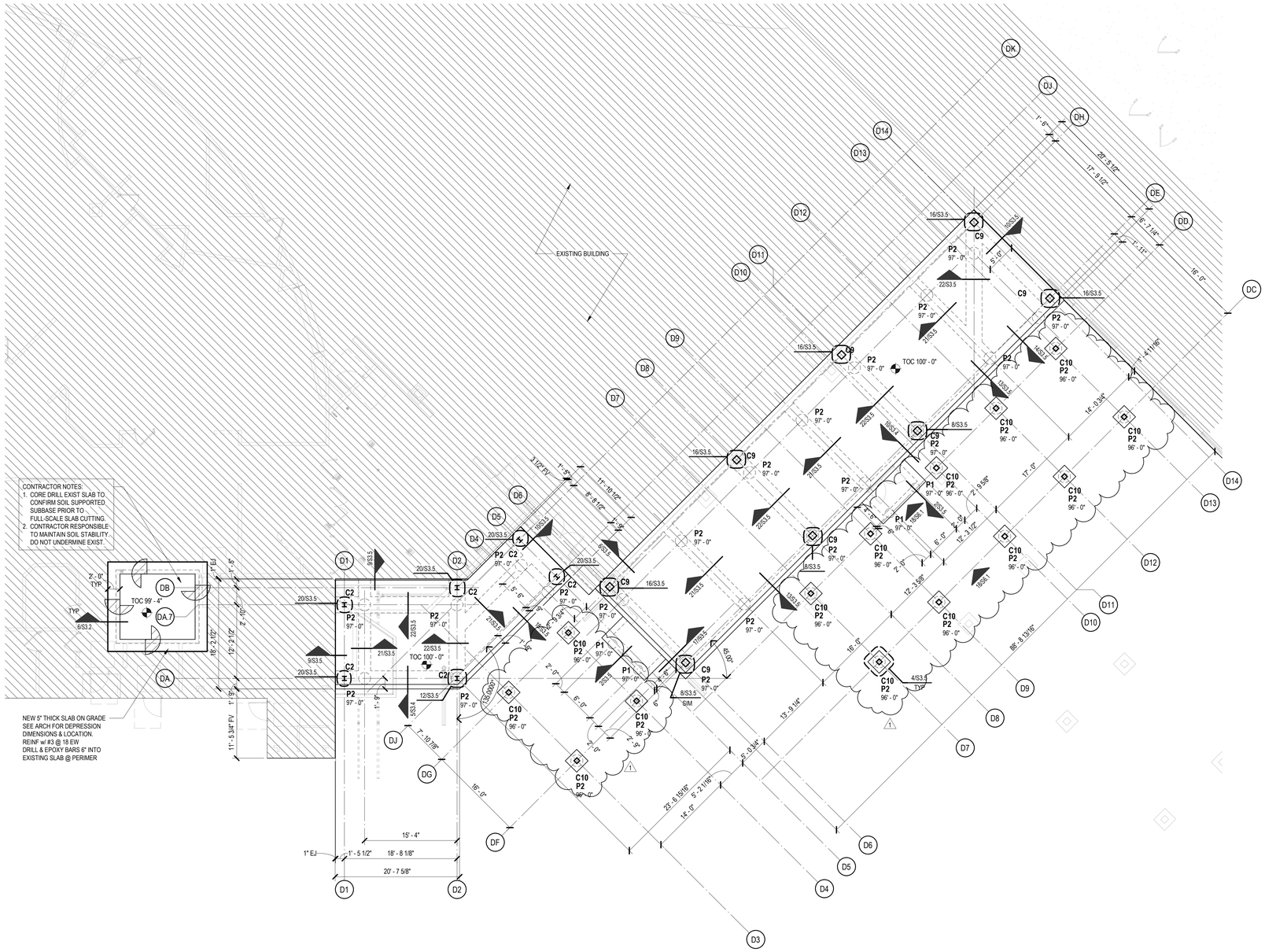
Date: 04/22/2025	



1 FOUNDATION PLAN - AREA D

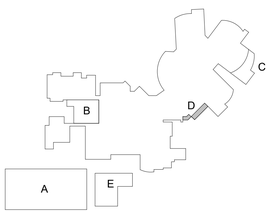
1/8" = 1'-0"

FOUNDATION PLAN NOTES

1. FINISH FLOOR ELEVATION IS SHOWN ON PLAN (RELATIVE TO DATUM 100'-0").
2. TOP OF CONCRETE SLAB IS FINISH FLOOR UNLESS SHOWN OTHERWISE.
3. SHEET INDEX:

GENERAL NOTES	S1.1
TYPICAL CONC DETAILS	S3.1, S3.2
PIER SCHEDULE	S3.1
STEEL COLUMN SCHEDULE	S5.1
VERTICAL BRACES	S6.1

4. TYPICAL CONCRETE SLAB THICKNESS IS 8" (OVERALL) UNLESS NOTED OTHERWISE.
5. BRICK LEDGE ELEVATION IS 8" BELOW FINISH FLOOR UNLESS SHOWN OTHERWISE.
6. TOP AND BOTTOM REINFORCING IN FLAT SLAB SHALL BE PLACED IN PROPER SEQUENCE - SEE SLAB REINFORCEMENT PLANS AND DETAIL.

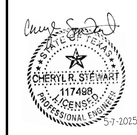


KEY PLAN

L.A. FUESS PARTNERS, INC.
Structural Engineers
3333 Lee Parkway, Suite 300 • Dallas, TX 75219
LAFP PROJ. NO. 24081 FIRM REG. NO. F-537

Revision /	Date	
1	05/07/25	Addendum 1

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX



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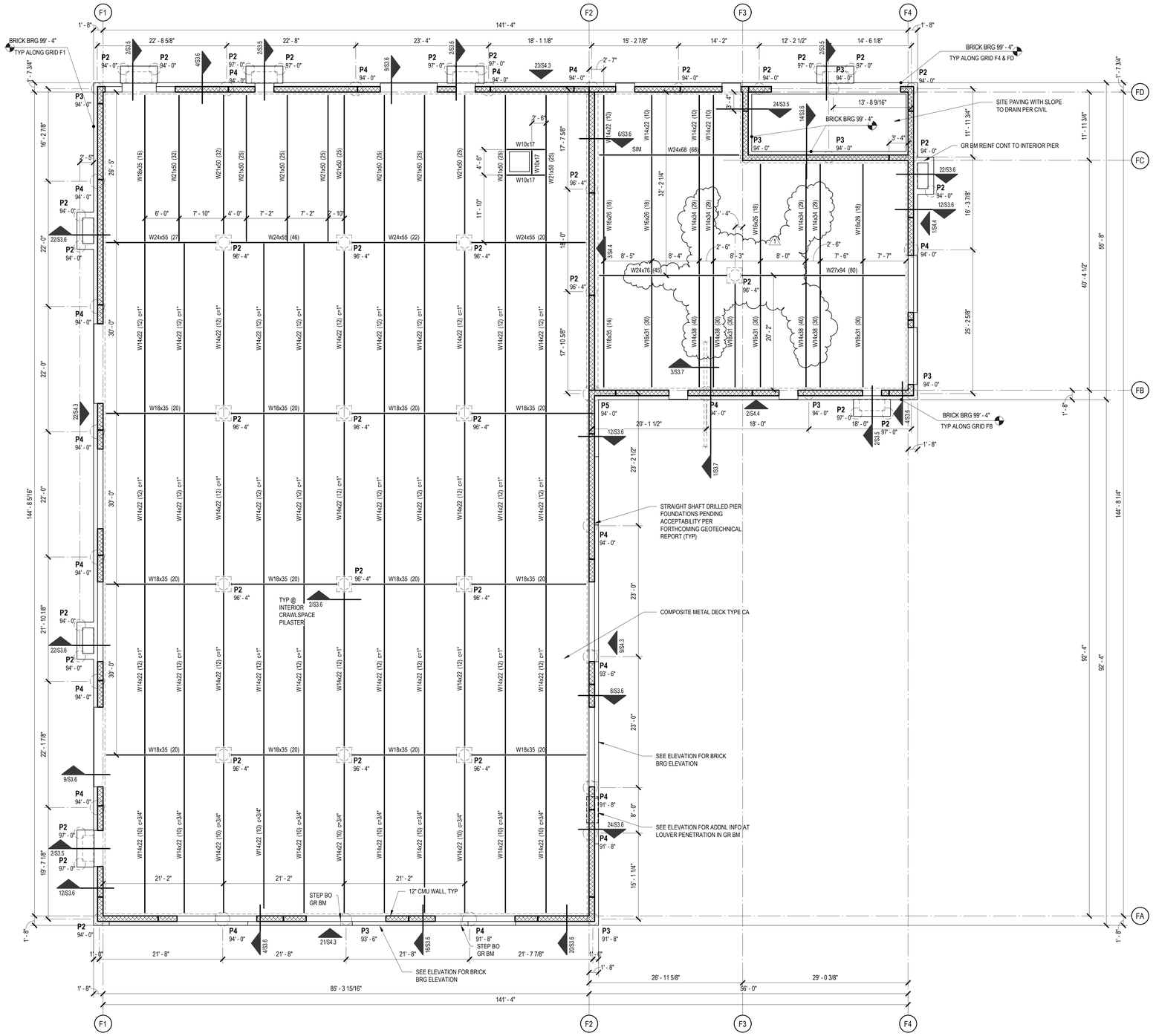
FOUNDATION PLAN - AREA D

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S2.1D1
Drawn By: LAFP	
Date: 04/22/2025	



1 FOUNDATION PLAN - AREA E

1/8" = 1'-0"

NORTH

FOUNDATION PLAN NOTES

1. FINISH FLOOR ELEVATION IS SHOWN ON PLAN (RELATIVE TO DATUM 100'-0").
2. TOP OF CONCRETE SLAB IS FINISH FLOOR UNLESS SHOWN OTHERWISE.
3. SHEET INDEX:

GENERAL NOTES	S1.1
TYPICAL CONC DETAILS	S3.1, S3.2
PIER SCHEDULE	S3.1
STEEL COLUMN SCHEDULE	S5.1
VERTICAL BRACES	S6.1

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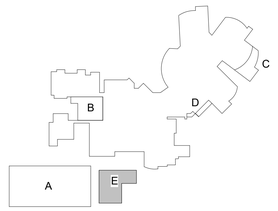
6. TOP AND BOTTOM REINFORCING IN FLAT SLAB SHALL BE PLACED IN PROPER SEQUENCE - SEE SLAB REINFORCEMENT PLANS AND DETAIL.



KEY PLAN

L.A. FUESS PARTNERS, INC.
Structural Engineers
3333 Lee Parkway, Suite 300 • Dallas, TX 75219
LAFP PROJ. NO. 24081 FIRM REG. NO. F-537

Revision /	Date	
1	05/07/25	Addendum 1

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

CHERYL R. STEWART
117468
LICENSED
PROFESSIONAL ENGINEER
5-7-2025

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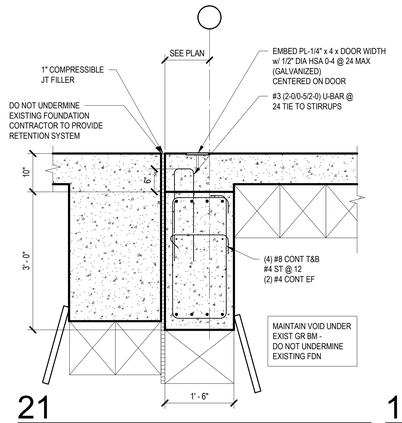
FOUNDATION PLAN - AREA E

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S2.1E1
Drawn By: LAFP	
Date: 04/22/2025	



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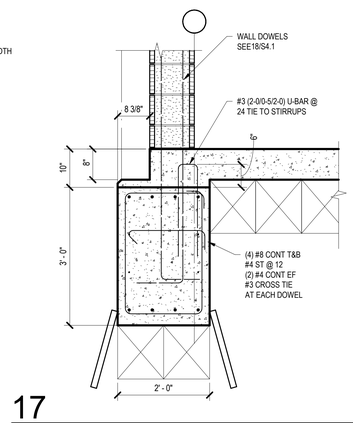

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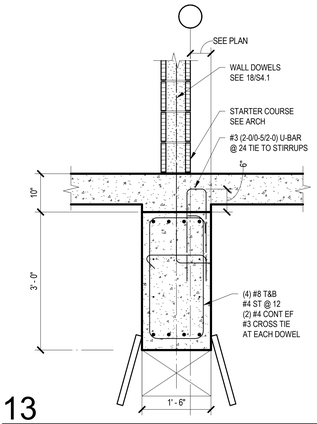

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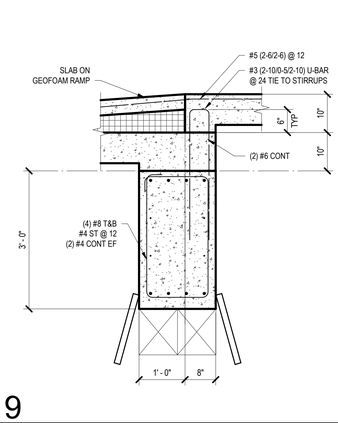

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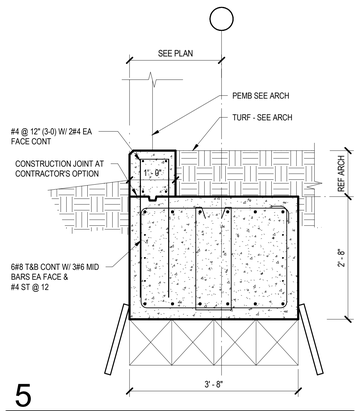

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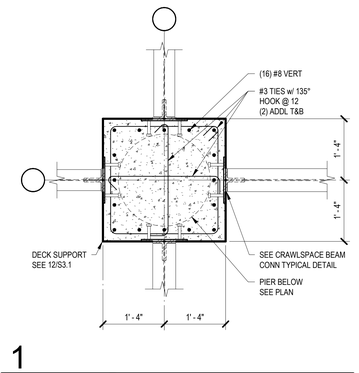

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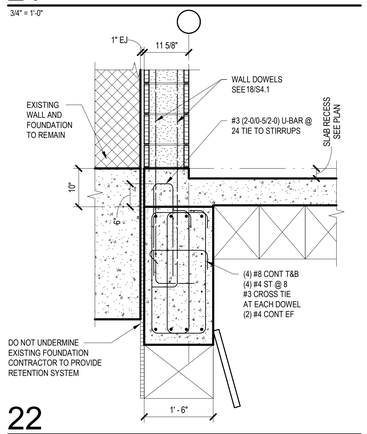

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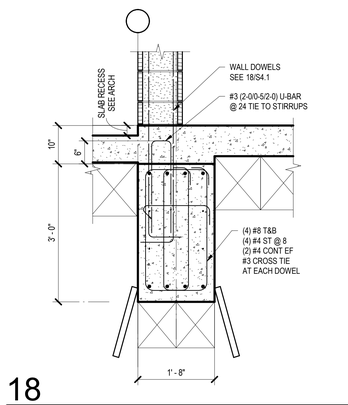

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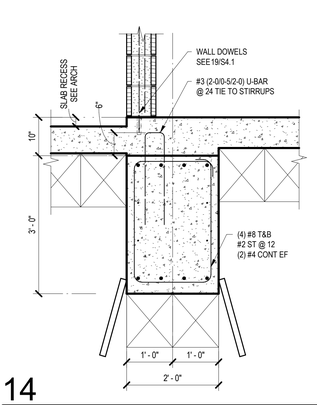

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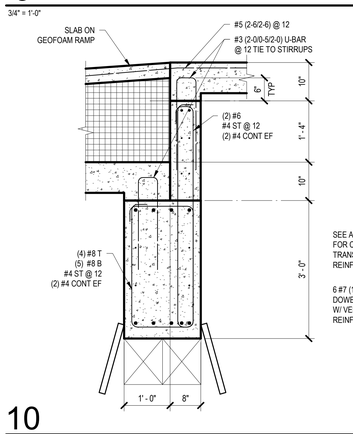

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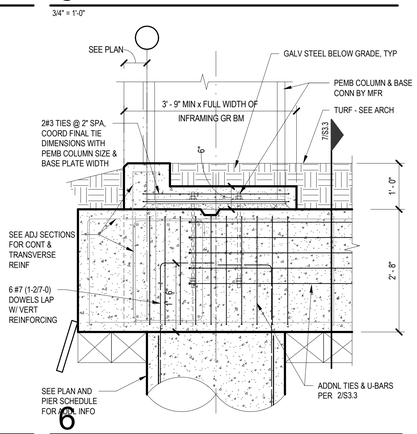

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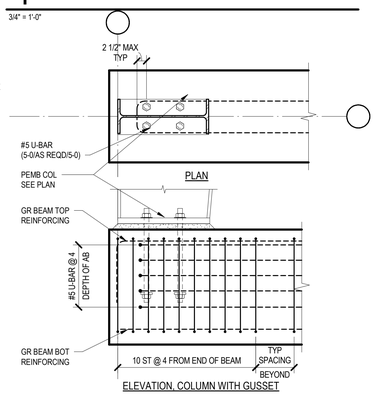

2 GR BM REINF @ PEMB COLUMN

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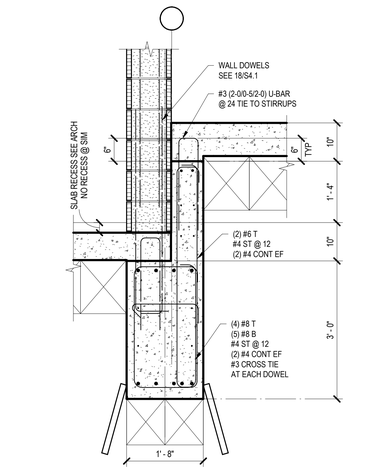

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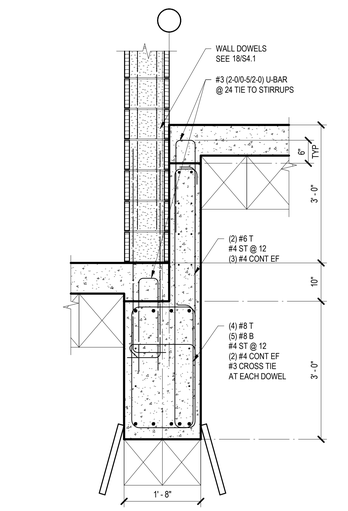

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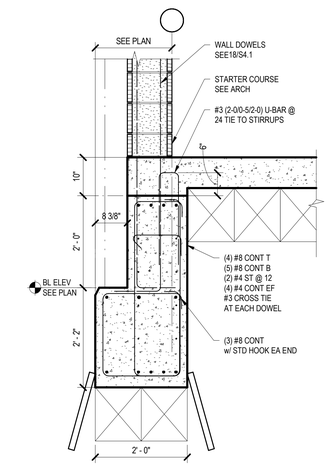

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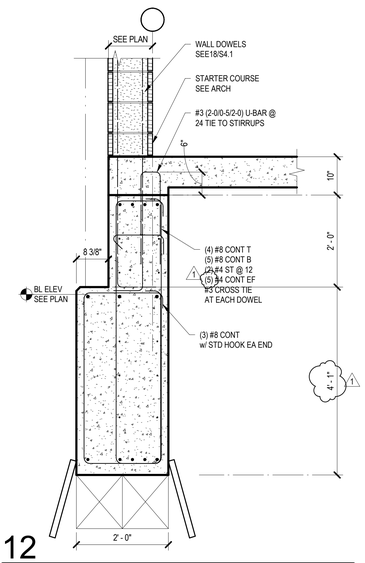

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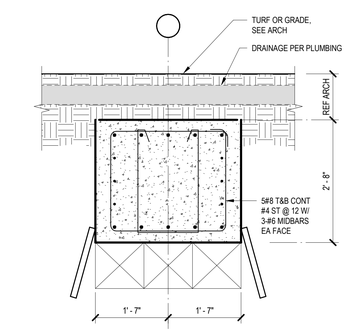

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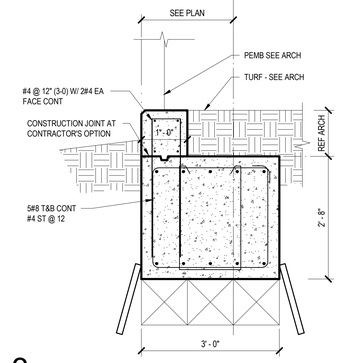

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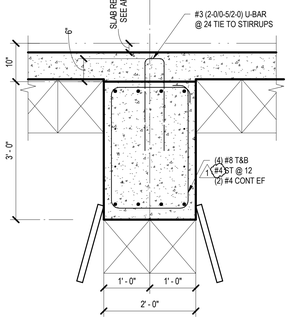

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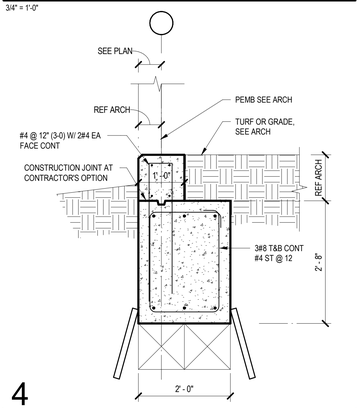

4

3/4" = 1'-0"

Revision / 1: Addendum 1
Date: 05/07/25

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

CHERYL R. STEWART
117498
LICENSED
PROFESSIONAL ENGINEER
05-7-2025

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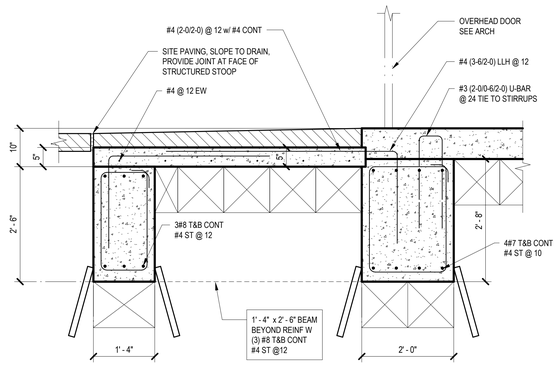
CONCRETE DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S3.3
Drawn By: LAFP	
Date: 04/22/2025	

L.A. FUESS PARTNERS, INC.
Structural Engineers
3333 Lee Parkway, Suite 300 • Dallas, TX 75219
LAFP PROJ. NO. 24081 FIRM REG. NO. F-537



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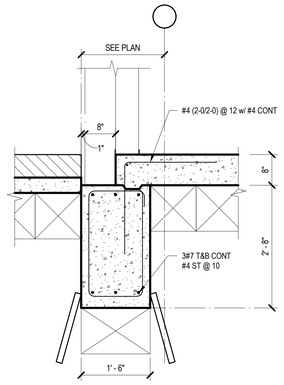

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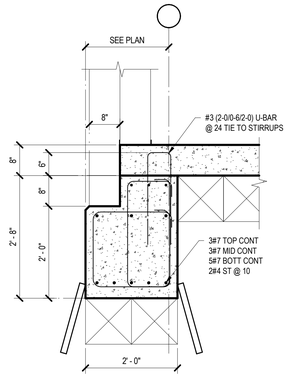

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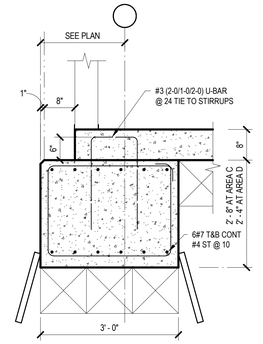

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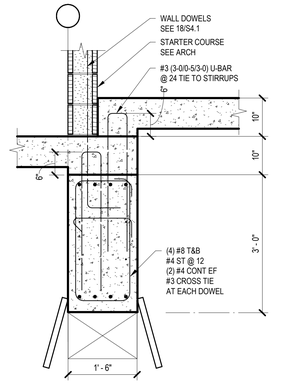

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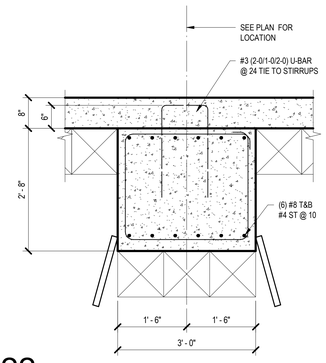

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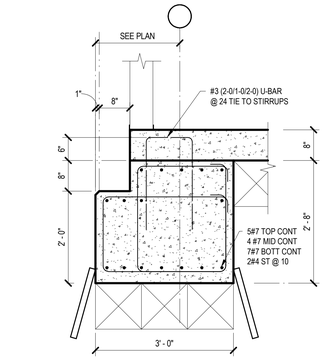

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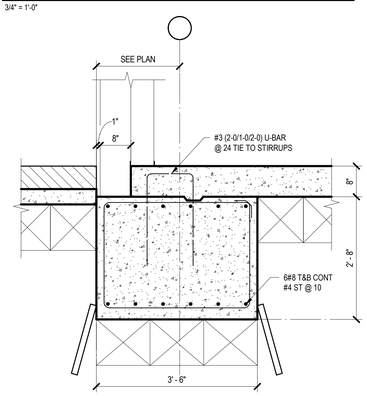

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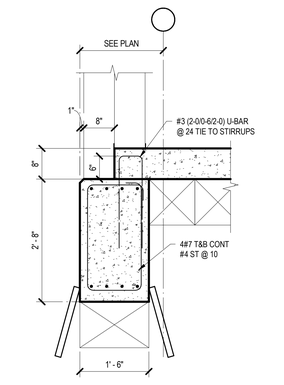

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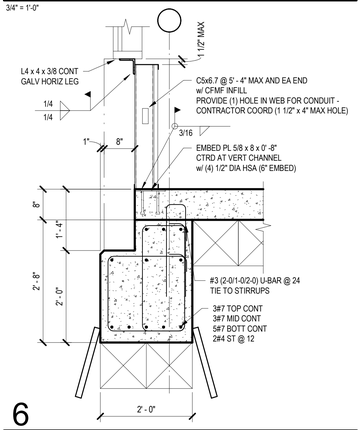

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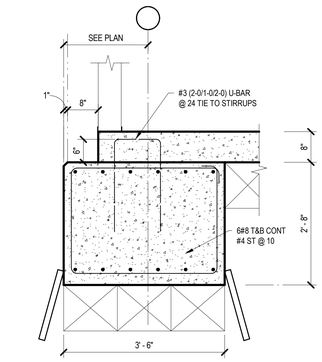

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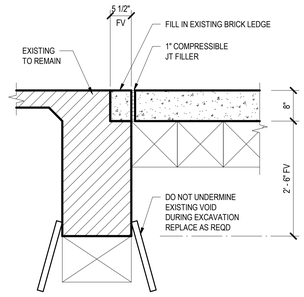

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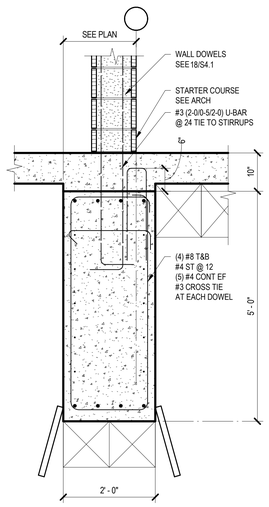

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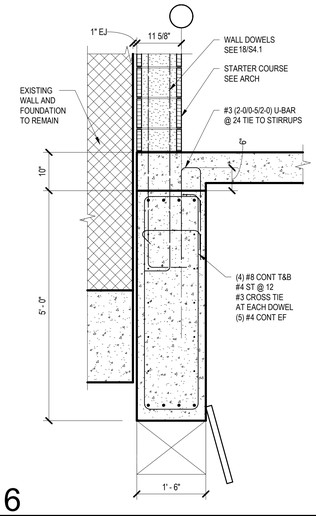

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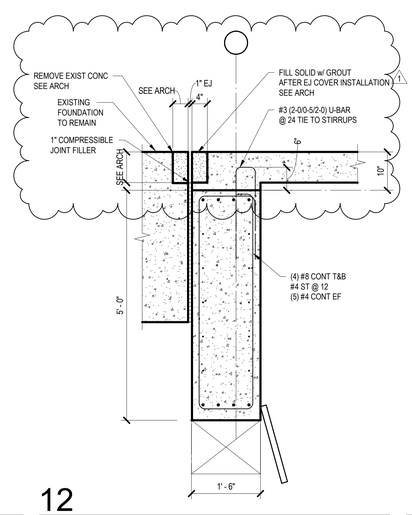

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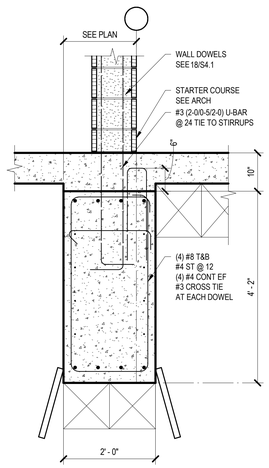

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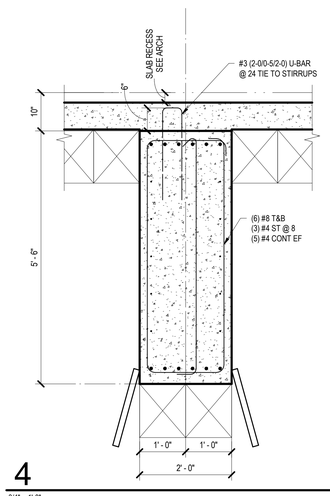

4
3/4" = 1'-0"

Revision / 1 — Addendum 1 — Date 05/07/25

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

CHERYL R. STEWART — 117498 — LICENSED PROFESSIONAL ENGINEER — 5-7-2025

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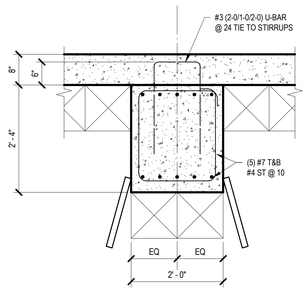
CONCRETE DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. S3.4
Drawn By: LAFP	ISSUE FOR BID
Date: 04/22/2025	

L.A. FUESS PARTNERS, INC.
Structural Engineers
3333 Lee Parkway, Suite 300 • Dallas, TX 75219
LAFP PROJ. NO. 24081 FIRM REG. NO. F-537



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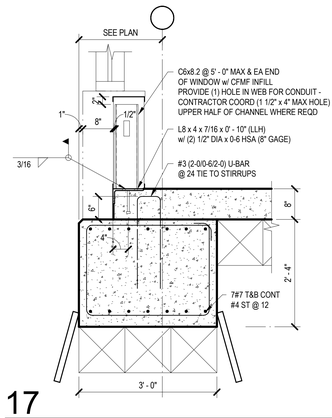

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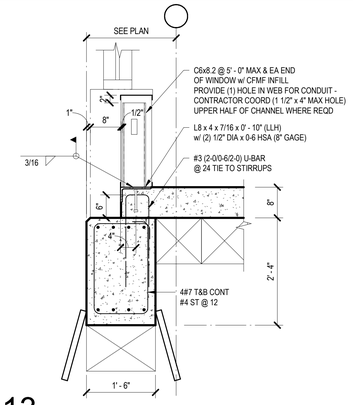

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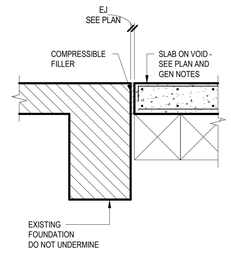

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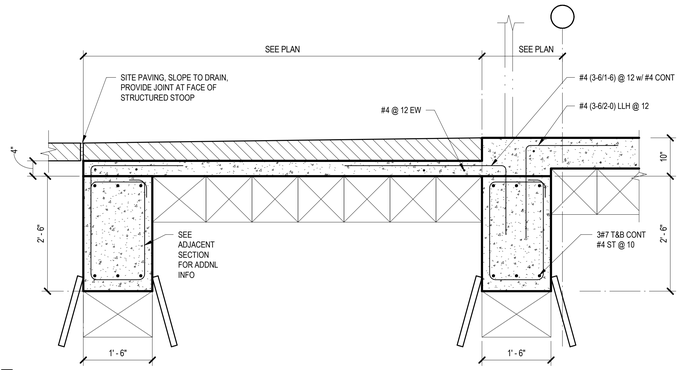

5
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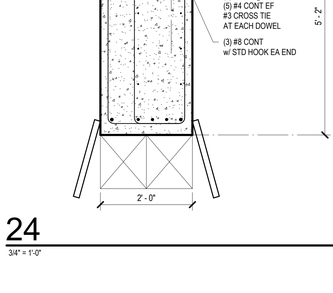

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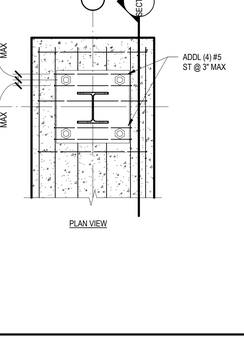

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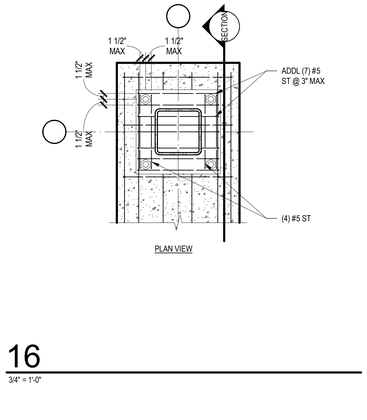

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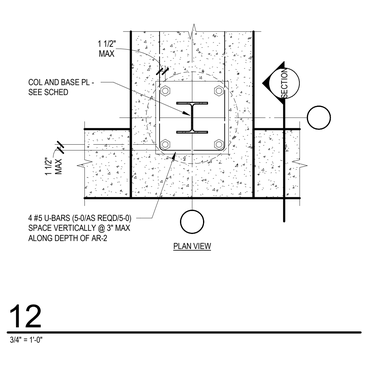

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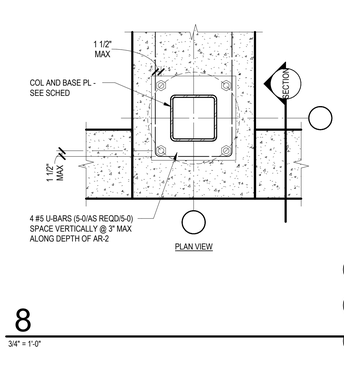

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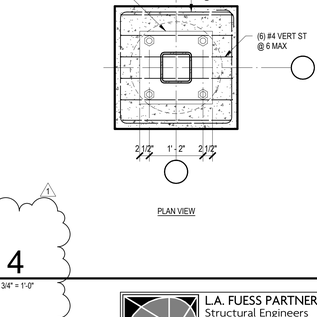
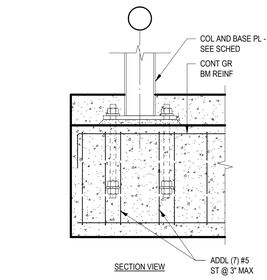


2
3/4" = 1'-0"

24
3/4" = 1'-0"



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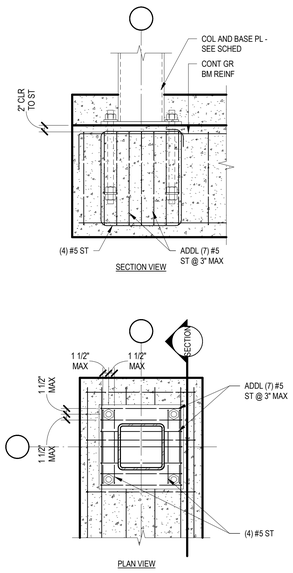

16
3/4" = 1'-0"

12
3/4" = 1'-0"

8
3/4" = 1'-0"

4
3/4" = 1'-0"

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Structural Engineers
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LAFP PROJ. NO. 24081 FIRM REG. NO. F-537

Revision / 1 — Addendum 1
Date 05/07/25

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

CHERYL R. STEWART
117498
LICENSED
PROFESSIONAL ENGINEER
5-7-2025

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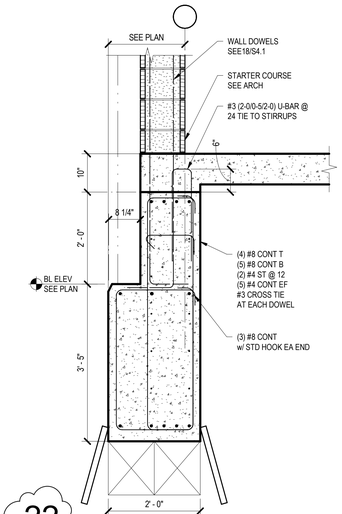
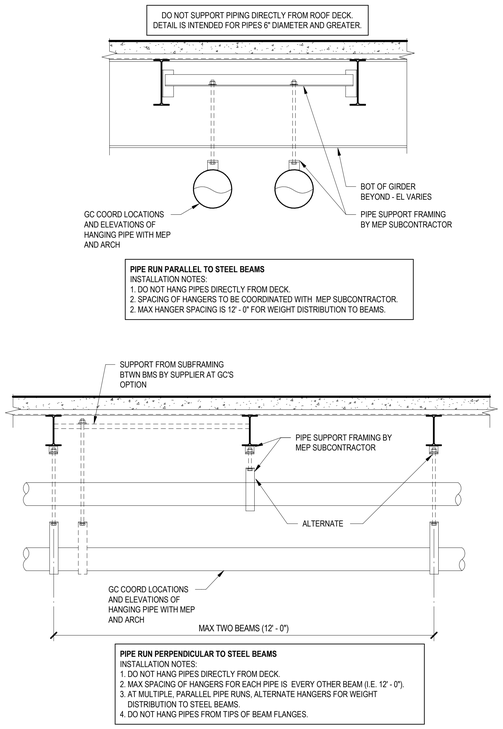
CONCRETE DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S3.5
Drawn By: LAFP	
Date: 04/22/2025	



22
3/4" = 1'-0"



SUSPENDED WATER PIPE SUPPORT

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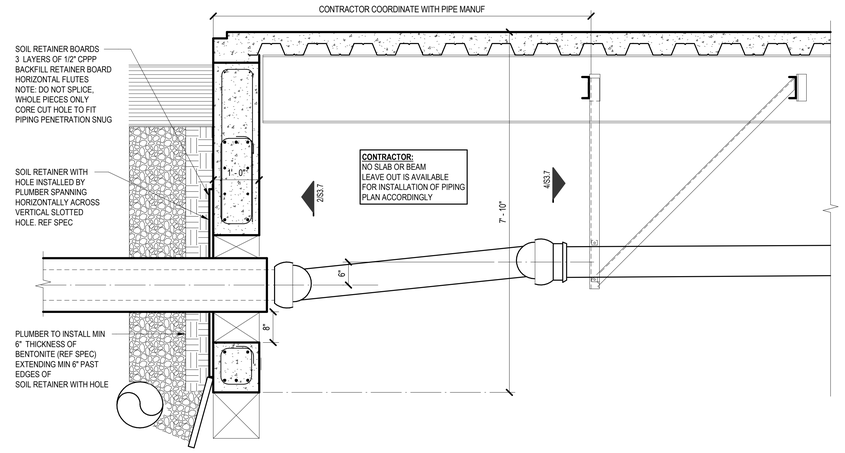

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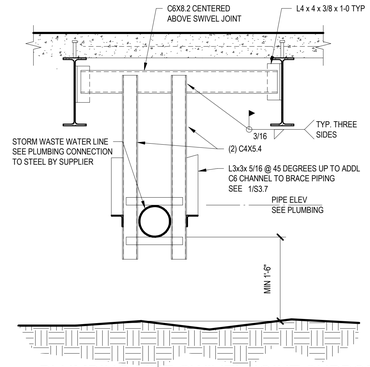

PLUMBING LINE SWIVEL JOINT SUPPORT

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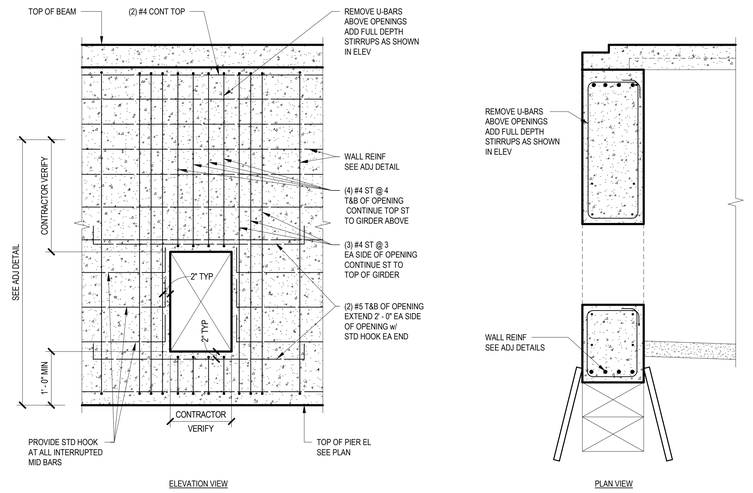

LARGE OPENING IN GRADE BEAM

2
3/4" = 1'-0"

Revision /	Date	
1	05/07/25	Addendum 1

Project:
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Job No. 01954-08-01	Sheet No. ISSUE FOR BID S3.7
Drawn By: LAFP	
Date: 04/22/2025	

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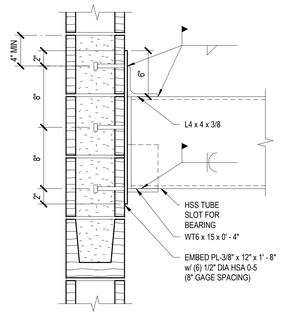
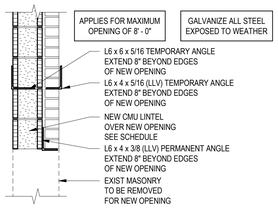


21 TUBE BEARING SEAT TYPICAL DETAIL

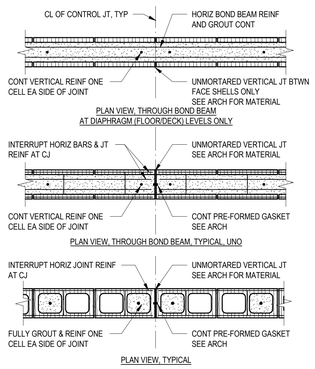
NO SCALE 420 EMBED

SEQUENCE OF CONSTRUCTION:
1. CONTRACTOR CONFIRM EXIST VENEER IS RUNNING BOND AND STRUCTURE IS 8" CMU. NOTIFY ARCHITECT IF OTHERWISE.
2. SAWCUT MASONRY WALL FOR ANGLES TO BE INSTALLED.
3. INSERT STEEL ANGLES & REMOVE MASONRY AS REQD.
4. FULLY GROUT & REINF FIRST CMU CELL AT JAMB WITH #5 VERT UP TO BOTTOM OF OPENING.
5. INSTALL PERMANENT L6 x 4 LINTEL & CMU LINTEL AS REQD.
6. INSTALL MASONRY VENEER ABOVE PERMANENT LINTEL.
7. REMOVE TEMPORARY L6 x 4 ANGLE, REGROUT & TOOL JOINT.



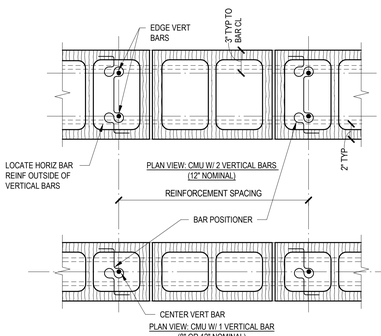
17 NEW OPENING AT EXIST MASONRY WALL TYPICAL DETAIL

NO SCALE 406



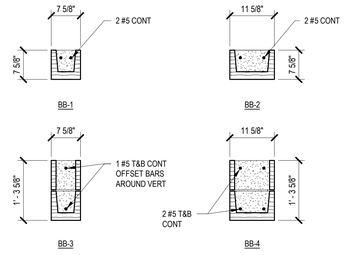
13 CMU CONTROL JOINT TYPICAL DETAIL

NO SCALE TD04140



9 CMU REINFORCEMENT LAYOUT, BOND BEAMS, AND SPLICE SCHED TYPICAL DETAIL

NO SCALE TD04100

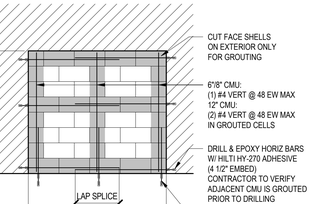


CMU REINFORCEMENT LAYOUT NOTES:
1. LOCATE VERTICAL OR HORIZONTAL REINFORCEMENT AT LEAST 1/2" FROM CMU SHELL TO ALLOW FOR PROPER GROUT CONSOLIDATION.
2. THESE BOND BEAMS APPLY TO WALLS WHERE BOND BEAMS ARE NOT SPECIFICALLY REFERENCED. SEE SECTIONS & DETAILS FOR SPECIAL BOND BEAMS THAT ARE REQUIRED.
3. CONSTRUCT BOTTOM COURSE OF A BOND BEAM USING SOLID BOTTOM, TROUGH-STYLE MASONRY UNITS, UNO.

MASONRY SPLICE SCHEDULE - GRADE 60

f'm	1900 - 2000 PSI		2500 PSI	
BAR SIZE	HORIZ OR EDGE VERT	CENTER VERT	HORIZ OR EDGE VERT	CENTER VERT
#4	24"	16"	20"	14"
#5	36"	24"	32"	22"
#6	54"	45"	54"	40"
#7	64"	62"	64"	54"
#8	72"	72"	72"	72"

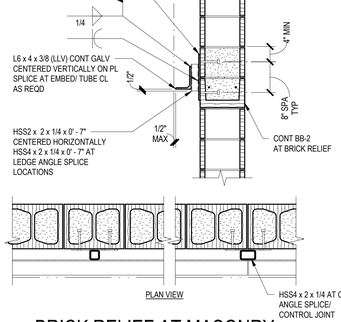
NOTES:
1. SPLICE LENGTHS GIVEN ARE FOR GRADE 60 REINFORCING BARS.
2. USE LONGEST LENGTH WHEN SPLICING BARS OF DIFFERENT SIZE.
3. INCREASE SPLICE LENGTH OF EPOXY-COATED REINFORCEMENT 50% OVER TABLE VALUES, UP TO A MAXIMUM OF 96 BAR DIAMETERS
4. SEE PLAN VIEWS FOR DEFINITION OF HORIZ, EDGE VERT, AND CENTER VERT BARS.



BASIS OF DESIGN FOR SHOWN CONNECTION IS McKEON SAFESPACE 500 OVERHEAD DOOR. ANY ACCEPTABLE PRODUCT MUST BE INSTALLED TO MEET REQUIREMENTS OF ICC-500 (2014) FOR DEBRIS IMPACT AND PRESSURE.

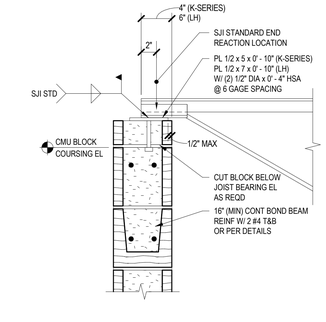
22 OVERHEAD DOOR JAMB TYPICAL DETAIL

NO SCALE 426 OVHD JAMB



18 CMU DOWELS TYPICAL DETAIL

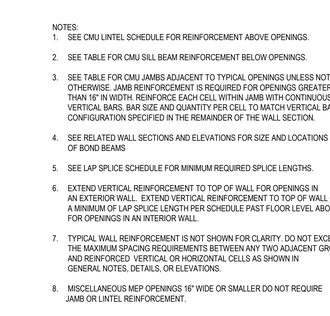
NO SCALE 411



BEAM SIZE	MIN DEPTH
W8, W10	5"
W12, W14, W16	8"
W18, W21, W24	12"

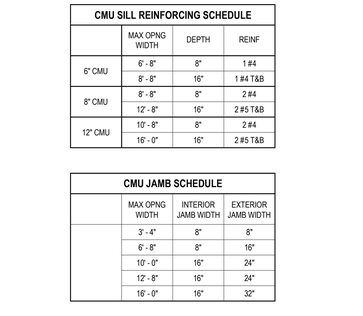
14 BEAM TO CMU CONNECTION TYPICAL DETAIL

NO SCALE TD04151



10 MASONRY CONSTRUCTION JOINT TYPICAL DETAIL

NO SCALE TD04130



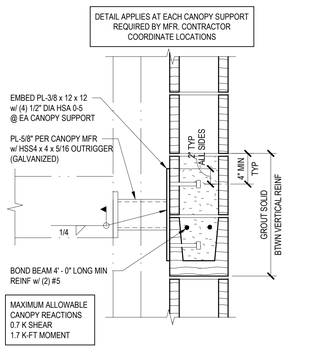
CMU LINTEL SCHEDULE

WALL THICKNESS	MAX OPENING	LINTEL DEPTH	TOP REINF	BOT REINF
6"	3' - 4"	8"	NONE	1 #4
	6' - 8"	16"	1 #4	1 #4
	8' - 8"	24"	1 #4	1 #4
8"	3' - 4"	8"	NONE	2 #4
	6' - 8"	16"	2 #4	2 #4
	10' - 0"	24"	2 #4	2 #4
	12' - 8"	32"	2 #5	2 #5
12"	3' - 4"	8"	NONE	2 #4
	6' - 8"	16"	2 #4	2 #4
	10' - 0"	24"	2 #4	2 #4
	12' - 8"	24"	2 #5	2 #5
	16' - 0"	32"	2 #6	2 #6

NOTES:
1. CONSTRUCT BOTTOM COURSE OF LINTEL OF SOLID-BOTTOM, TROUGH-STYLE BLOCKS, UNLESS OTHERWISE NOTED.
2. A CMU LINTEL IS REQUIRED FOR OPENINGS GREATER THAN 16" IN WIDTH.
3. LINTEL REINFORCEMENT MAY NOT BE LAP SPLICED ABOVE AN OPENING.
4. CONTROL JOINTS ARE NOT PERMITTED TO OCCUR WITHIN LINTEL REINFORCEMENT.
5. SEE CMU WALL REINFORCING DETAIL FOR VERTICAL REINFORCEMENT PLACEMENT AT JAMBS.
6. EXTEND HORIZONTAL REINFORCEMENT 24" OR 40 BAR DIAMETERS BEYOND THE FACE OF OPENING. PROVIDE STD HOOK AT END OF BAR IF HORIZONTAL EXTENSION IS NOT POSSIBLE.
7. PRECAST CONCRETE LINTELS OF THE SAME SIZE AND LENGTH MAY BE SUBSTITUTED EXCEPT PROVIDE TOP BARS EQUAL TO SCHEDULED BOTTOM BARS.

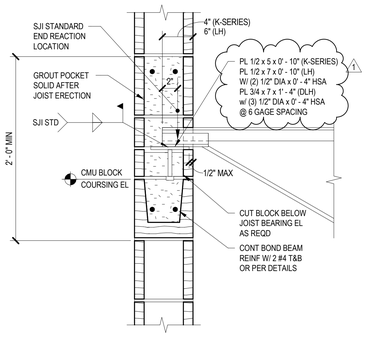
6 CMU LINTELS TYPICAL DETAIL

NO SCALE TD04101



23 CMU CANOPY SUPPORT TYPICAL DETAIL

NO SCALE 427

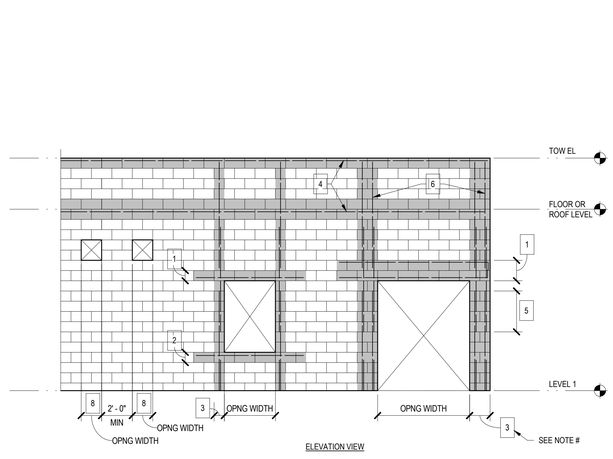


19 NON-LOAD BEARING CMU DOWELS TYPICAL DETAIL

NO SCALE 411 NON-LOAD BRG

15 SINGLE JOIST TO CMU CONNECTION TYPICAL DETAIL

NO SCALE TD04160



ELEVATION VIEW

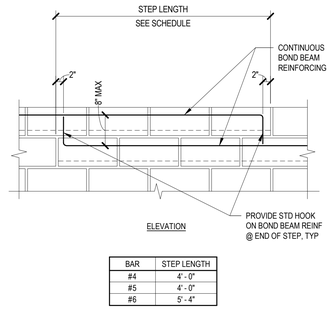
NOTES:
1. SEE CMU LINTEL SCHEDULE FOR REINFORCEMENT ABOVE OPENINGS.
2. SEE TABLE FOR CMU SILL BEAM REINFORCEMENT BELOW OPENINGS.
3. SEE TABLE FOR CMU JAMBS ADJACENT TO TYPICAL OPENINGS UNLESS NOTED OTHERWISE. JAMB REINFORCEMENT IS REQUIRED FOR OPENINGS GREATER THAN 16" IN WIDTH. REINFORCE EACH CELL WITHIN JAMB WITH CONTINUOUS VERTICAL BARS. BAR SIZE AND QUANTITY PER CELL TO MATCH VERTICAL BAR CONFIGURATION SPECIFIED IN THE REMAINDER OF THE WALL SECTION.
4. SEE RELATED WALL SECTIONS AND ELEVATIONS FOR SIZE AND LOCATIONS OF BOND BEAMS
5. SEE LAP SPLICE SCHEDULE FOR MINIMUM REQUIRED SPLICE LENGTHS.
6. EXTEND VERTICAL REINFORCEMENT TO TOP OF WALL FOR OPENINGS IN AN EXTERIOR WALL. EXTEND VERTICAL REINFORCEMENT TO TOP OF WALL OR A MINIMUM OF LAP SPLICE LENGTH PER SCHEDULE PAST FLOOR LEVEL ABOVE FOR OPENINGS IN AN INTERIOR WALL.
7. TYPICAL WALL REINFORCEMENT IS NOT SHOWN FOR CLARITY. DO NOT EXCEED THE MAXIMUM SPACING REQUIREMENTS BETWEEN ANY TWO ADJACENT GROUTED AND REINFORCED VERTICAL OR HORIZONTAL CELLS AS SHOWN IN GENERAL NOTES, DETAILS, OR ELEVATIONS.
8. MISCELLANEOUS MEP OPENINGS 16" WIDE OR SMALLER DO NOT REQUIRE JAMB OR LINTEL REINFORCEMENT.

CMU SILL REINFORCING SCHEDULE

	MAX OPNG WIDTH	DEPTH	REINF
6" CMU	6' -8"	8"	1 #4
	8' -8"	16"	1 #4 T&B
8" CMU	8' -8"	8"	2 #4
	12' - 8"	16"	2 #5 T&B
12" CMU	10' - 8"	8"	2 #4
	16' - 0"	16"	2 #5 T&B

CMU JAMB SCHEDULE

	MAX OPNG WIDTH	INTERIOR JAMB WIDTH	EXTERIOR JAMB WIDTH
	3' -4"	8"	8"
	6' -8"	8"	16"
	10' - 0"	16"	24"
	12' - 8"	16"	24"
	16' - 0"	16"	32"



BAR	STEP LENGTH
#4	4' - 0"
#5	4' - 0"
#6	5' - 4"

3 STEP IN BOND BEAM TYPICAL DETAIL

NO SCALE TD04103

CMU OPENING INFILL

24 TYPICAL DETAIL

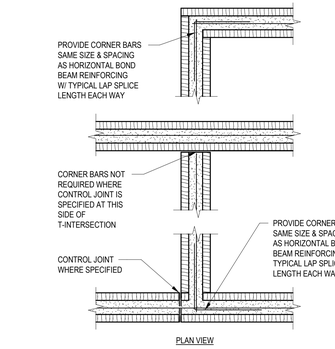
NO SCALE

20 BRICK RELIEF AT MASONRY TYPICAL DETAIL (4" NOMINAL BRICK)

NO SCALE

16 SINGLE JOIST TO CMU CONNECTION TYPICAL DETAIL

NO SCALE TD04160A



12 CMU WALL OPENING REINFORCING TYPICAL DETAIL

NO SCALE TD04120

4 BOND BEAM INTERSECTIONS TYPICAL DETAIL

NO SCALE TD04104

Date 05/07/25
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TYPICAL MASONRY DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. S4.1
Drawn By: LAFP	ISSUE FOR BID
Date: 04/22/2025	

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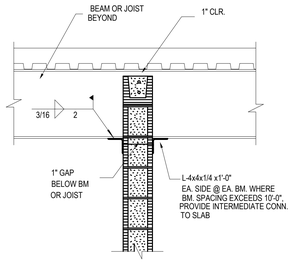
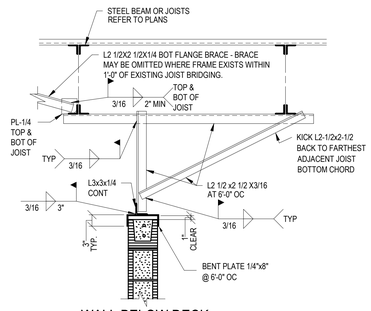
MASONRY WALL BRACING NOTES:

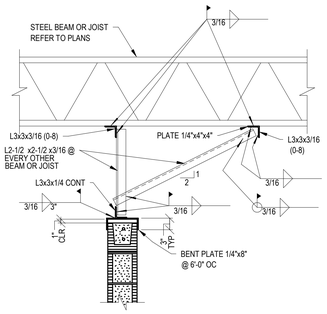
1. CLEAR GAPS INDICATED BETWEEN TOP OF WALLS AND STRUCTURE ARE TO BE FILLED (COMPRESSIBLE FILLER, ETC.) AS INDICATED ON ARCHITECTURAL DRAWINGS.
2. WHERE WALLS ARE ORIENTED SKEWED TO FRAMING, USE PARALLEL AND PERPENDICULAR DETAILS IN COMBINATION TO PROVIDE BRACING CONNECTIONS AT MAXIMUM SPACING INDICATED.
3. AT FREE ENDS OF WALLS, LOCATE FIRST BRACE WITHIN 5 FT OF END OF WALL.
4. CORNERS AND TEES WITH MINIMUM 5 FT LONG INTERSECTING WALLS CAN BE CONSIDERED BRACED POINTS. HORIZONTAL RUNS OF WALL 10 FT LONG AND LESS WITH INTERSECTIONS ON BOTH ENDS DO NOT REQUIRE BRACING.
5. REFER TO SHEET ARCHITECTURAL DRAWINGS FOR LAYOUT AND DEFINITION OF WALLS THAT EXTEND TO DECK.



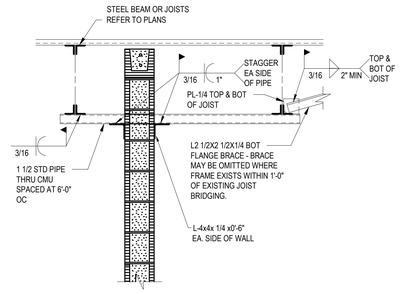
WALL EXTENDING TO DECK PERPENDICULAR TO BEAMS OR JOISTS



WALL BELOW DECK PARALLEL TO BEAMS OR JOISTS

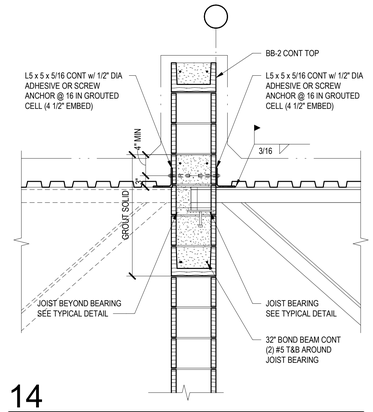


WALL BELOW DECK PERPENDICULAR TO BEAMS OR JOISTS

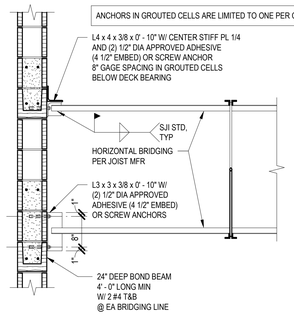


WALL EXTENDING TO DECK PARALLEL TO BEAMS OR JOISTS

17 CMU WALL BRACING
3/4" = 1'-0"

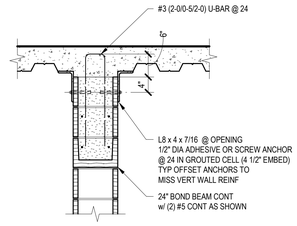


14
3/4" = 1'-0"

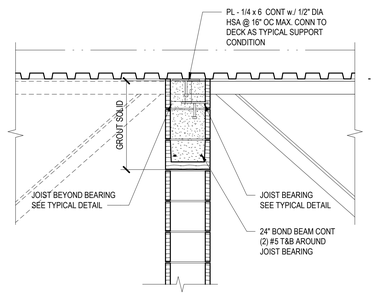


10 JOIST BRIDGING TO CMU (LH/DLH) TYPICAL DETAIL
NO SCALE TD04161A

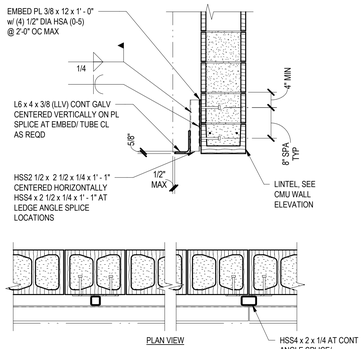
NOT USED



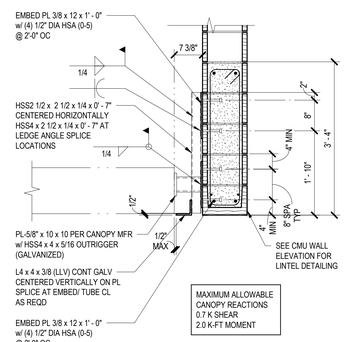
2
3/4" = 1'-0"

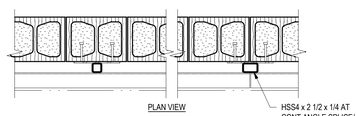


15
3/4" = 1'-0"

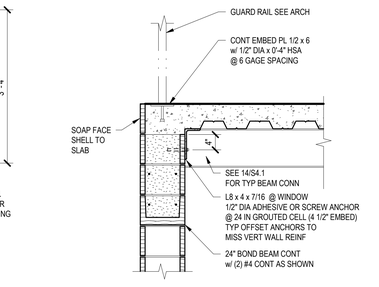


11 BRICK RELIEF AT MASONRY TYPICAL DETAIL (4" NOMINAL BRICK)
NO SCALE

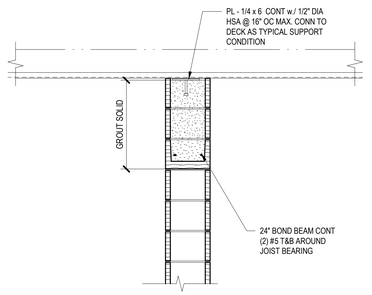




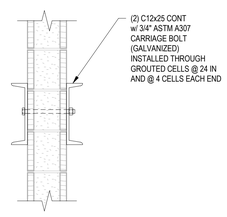
8
BOTTOM OF CANOPY ALIGNED WITH BRICK BRG ELEVATION
3/4" = 1'-0"



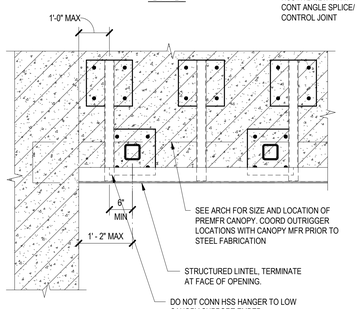
3
3/4" = 1'-0"

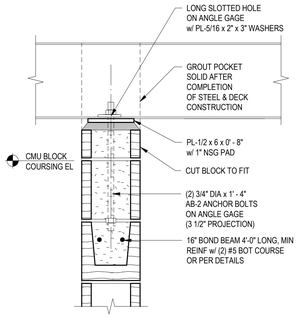


16
3/4" = 1'-0"



12
1" = 1'-0"





4 CONTINUOUS BEAM BEARING TYPICAL DETAIL
NO SCALE 421 ABOLT

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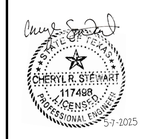
LAFP PROJ. NO. 24081 FIRM REG. NO. F-537

Revision / 1: Addendum 1
Date: 05/07/25

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX



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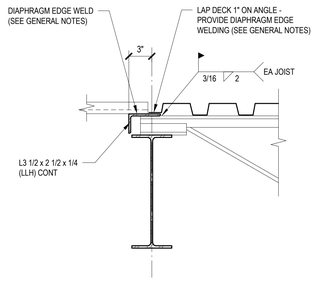
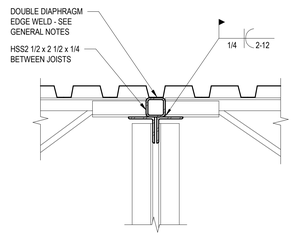
MASONRY DETAILS

PACKAGE	VOLUME
Job No. 018164-08-01	Sheet No. S4.2
Drawn By: LAFP	ISSUE FOR BID
Date: 04/22/2025	



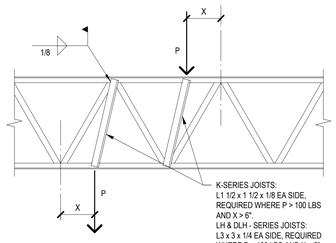
21 ROOF DECK SUPPORT AT CHANGE IN DECK DIRECTION TYPICAL DETAIL

NO SCALE TD05625



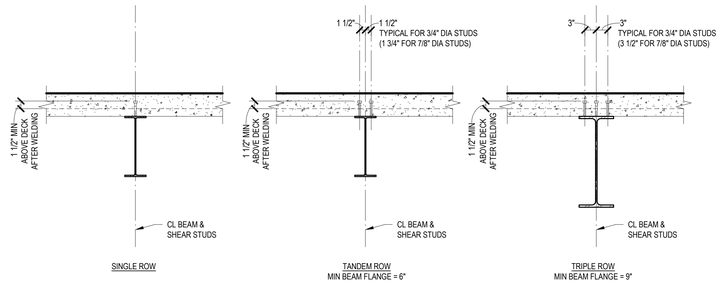
17 DIAPHRAGM TRANSFER TYPICAL DETAIL

NO SCALE TD05510



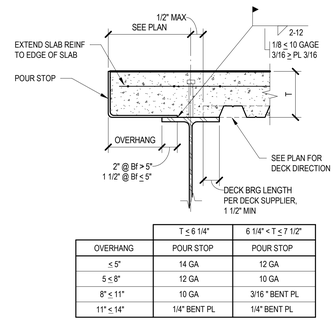
13 CONCENTRATED LOAD ON STL JST TYPICAL DETAIL

NO SCALE TD05489



9 STUD WELDING DETAILS TYPICAL DETAIL

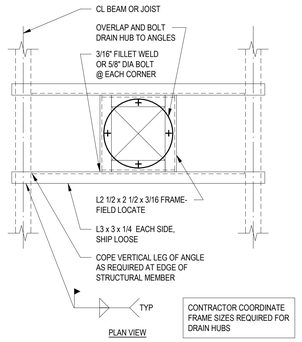
NO SCALE TD05355



	T ≤ 6 1/4"	6 1/4" < T ≤ 7 1/2"
OVERHANG	POUR STOP	POUR STOP
≤ 5"	14 GA	12 GA
5 < 8"	12 GA	10 GA
8" ≤ 11"	10 GA	3/16 " BENT PL
11" ≤ 14"	1/4" BENT PL	1/4" BENT PL

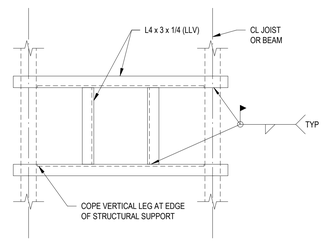
1 SLAB EDGE - COMPOSITE DECK TYPICAL DETAIL

NO SCALE TD05323X



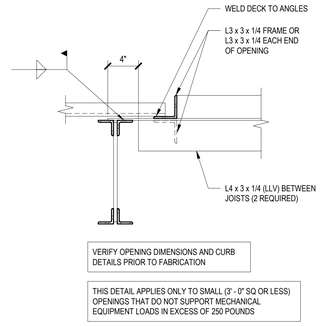
22 ROOF DRAIN SUPPORT FRAME TYPICAL DETAIL

NO SCALE TD05616



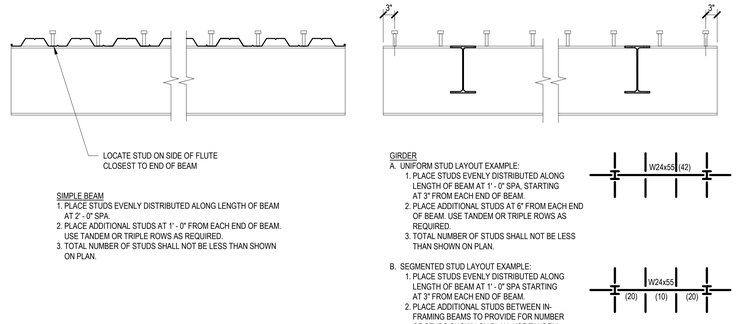
18 ROOF HATCH FRAME TYPICAL DETAIL

NO SCALE TD05615



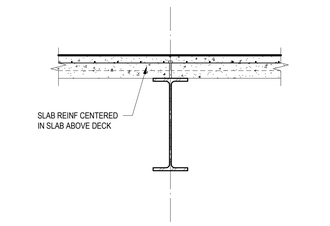
14 MISCELLANEOUS ROOF OPENING TYPICAL DETAIL

NO SCALE TD05495



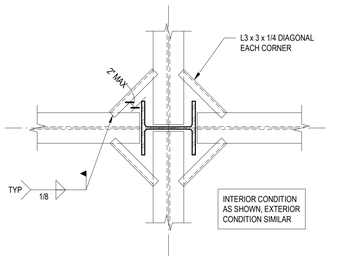
10 STUD LAYOUT - COMPOSITE BEAMS TYPICAL DETAIL

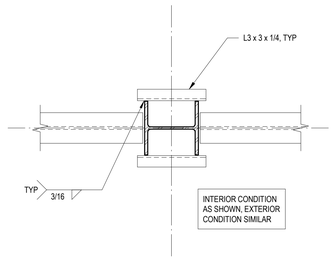
NO SCALE TD05356

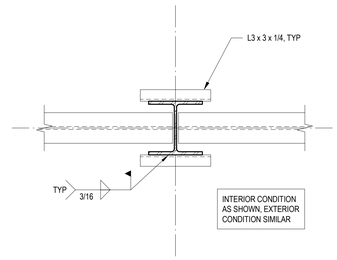


2 COMPOSITE BEAM TYPICAL DETAIL

NO SCALE TD05350

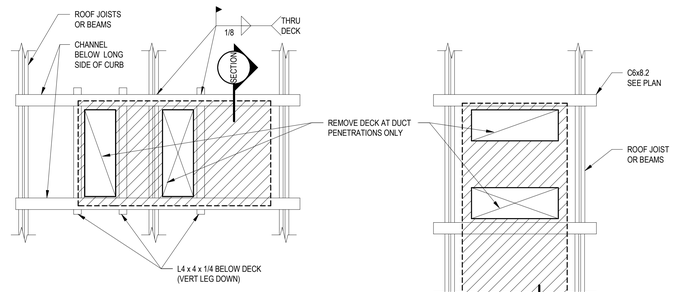


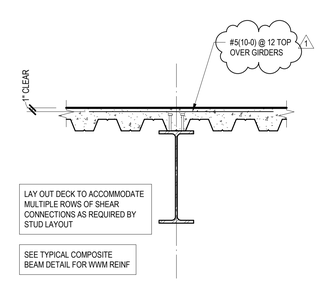




23 FLOOR DECK SUPPORT TYPICAL DETAIL

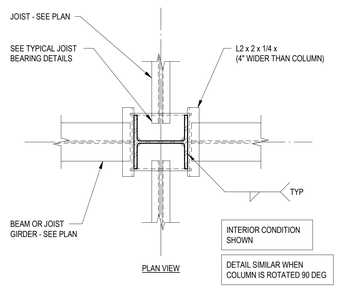
NO SCALE TD05330





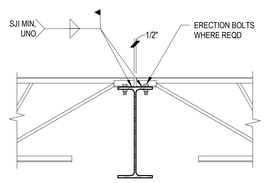
3 COMPOSITE GIRDER TYPICAL DETAIL

NO SCALE TD05351



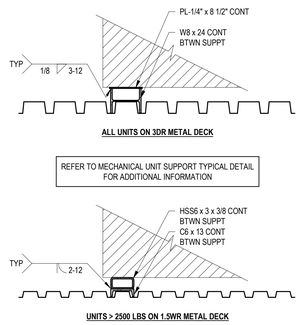
24 FLOOR DECK SUPPORT TYPICAL DETAIL

NO SCALE TD05331



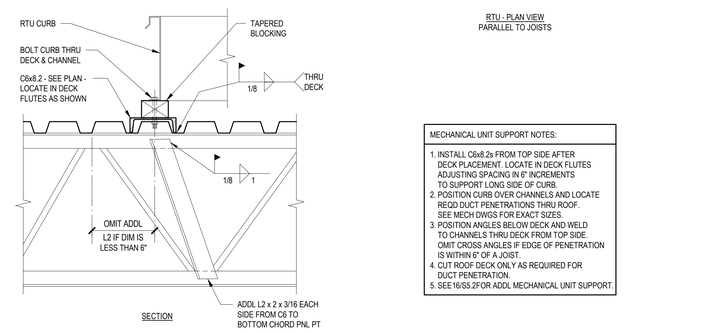
20 JOIST CONNECTION TYPICAL DETAIL

NO SCALE TD05420



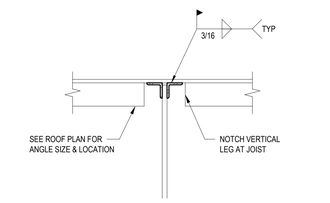
16 ADDL MECHANICAL UNIT SUPPORTS TYPICAL DETAIL

NO SCALE 543



12 MECHANICAL UNIT SUPPORT TYPICAL DETAIL

NO SCALE TD05700



4 DIAPHRAGM BRACING ANGLE TYPICAL DETAIL

NO SCALE TD05501

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Revision / 1 — Date 05/07/25 — Addendum 1

Project:
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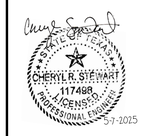

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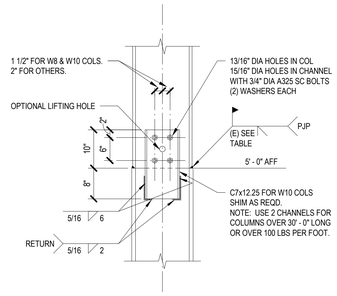
TYPICAL STEEL DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S5.2
Drawn By: LAFP	
Date: 04/22/2025	

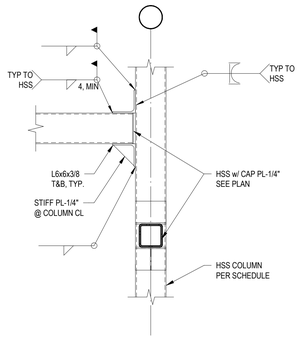
WELD SIZE TABLE	
FLANGE T (SMALLEST)	EFFECTIVE THROAT (E)
1/2" < T ≤ 3/4"	1/4"
3/4" < T ≤ 1 1/2"	5/16"
1 1/2" < T ≤ 2 1/4"	3/8"
2 1/4" < T ≤ 6"	1/2"
NOTE: PROVIDE A LAND WIDTH OF 1/4" OR GREATER FOR PJP FLANGE WELDS	

NOTES:
1. THIS TYPICAL DETAIL APPLIES TO COLUMNS WITH DEPTHS THAT ARE NOMINALLY THE SAME.
2. THIS TYPICAL DETAIL APPLIES TO GRAVITY COLUMNS ONLY. SEE OTHER TYPICAL DETAILS FOR SPLICES IN MOMENT FRAMES AND BRACED FRAMES.
3. THE SPLICING OPTION DEPICTED IN THIS TYPICAL DETAIL IS PRE-APPROVED FOR USE ON THIS PROJECT. THE FABRICATOR MAY SUBMIT ALTERNATE MEANS OF SPLICING FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.



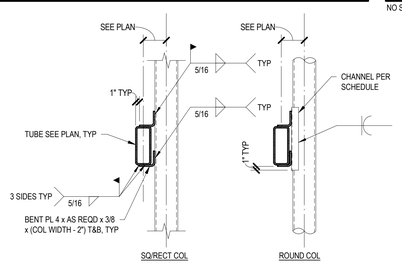
13 WF COLUMN SPLICES (GRAVITY COLUMNS) TYPICAL DETAIL

NO SCALE TD05140



1 HSS CONNECTION TO COLUMN TYPICAL DETAIL

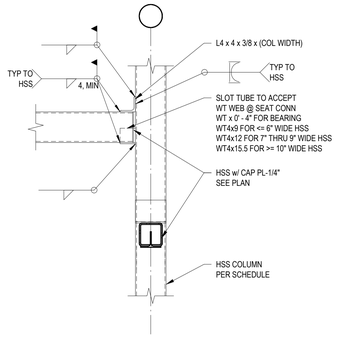
NO SCALE 514



SCHEDULE	
HSS ROUND	CHANNEL
HSS6	MC6x12
HSS6.625	MC6x12
HSS8	MC8x18.7
HSS8.625	MC8x18.7
HSS10.75	MC10x22

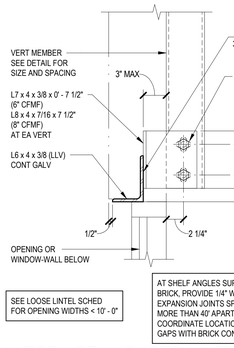
6 HSS CONNECTION TO COLUMN TYPICAL DETAIL

NO SCALE



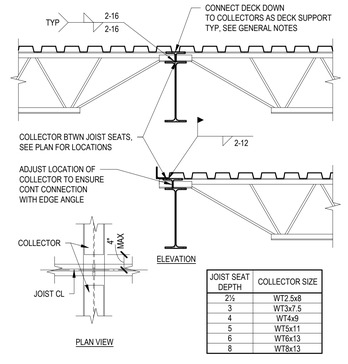
2 HSS CONNECTION TO COLUMN TYPICAL DETAIL

NO SCALE 514 WT



7 BOLTED STRUCTURED LINTEL TYPICAL DETAIL

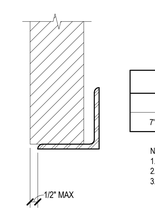
NO SCALE Drafting 13



JOIST SEAT DEPTH	COLLECTOR SIZE
2½	WT2.5x6
3	WT3x7.5
4	WT4x9
5	WT5x11
6	WT6x13
8	WT8x13

3 SHEAR COLLECTOR TYPICAL DETAIL

NO SCALE 534

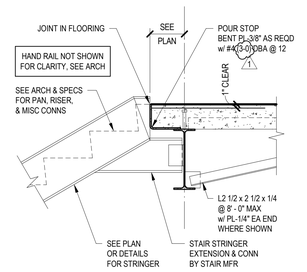


SPAN	ANGLE SIZE
< 7' - 6"	L4 x 4 x 5/16
7' - 6" TO 10' - 0"	L6 x 4 x 3/8 (LLV)

NOTES:
1. SPAN = CLEAR OPENING WIDTH
2. BEAR 8" EA END
3. SEE SPECS FOR COATING REQUIREMENT

8 LOOSE LINTEL SCHEDULE TYPICAL DETAIL

NO SCALE TD05190



4 STAIR STRINGER CONN TYPICAL DETAIL

NO SCALE 536

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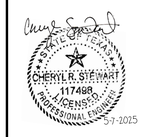
Revision /	Date	
1	05/07/25	Addendum 1

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX



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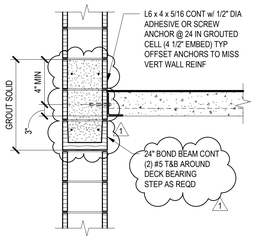
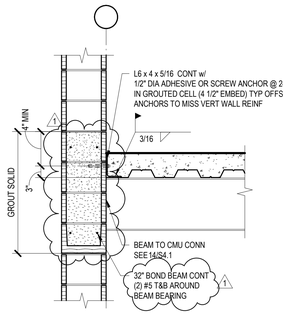
TYPICAL STEEL DETAILS

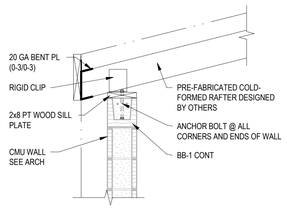
PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S5.3
Drawn By: LAFP	
Date: 04/22/2025	



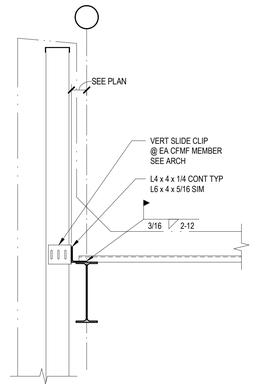
21
3/4" = 1'-0"



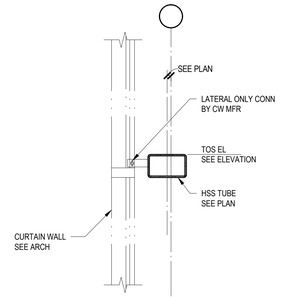
17
3/4" = 1'-0"



13
3/4" = 1'-0"



9
3/4" = 1'-0"



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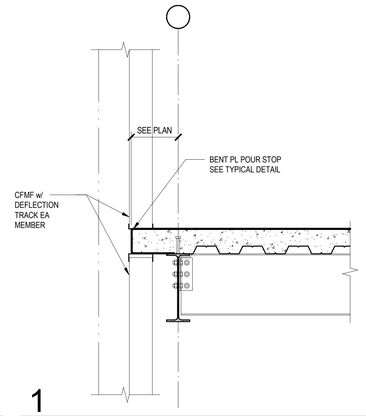

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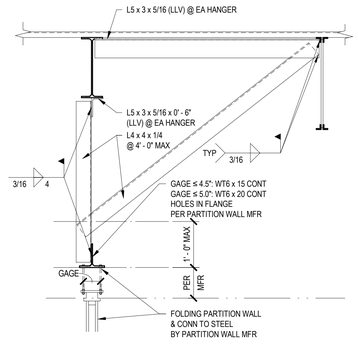
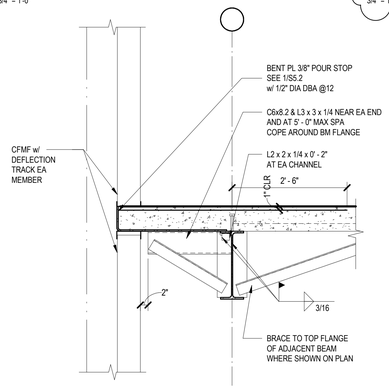


PARTITION WALL HANGER

22 TYPICAL DETAIL
NO SCALE 558



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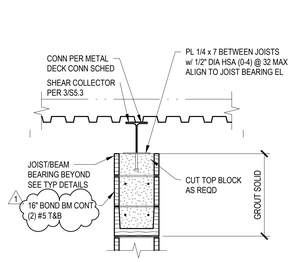

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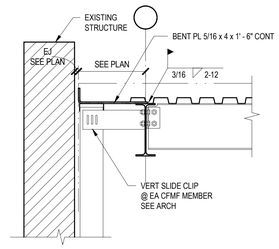

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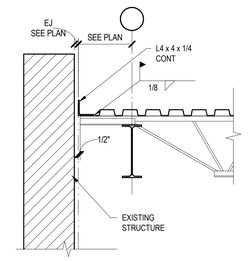

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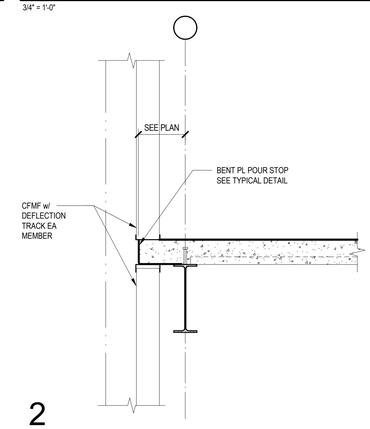

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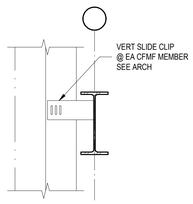

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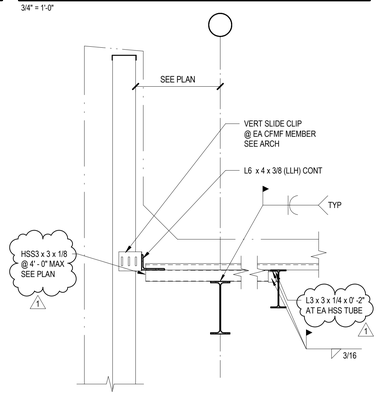

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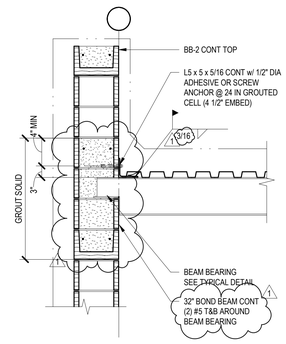

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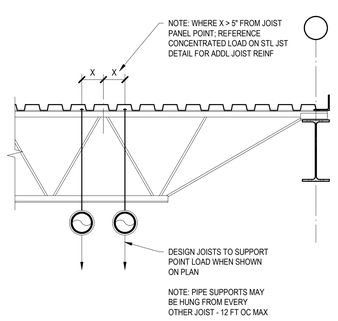
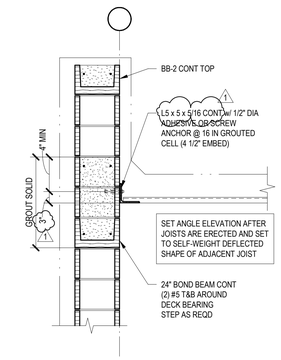


JOIST LOADING AT PIPING
11 TYPICAL DETAIL
NO SCALE S5.4/11



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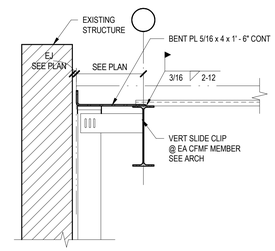

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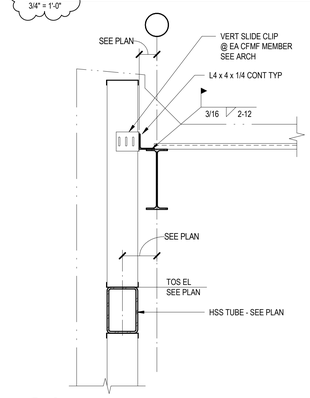

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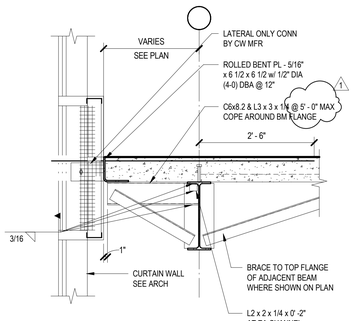

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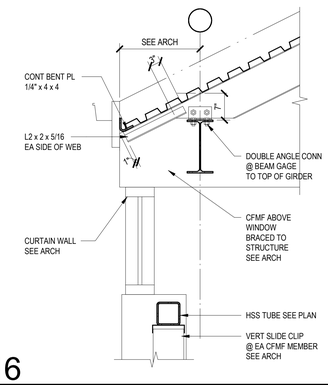

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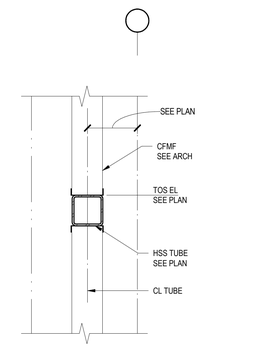

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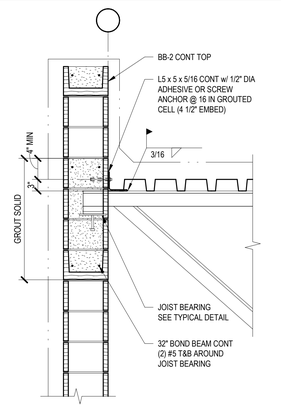

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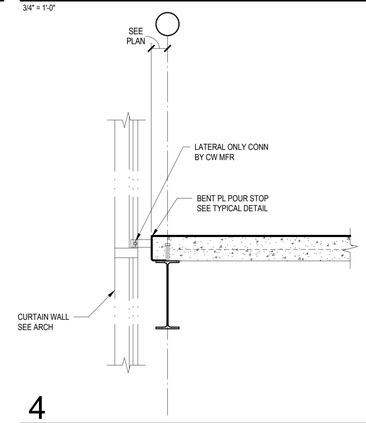


4
3/4" = 1'-0"

Revision /	Date	
1	05/07/25	Addendum 1

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

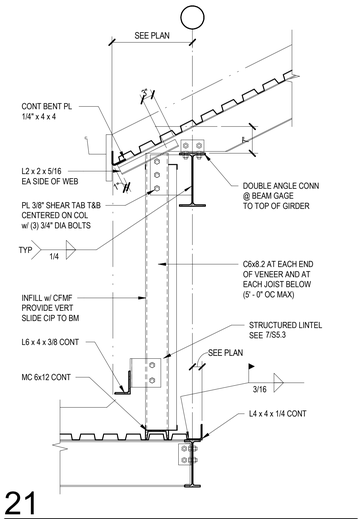
STATE OF TEXAS
CHERYL R. STEWART
117498
LICENSED
PROFESSIONAL ENGINEER
5-7-2025

Huckabee
www.huckabee-inc.com
800.687.1229

STEEL DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S5.4
Drawn By: LAFP	
Date: 04/22/2025	

L.A. FUESS PARTNERS, INC.
Structural Engineers
3333 Lee Parkway, Suite 300 • Dallas, TX 75219
LAFP PROJ. NO. 24081 FIRM REG. NO. F-537



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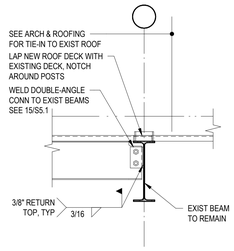

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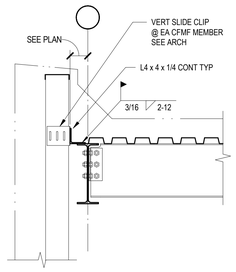

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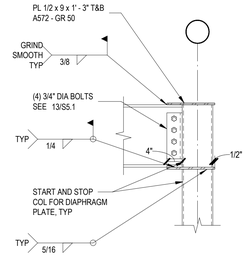

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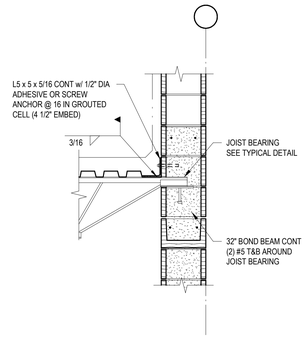

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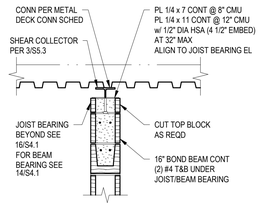

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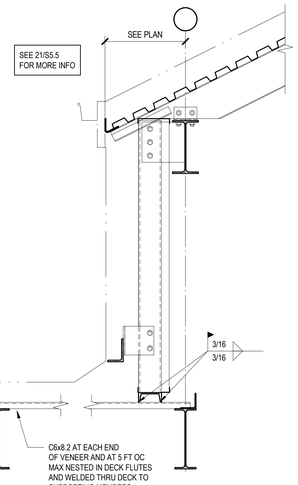

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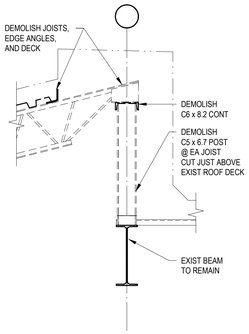

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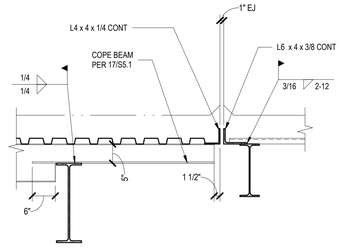

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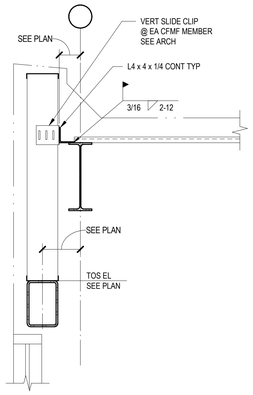

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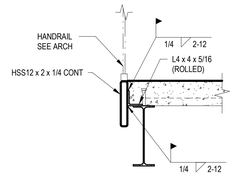

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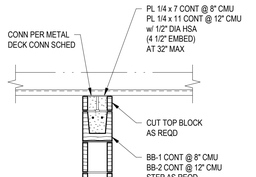

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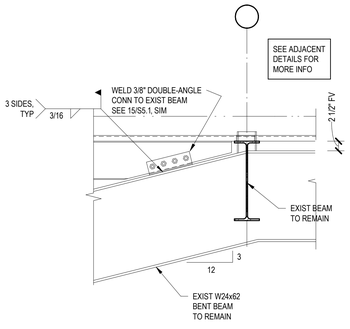

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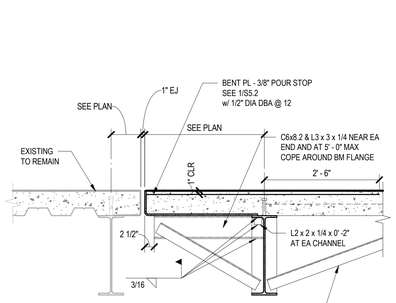

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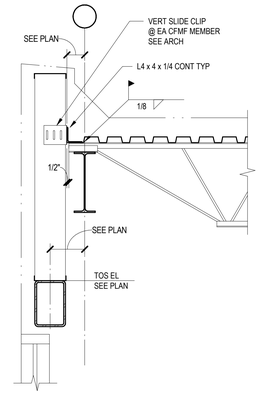

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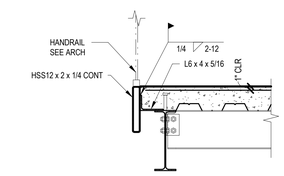

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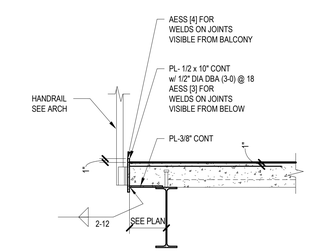

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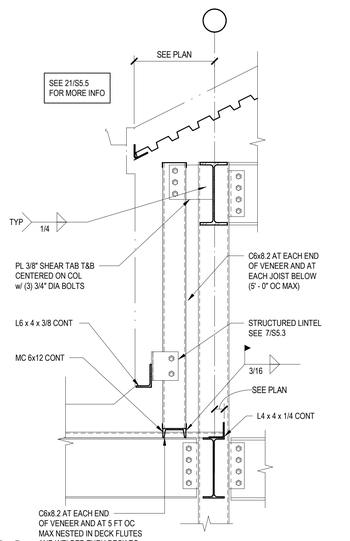

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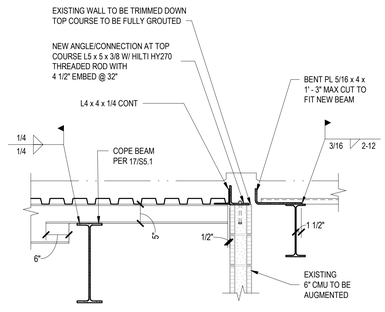

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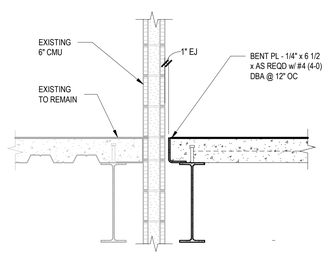

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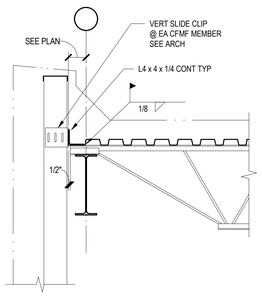

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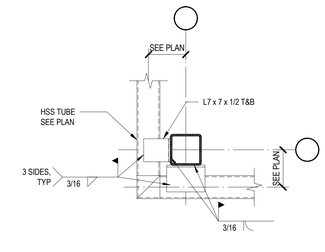

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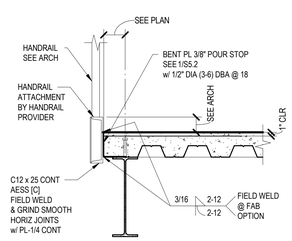

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3/4" = 1'-0"
/1

Revision / Date
1 05/07/25 Addendum 1

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

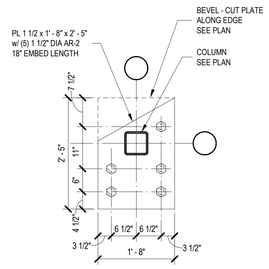
CHERYL R. STEWART
117498
LICENSED
PROFESSIONAL ENGINEER
5-7-2025

Huckabee
www.huckabee-inc.com
800.687.1229

STEEL DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S5.5
Drawn By: LAFP	
Date: 04/22/2025	

L.A. FUESS PARTNERS, INC.
Structural Engineers
3333 Lee Parkway, Suite 300 • Dallas, TX 75219
LAFP PROJ. NO. 24081 FIRM REG. NO. F-537



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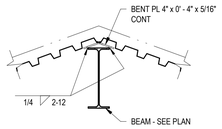

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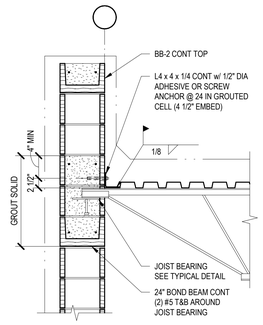

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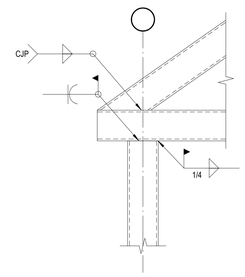

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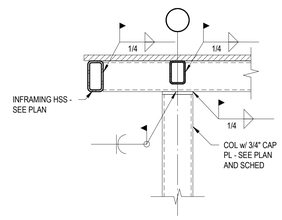

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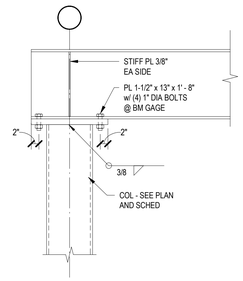

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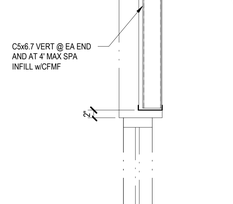

22
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18
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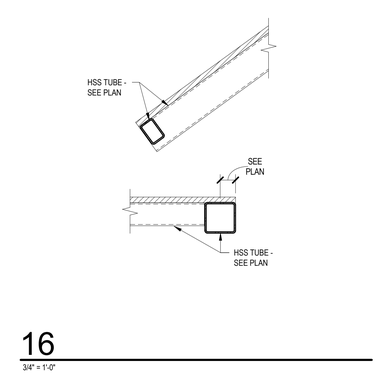

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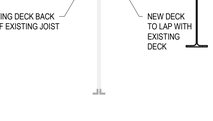

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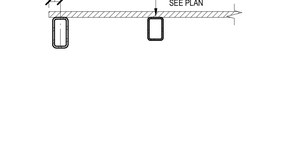

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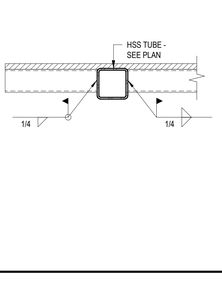

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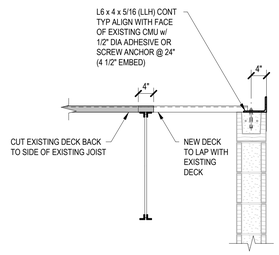

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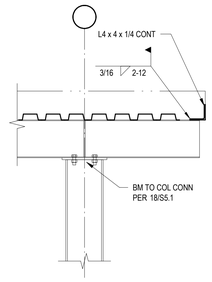

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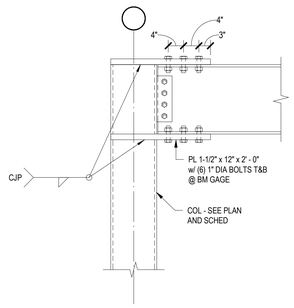

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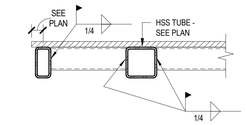

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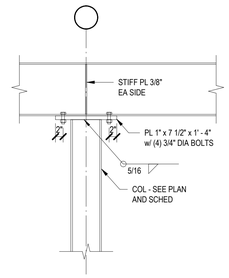
3/4" = 1'-0"



3
3/4" = 1'-0"

24
3/4" = 1'-0"

16
3/4" = 1'-0"

12
3/4" = 1'-0"

8
3/4" = 1'-0"

4
3/4" = 1'-0"

Revision / 1
Date 05/07/25
Addendum 1

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

CHERYL R. STEWART
117468
LICENSED
PROFESSIONAL ENGINEER
5-7-2025

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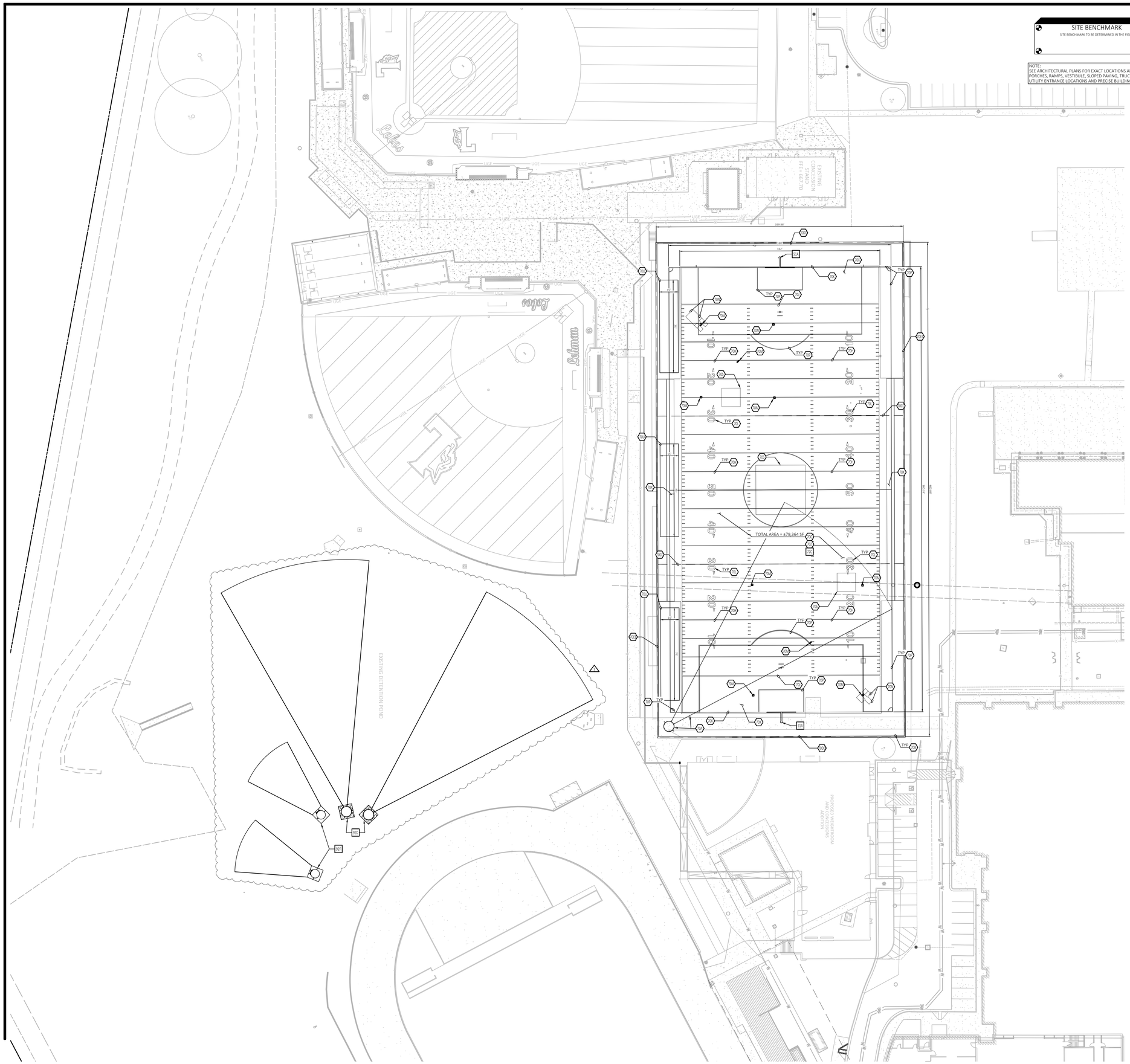
STEEL DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID S5.6
Drawn By: LAFP	
Date: 04/22/2025	

L.A. FUESS PARTNERS, INC.
Structural Engineers
3333 Lee Parkway, Suite 300 • Dallas, TX 75219
LAFP PROJ. NO. 24081 FIRM REG. NO. F-537



Revision /	Date
100% CD	02/26/2025
ADD-1	05/07/2025

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
BUDA, TX

JEFFERY J. BRESEE
F-7524
TX REGISTERED
ENGINEER
2025-05-06
JEFFERY J. BRESEE
TX 99277

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SURFACE PLAN - ALTERNATE 1

Job No.	Sheet No.
24206	100% CD F1
Drawn By: RM	
Date: 5/5/2025	

05-07-2025 ADD-1

PROJECT RELEASE TYPE

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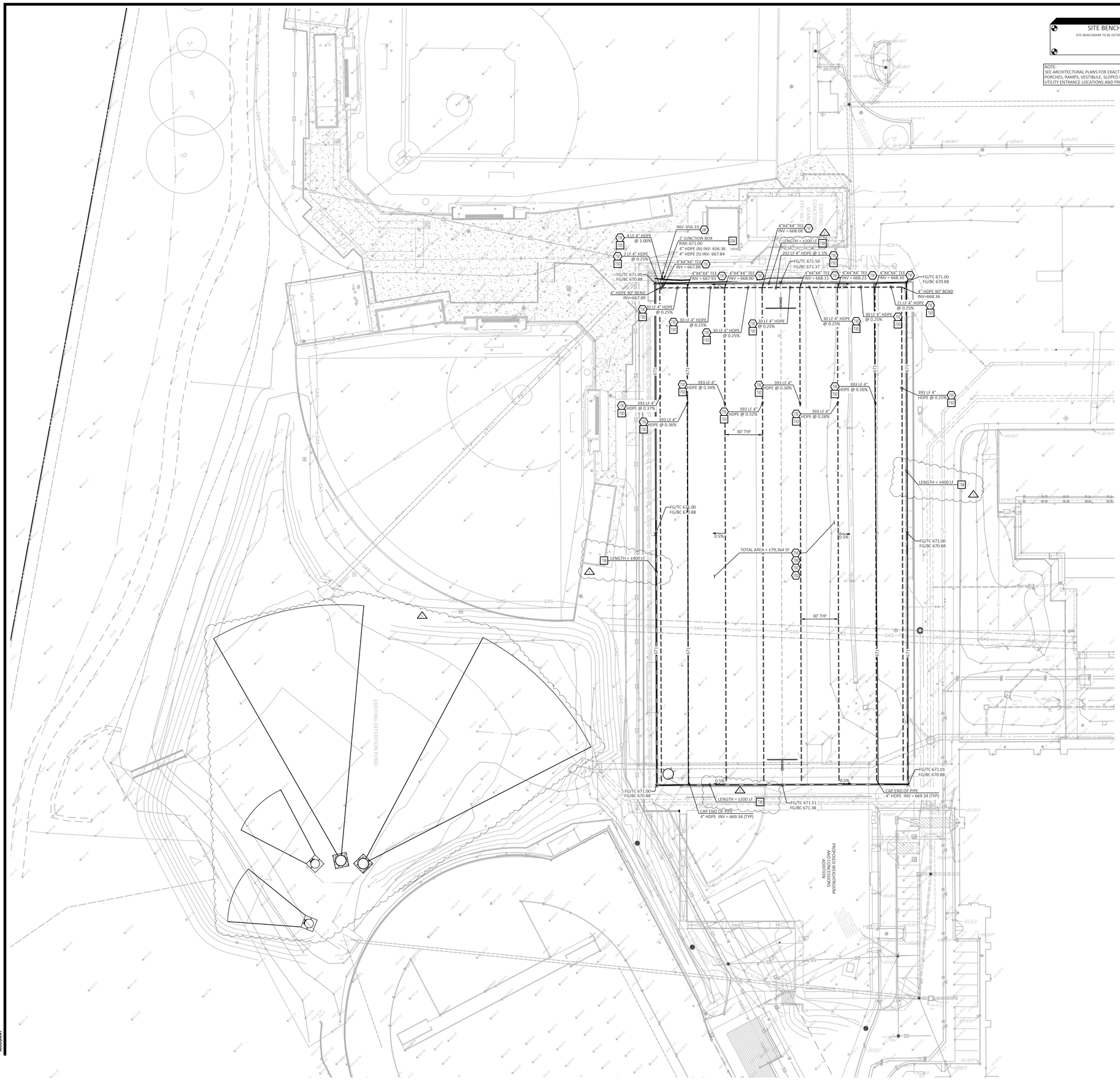

EXISTING LEGEND

- CLEANOUT
- FIRE HYDRANT
- LIGHT POLE
- SCOREBOARD
- TREE
- CHAINLINK FENCE LINE
- STORM DRAIN
- X"SS" SANITARY SEWER
- X"W" WATER

PROPOSED LEGEND

- PROPERTY LINE/RIGHT OF WAY LINE
- XXX CONTOUR ELEVATIONS
- GRADE BREAK
- FLOWLINE
- STORM DRAIN
- x XX.XX SPOT ELEVATIONS:
  TC = TOP OF CURB
  G = GUTTER
  FG = FINISH GRADE

GENERAL GRADING NOTES

A. PRIOR TO INSTALLATION OF STORM OR SANITARY SEWER, THE CONTRACTOR SHALL EXCAVATE, VERIFY, AND CALCULATE ALL CROSSINGS AND INFORM THE OWNER AND THE ENGINEER OF ANY CONFLICTS PRIOR TO CONSTRUCTION. THE ENGINEER WILL BE HELD HARMLESS IN THE EVENT THE ENGINEER IS NOT NOTIFIED OF DESIGN CONFLICTS.

B. ALL SLOPES AND AREAS DISTURBED BY CONSTRUCTION SHALL BE GRADED SMOOTH AND 4" OF TOPSOIL APPLIED. IF ADEQUATE TOPSOIL IS NOT AVAILABLE ON SITE, THE CONTRACTOR SHALL PROVIDE TOPSOIL, APPROVED BY THE OWNER, AS NEEDED. THE AREA SHALL THEN BE SEEDED, FERTILIZED, MULCHED, WATERED AND MAINTAINED UNTIL HARDY GRASS GROWTH IS ESTABLISHED IN ALL AREAS (SEE LANDSCAPE PLAN FOR SEED MIX AND PROPER APPLICATION RATE). ANY AREAS DISTURBED FOR ANY REASON PRIOR TO FINAL ACCEPTANCE OF THE PROJECT SHALL BE CORRECTED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.

C. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AT LEAST 48 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES.

GRADING NOTES

18E CONNECT TO NEW STORM DRAIN SYSTEM (REFER TO CIVIL DRAWINGS)

73A SOIL SHALL BE STABILIZED WITH 8 INCHES OF LIME STABILIZATION. SUB-GRADE SHALL BE STABILIZED AND COMPACTED SUBGRADE PER GEOTECH REPORT TO ACHIEVE PRV OF 1.5 INCHES.

73B PLACE AND COMPACT 4 INCH CRUSHED STONE MATERIAL (3 1/2' INCH TO BASE STONE AND 1/2 INCH OF FINISHED STONE AS SPECIFIED) OVER ENTIRE SURFACE OF FIELD WHERE SYNTHETIC TURF IS TO BE PLACED.

73C INSTALL 30 MIL GEOMEMBRANE LINER OVER ENTIRE SURFACE OF FIELD WHERE SYNTHETIC TURF IS TO BE PLACED. WRAP GEOMEMBRANE BEHIND TREX NAILER BOARD AT ALL TURF EDGES PER SPECIFICATIONS.

73D ANCHOR TURF TO CONCRETE CURB WITH 2 INCH X 4 INCH TREX NAILER BOARD AROUND ENTIRE PERIMETER OF SYNTHETIC TURF.

73E INSTALL 4" PERFORATED PIPES AT 30' O.C.

73F INSTALL TEE AT EACH DRAIN JUNCTION, DIAMETER AS INDICATED ON PLAN

GRADING DETAILS

20A JUNCTION BOX

73B CONCRETE FENCE SKIRT

73D PERFORATED STORM PIPE (TURF)

Revision /	Date
100% CD	02/26/2025
ADD-1	05/07/2025

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
BUDA, TX

JEFFERY J. BRESEE
F-7524
2025-05-06
JEFFERY J. BRESEE
TX 80217

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GRADING PLAN - ALTERNATE 1

Job No.	Sheet No.
34206	100% CD F2
Drawn By: RM	
Date: 5/5/2025	

05-07-2025 ADD-1

PROJECT RELEASE TYPE

5/6/2025 7:57 AM
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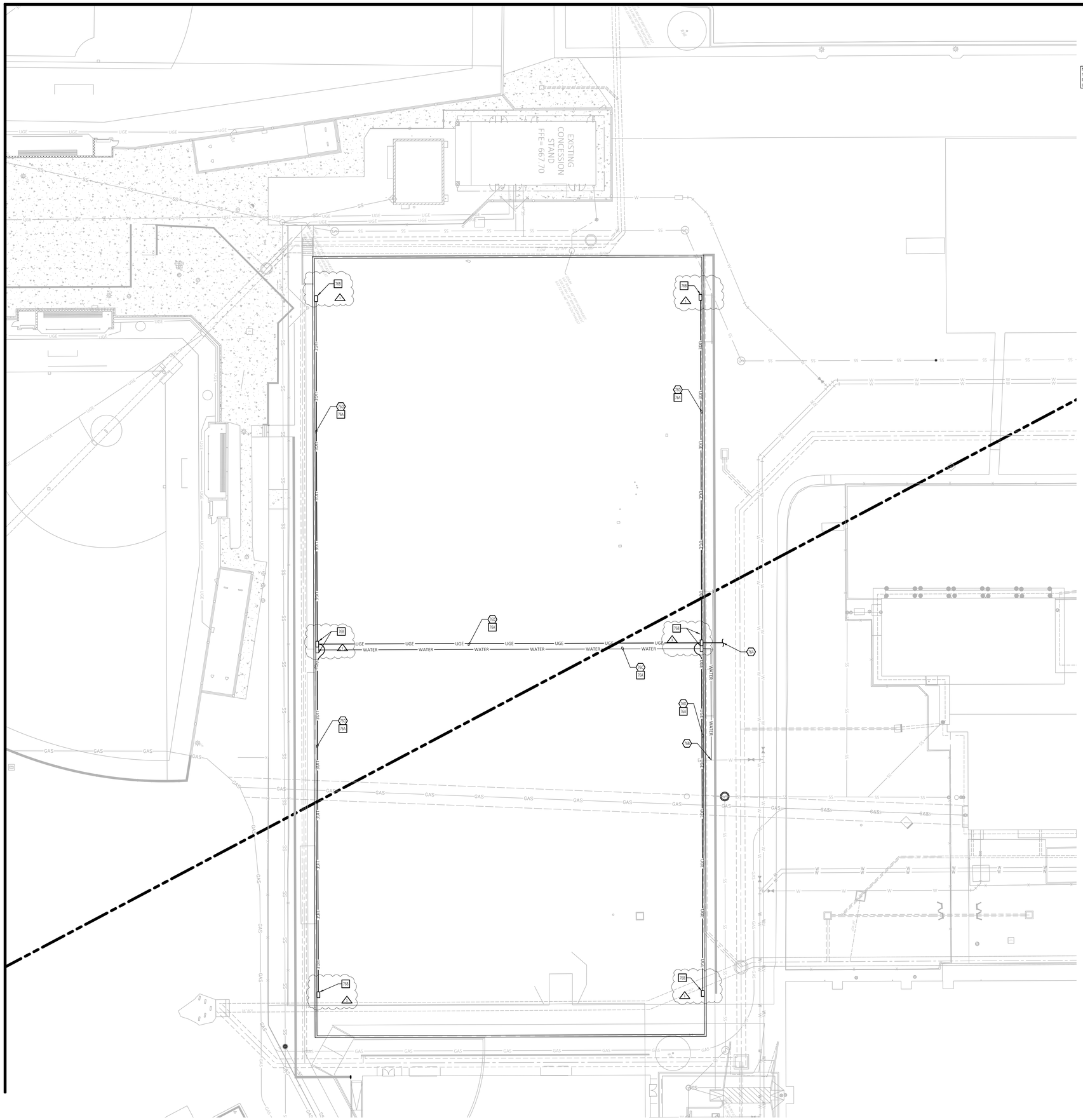

SITE BENCHMARK

SITE BENCHMARK TO BE DETERMINED IN THE FIELD.

NOTE:
SEE ARCHITECTURAL PLANS FOR EXACT LOCATIONS AND DIMENSIONS OF PORCHES, RAMPS, VESTIBULE, SLOPED PAVING, TRUCK DOCKS, BUILDING UTILITY ENTRANCE LOCATIONS AND PRECISE BUILDING DIMENSIONS.

0 20' 30' 40'
SCALE IN FEET

811 Know what's below. Call before you dig.

EXISTING LEGEND

+00	CLEANOUT		CHAINLINK FENCE LINE
	FIRE HYDRANT		STORM DRAIN
	LIGHT POLE	X"SS	SANITARY SEWER
	SCOREBOARD	X"W	WATER
	TREE		

PROPOSED LEGEND

	PROPERTY LINE/RIGHT OF WAY LINE
UGE	UNDERGROUND ELECTRIC SERVICE
X"W	WATER SERVICE

GENERAL UTILITY NOTES

A. CONTRACTOR SHALL COORDINATE ANY DISRUPTIONS TO EXISTING UTILITY SERVICES WITH ADJACENT PROPERTY OWNERS.

B. ALL ELECTRIC, TELEPHONE AND GAS EXTENSIONS INCLUDING SERVICE LINES SHALL BE CONSTRUCTED TO THE APPROPRIATE UTILITY COMPANY SPECIFICATIONS. ALL UTILITY DISCONNECTIONS SHALL BE COORDINATED WITH THE DESIGNATED UTILITY COMPANIES.

C. CONSTRUCTION SHALL NOT START ON ANY PUBLIC UTILITY SYSTEM UNTIL WRITTEN APPROVAL HAS BEEN RECEIVED BY THE ENGINEER FROM THE APPROPRIATE GOVERNING AUTHORITY AND CONTRACTOR HAS BEEN NOTIFIED BY THE ENGINEER.

D. PRIOR TO THE CONSTRUCTION OF OR CONNECTION TO ANY STORM DRAIN, SANITARY SEWER, WATER MAIN OR ANY OF THE DRY UTILITIES, THE CONTRACTOR SHALL EXCAVATE, VERIFY AND CALCULATE ALL POINTS OF CONNECTION AND ALL UTILITY CROSSINGS AND INFORM CEI ENGINEERING AND THE OWNER/DEVELOPER OF ANY CONFLICT OR REQUIRED DEVIATIONS FROM THE PLAN. NOTIFICATION SHALL BE MADE A MINIMUM OF 48 HOURS PRIOR TO CONSTRUCTION. CEI ENGINEERING AND ITS CLIENTS SHALL BE HELD HARMLESS IN THE EVENT THAT THE CONTRACTOR FAILS TO MAKE SUCH NOTIFICATION.

UTILITY NOTES

76A TIE IN ELECTRIC LINE TO NEAREST ELECTRIC SOURCE, REF ELECTRICAL PLANS FOR CONTINUATION.

76B TIE IN WATER LINE TO NEAREST POTABLE WATER SOURCE.

76C INSTALL 3 INCH Ø PVC CONDUIT WITH 1 INCH Ø POLYLINE WATERLINE.

76D INSTALL 1 1/2 INCH Ø PVC CONDUIT W/ELECTRIC LINES AND PULL STRINGS.

UTILITY DETAILS

76A PVC CONDUIT-CLASS "C" INCH PIPE BEDDING DETAIL

76B SYNTHETIC TURF UTILITY BOX

Revision /	Date
100% CD	02/26/2025
ADD-1	05/07/2025

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
BUDA, TX

TEXAS REGISTERED ENGINEERING FIRM F-7524

2025-05-06

F-7524

JEFFERY J. BRESEE
TX 90217

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UTILITY PLAN - ALTERNATE 1

Job No. 24206	Sheet No. 100% CD F3
Drawn By: RM	
Date: 5/5/2025	

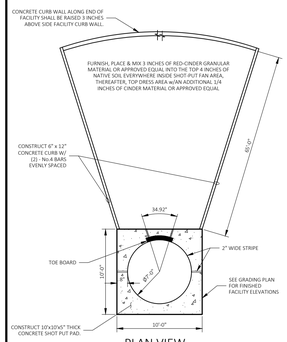
PROJECT RELEASE TYPE

05-07-2025 ADD-1

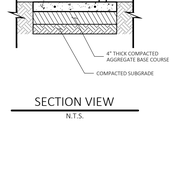
5/6/2025 7:57 AM

DRAWING LOCATION: P:\24000\24206.02\DRAWINGS\DESIGN\WORKING\24206-UP.DWG - SAVED BY - ARIDGWAY

NOTE:
1. CONSTRUCT SHOT PUT PAD IN ACCORDANCE WITH N.F.H.S. STANDARDS.
2. CONCRETE PAD SHALL BE APPROXIMATELY LEVEL.
3. SLOPE IN THROWING DIRECTION NOT TO EXCEED 1% LONGITUDINALLY AND 2% LATERALLY.
4. TOE BOARD SHALL BE 4' LONG, 4" TALL, AND 4-1/2" WIDE PER N.F.H.S. STANDARDS.



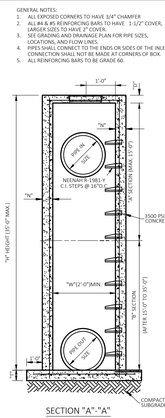
PLAN VIEW
N.T.S.



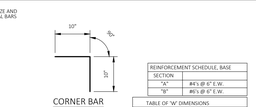
SECTION VIEW
N.T.S.

SHOT PUT PAD CONSTRUCTION
301

GENERAL NOTES:
1. ALL EXPOSED CORNERS TO HAVE 3/4" CHAMFER
2. #4 BAR CAGE REINFORCING BARS TO HAVE 1-1/2" COVER, LARGER SIZES TO HAVE 2" COVER.
3. SEE GRADING AND DRAINAGE PLAN FOR PIPE SIZES, LOCATIONS, AND FLOW LINES.
4. PIPES SHALL CONNECT TO THE ENDS OR SIDES OF THE INLET. CONNECTION SHALL NOT BE MADE AT CORNERS OF BOX.
5. ALL REINFORCING BARS TO BE GRADE 60.



JUNCTION BOX
304



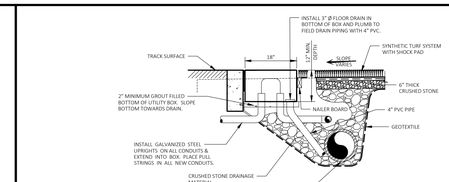
REINFORCEMENT SCHEDULE, BASE	
SECTION	
"A"	#4's @ 6" E.W.
"B"	#6's @ 6" E.W.

TABLE OF 'W' DIMENSIONS			
PIPE SIZE	SKEW OF CROSS DRAIN		
SINGLE	STRAIGHT	30°	45°
4'-24"	4'-0"	4'-0"	4'-10"
30"	4'-0"	4'-7"	5'-4"
36"	4'-0"	5'-3"	6'-5"
42"	5'-3"	5'-11"	7'-3"
48"	5'-10"	6'-7"	8'-0"
60"	7'-0"	7'-10"	9'-8"
DOUBLE	FOR "A" SECTION ONLY		
24"	7'-0"	7'-10"	9'-5"
30"	8'-2"	9'-2"	11'-0"
36"	9'-4"	10'-6"	12'-6"
42"	10'-6"	11'-10"	14'-2"
48"	11'-8"	13'-2"	15'-10"

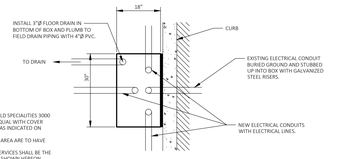
REINFORCEMENT SCHEDULE, WALLS			
SECTION	WIDTH ("W")	HOR.	VERT.
"A"	4'	#4's @ 9"	#4's @ 10"
	BETWEEN 4' & 7'	#5's @ 9"	#4's @ 10"
	GREATER THAN 7'	#5's @ 4 1/2"	#4's @ 10"
"B"	4'	#4's @ 6"	#4's @ 10"
	BETWEEN 4' & 7'	#6's @ 6"	#4's @ 10"

TABLE OF "T" & "H" DIMENSIONS				
SECTION	WIDTH ("W")	"T"	"H"	"D"
"A"	BETWEEN 4' & 7'	6" + PIPE THICKNESS	6"	6"
	GREATER THAN 7'	6" + PIPE THICKNESS	8"	8"
"B"	4'	6" + PIPE THICKNESS	8"	8"
	BETWEEN 4' & 7'	6" + PIPE THICKNESS	10"	8"

REINFORCEMENT SCHEDULE, TOP		
DIMENSIONS	STEEL	SPECIAL PATTERN
W1 = 7' OR LESS	#4's @ 8" E.W.	DIAGONAL @ COVER
W1 > 7' OR LESS	#4's @ 8" E.W.	DIAGONAL @ COVER
W1 = 7' OR LESS	#4's @ 8" E.W.	DIAGONAL @ COVER
W2 = 7' OR GREATER	#4's @ 8" E.W.	DIAGONAL @ COVER
W1 = 7' OR GREATER	#4's @ 8" E.W.	DIAGONAL @ COVER
W2 = 7' OR GREATER	#4's @ 8" E.W.	DIAGONAL @ COVER



UTILITY BOX SECTION
N.T.S.

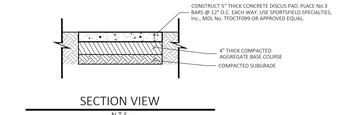


NOTE:
1. UTILITY BOXES - SPORTSFIELD SPECIALITIES 3000 COMBOX OR APPROVED EQUAL WITH COVER COMPATIBLE TO SURFACE AS INDICATED ON PLANS.
2. BOXES IN SYNTHETIC TURF AREA ARE TO HAVE SYNTHETIC TURF LIDS.
3. UTILITY BOX FOR WATER SERVICES SHALL BE THE SAME CONFIGURATION AS SHOWN HEREON, BUT SHALL HAVE A DRY-BARREL WATER HYDRANT INSTALLED ON A VALVE AS SPECIFIED ON THE UTILITY PLAN.
4. UTILITY BOXES FOR ELECTRIC SHALL HAVE WEATHER RESISTANT 110V ELECTRICAL RECEPTACLE WITH PIT TAIL CONFIGURATION.

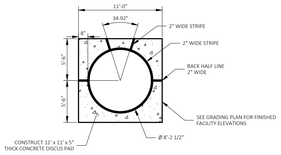
CONDUIT LAYOUT
N.T.S.

SYNTHETIC TURF UTILITY BOX
305

NOTE:
1. CONSTRUCT DISCUS PAD IN ACCORDANCE WITH N.F.H.S. STANDARDS.
2. CONCRETE PAD SHALL BE APPROXIMATELY LEVEL.
3. SLOPE IN THROWING DIRECTION NOT TO EXCEED 1% LONGITUDINALLY AND 2% LATERALLY.



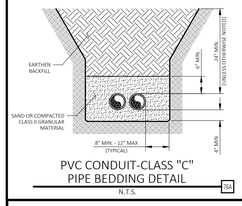
SECTION VIEW
N.T.S.



PLAN VIEW
N.T.S.

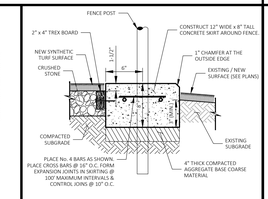
DISCUS PAD CONSTRUCTION
303

NOTE:
BACK FILL TO BE MECHANICALLY TAMPED IN 6" LIFTS TO 95% OF MAXIMUM STANDARD PROCTOR DRY DENSITY UNDER DRIVE LOCATIONS.



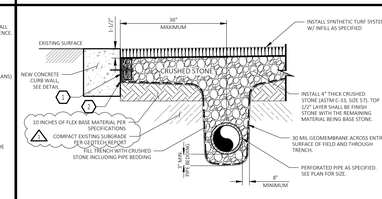
PVC CONDUIT-CLASS "C" PIPE BEDDING DETAIL
N.T.S.

302



CONCRETE FENCE SKIRT
N.T.S.

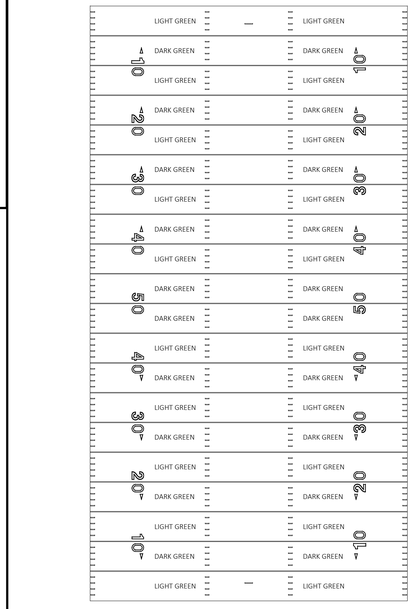
308



NOTES INDICATED BY NUMBER IN ◇
1. INSTALL COMPOSITE NAILER BOARD (2 X 4 TREX BOARD, OR APPROVED EQUAL) ON TOP OF CONCRETE EDGES. ANCHOR SYNTHETIC TURF TO NAILER BOARD.
2. WRAP GEOMEMBRANE BETWEEN NAILER BOARD AND CONCRETE FENCE SKIRT.

PERFORATED STORM PIPE
N.T.S.

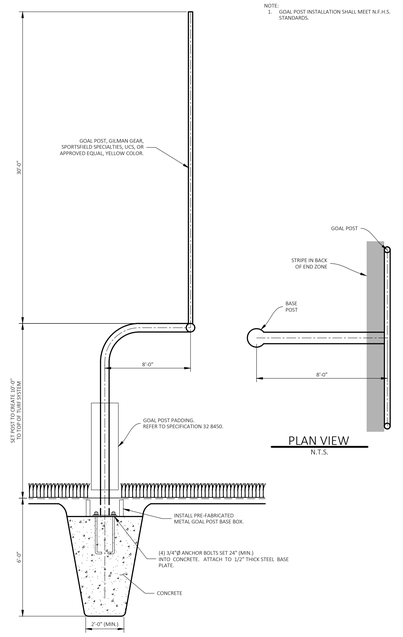
310



FOOTBALL FIELD ALTERNATING GREEN
N.T.S.

306

NOTE:
1. GOAL POST INSTALLATION SHALL MEET N.F.H.S. STANDARDS.



PLAN VIEW
N.T.S.

GOAL POST INSTALLATION
N.T.S.

314

Revision /	Date
100% CD	02/26/2025
ADD-1	05/07/2025

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
BUDA, TX

JEFFERY J. BRESEE
TEXAS
F-7524
REGISTERED
ENGINEER
2025-05-06

F-7524
JEFFERY J. BRESEE
TX 88217

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DETAIL SHEET - ALTERNATE 1

Job No.	Sheet No.
34200	100% CD
Drawn By: RM	F4
Date: 5/5/2025	

PROJECT RELEASE TYPE

05-07-2025 ADD-1

5/6/2025 7:57 AM
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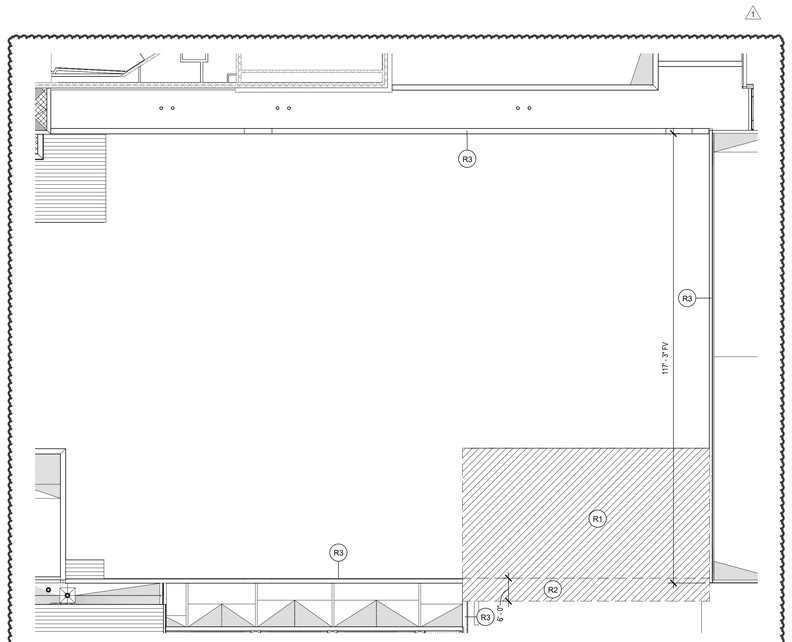
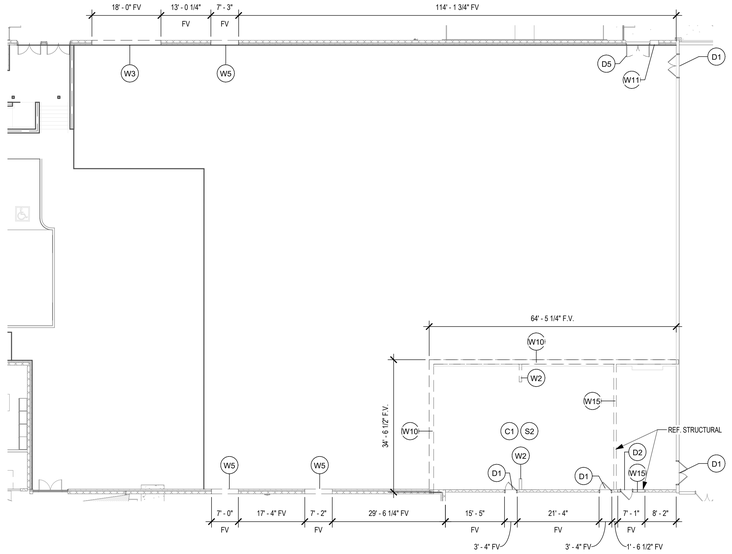


2 DEMOLITION PLAN, AREA B, ROOF
1/16" = 1'-0"



1 DEMOLITION PLAN, AREA B, LEVEL 1
1/16" = 1'-0"

EXTERIOR

E1. SAW CUT & REMOVE EXISTING CONC SIDEWALK, CURB OR FLAT WORK

E2. SAW CUT & REMOVE EXIST CONC LANDING / STEPS & RAILING & RETAINING WALL

E3. REMOVE EXISTING GUTTERS, DOWNSPOUTS & SPLASH BLOCKS

E4. REMOVE EXIST BRICK/CONC SCREEN WALL & FOUNDATION

E5. REMOVE EXISTING CANOPY SYSTEM, CONC. FOUNDATION, AND ASSOCIATED COMPONENTS, TRIMS, ATTACHMENTS, AND HARDWARE

E6. REMOVE EXIST FLAGPOLE & BASE, AND ASSOCIATED COMPONENTS, OPERATIONAL MECHANISMS, ATTACHMENTS, HARDWARE, AND LIGHTING

FLOORS

F1. REMOVE EXISTING TILE FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F2. REMOVE EXISTING CARPETED FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F3. REMOVE EXISTING VCT FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F4. SAW CUT EXISTING FLOOR SLAB AS REQ'D. FOR INSTALLATION OF NEW PLUMBING. REF PLUMBING DRAWINGS.

F5. SHOT BLAST EXISTING CONC SURFACE FOR PREPARATION OF NEW SCHEDULED FINISH

F6. SAW CUT & REMOVE EXISTING CONC RAMP

F7. SAW CUT & REMOVE PORTION OF EXISTING CONC. SLAB AS SHOWN - REF. STRUCTURAL DRAWINGS

F8. SAW CUT & REMOVE EXISTING FLOOR & CONC SLAB - REMOVE GRADE BEAMS & PIERS TO A DEPTH OF 3'-0" BELOW PROPOSED NEW GRADE AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

F9. CORE EXISTING TWO-WAY CONCRETE SLAB OVER CRAWLSPACE WITH THE MINIMUM DIAMETER CORE POSSIBLE, NOT TO EXCEED 8', WHERE NEEDED FOR NEW SANITARY SEWER PIPING ONLY AFTER USING A HILTI PS-200 FERROSCAN TO LOCATE EXISTING TOP AND BOTTOM BARS IN THE SLAB AND CENTERING THE CORE BETWEEN EXISTING BARS. IF IT IS CONSIDERED NECESSARY TO CUT THROUGH ANY EXISTING REINFORCEMENT, THE CONTRACTOR SHALL SUBMIT AN RFI ASKING FOR VERIFICATION THAT IT IS ACCEPTABLE TO CUT THE BAR.

WALLS

W1. REMOVE EXISTING GYP PARTITION, INCLUDING ANYTHING CONTAINED WITHIN THE PARTITION. CAP OFF / TERMINATE UTILITIES THEREIN

W2. REMOVE EXISTING MASONRY PARTITION, INCLUDING ANYTHING CONTAINED WITHIN THE PARTITION. CAP OFF OR TERMINATE UTILITIES THEREIN

W3. SAW CUT & REMOVE PORTION OF EXISTING MASONRY WALL INDICATED

W4. CUT IN NEW DOOR/WINDOW OPENING IN EXISTING GYP WALL, REF PLAN FOR TYPE

W5. SAW CUT EXISTING MASONRY WALL FOR NEW DOOR/WINDOW OPENING

W6. SAW CUT NEW RECTANGULAR HOLE THROUGH EXISTING WALL FOR HVAC DUCT, REF. MECH. FOR DUCT SIZE

W7. REMOVE EXISTING CERAMIC WALL TILE IN ENTIRE ROOM AND/OR SPACE & PREPARE EXISTING WALL FOR NEW TILE

W8. REMOVE PORTION OF EXISTING FACE BRICK FROM WALL AS INDICATED

W9. REMOVE PORTION OF EXISTING CONC. TILT-WALL PANEL, SHORE AND BRACE AND PREPARE FOR NEW CONSTRUCTION.

W10. REMOVE EXISTING EXTERIOR WALL, INCLUDING BASE, DOORS, WINDOWS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE WALL. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING WALLS AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW CONSTRUCTION AS SHOWN ON PLANS.

W11. REMOVE PORTION OF EXISTING MASONRY WALL TO 1 1/2" BELOW ADJACENT FINISH FLOOR LEVEL. HONE CUT SURFACE SMOOTH AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

W12. REMOVE PORTION OF EXISTING EXTERIOR METAL STUD WALL WITH MASONRY VENEER. SHORE AND BRACE AS REQUIRED AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

W13. REMOVE EXISTING INTERIOR WALL PANELING AND/OR EXISTING VINYL WALL COVERING AND SUBSTRATE AND PROVIDE NEW GYP. BOARD AT CORRIDOR SIDE OF WALL. PREPARE WALL TO RECEIVE NEW FINISH AS SHOWN ON FINISH PLANS.

W14. REMOVE EXTERIOR MASONRY VENEER AND SUBSTRATE FROM EXISTING METAL STUD WALL. PREPARE FOR NEW SUBSTRATE AND MASONRY AS SHOWN.

W15. REMOVE EXISTING PARTITION (SHOWN DASHED, TYPICAL), INCLUDING BASE, DOORS, WINDOWS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE PARTITION. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PARTITIONS AND PREPARE TO RECEIVE NEW AS SHOWN ON PLANS.

W16. SHORE, BRACE, AND SAW CUT NEW OPENING INTO EXISTING WALL. PREPARE OPENING TO RECEIVE NEW DOOR, FRAME, AND HARDWARE AS SCHEDULED.

W17. SHORE, BRACE, AND SAW CUT NEW OPENING INTO EXISTING WALL. PREPARE OPENING TO RECEIVE NEW LINTEL AND FINISHES AS SPECIFIED.

W18. REMOVE EXISTING CERAMIC TILE AND BACKING LAYER OF GYP. BOARD. REPLACE GYP. BD. WITH NEW GYP. BD. (TILE BACKER BD. AT NEW TILE FINISH) TO CREATE LEVEL WALL WITH ANY ADJACENT EXISTING GYP. WALL BOARD AND PREPARE FOR NEW FINISHES AS SCHEDULED.

W19. PILASTER TO REMAIN.

CEILINGS

C1. REMOVE EXISTING CEILING GRID, TILE, GRILLES, DIFFUSERS, LIGHTING, EQUIPMENT, DEVICES, AND ALL OTHER ITEMS OCCURRING WITHIN THE CEILING PLANE. REMOVE ANY CEILING TRIM WHEREVER IT OCCURS. PREPARE ADJACENT WALLS TO RECEIVE NEW CEILING AS SHOWN ON REFLECTED CEILING PLANS. REF. ELEC.

C2. REMOVE EXISTING PLASTER CEILING SYSTEM AND ASSOCIATED TRIMS, FRAMING, HANGERS, AND HARDWARE, LIGHTING, AND ALL OTHER ITEMS OCCURRING WITHIN THE CEILING PLANE. PREPARE ADJACENT WALLS TO RECEIVE NEW CEILING AS SHOWN ON REFLECTED CEILING PLANS. REF. ELEC.

C3. REMOVE EXISTING ACOUSTICAL CEILING. REMOVE LIGHTING, EQUIPMENT, DEVICES, AND OTHER ITEMS ATTACHED TO CEILING.

C4. REMOVE EXISTING CELLULOSE FIREPROOFING AT DECK & STRUCTURAL FRAMING. POWER WASH REMOVAL IS ACCEPTABLE.

C5. REMOVE EXISTING WOOD PANEL CEILING CLOUDS. PROTECT & STORE FOR REINSTALLATION.

NOTES BY SYMBOL

CEILINGS CONT-

C6. REMOVE EXISTING CEILING SYSTEM IN ENTIRE ROOM AND/OR SPACE. INCLUDING, BUT NOT LIMITED TO, LIGHT FIXTURES, HVAC DIFFUSERS, CEILING-MOUNTED EQUIPMENT, AND CEILING-MOUNTED DEVICES.

C7. REMOVE EXISTING LAY-IN CEILING GRID IN ENTIRE ROOM AND/OR SPACE. RETAIN LIGHTS, HVAC ETC. SUPPORT IN PLACE FOR REINSTALLATION IN NEW CEILING.

C8. REMOVE EXISTING LIGHTS (PULL WIRE BACK TO ELECT PANEL).

C9. REMOVE EXISTING CEILING TILES IN ENTIRE ROOM AND/OR SPACE.

C10. REMOVE EXISTING FURRDOWNS/BULKHEADS ABOVE.

C11. REMOVE PORTION OF CEILING GRID AS REQ'D FOR INSTALLATION OF FIRE SPRINKLER SYSTEM / FIRE ALARM DEVICES.

C12. REMOVE PORTION OF GYP BD CEILING AS REQ'D FOR INSTALLATION OF FIRE SPRINKLER SYSTEM / FIRE ALARM DEVICES. PATCH & TEXTURE CEILING TO MATCH EXISTING.

C13. REMOVE PORTION OF EXISTING CEILING SYSTEM FOR INSTALLATION OF ROOF DRAINS, REF PLUMBING.

C14. REMOVE EXISTING PLASTER CEILING / SOFFIT AND ASSOCIATED TRIMS, FRAMING HANGERS, AND HARDWARE

DOORS & WINDOWS

D1. REMOVE EXISTING DOOR AND/OR WINDOW, FRAME, AND HARDWARE (SHOWN DASHED, TYPICAL). PATCH, REPAIR, AND CLEAN OPENING AND PREPARE TO INFILL OPENING AS SHOWN ON FLOOR PLANS.

D2. REMOVE EXISTING DOOR AND/OR WINDOW AND FRAME (SHOWN DASHED, TYPICAL). PATCH, REPAIR AND CLEAN OPENING AND PREPARE FOR NEW DOOR OR WINDOW AS SHOWN ON FLOOR PLANS.

D3. REMOVE EXISTING OVERHEAD DOOR, TRACKS, HARDWARE, OPERATIONAL MECHANISMS, AND ASSOCIATED SUPPORTS. PREPARE WALL FOR NEW FINISH.

D4. REMOVE EXISTING DOOR (FRAME TO REMAIN, PREP FOR PAINTING AND NEW HARDWARE).

D5. REMOVE EXISTING DOOR, FRAME, AND ASSOCIATED HARDWARE.

D6. REMOVE EXISTING DOOR, INFILL HINGE LOCATIONS TO A SMOOTH FINISH, AND PREP FOR PAINTING.

D7. REMOVE EXISTING OVERHEAD DOOR, TRACKS, HARDWARE, OPERATIONAL MECHANISMS, AND ASSOCIATED SUPPORTS.

D8. REMOVE EXISTING DOOR FRAME. SALVAGE DOOR & PREP DOOR FOR NEW HARDWARE.

D9. REMOVE EXISTING WINDOWS.

D10. REMOVE EXISTING ALUM STOREFRONT SYSTEM OR CURTAIN WALL SYSTEM, GLAZING, GASKETS, HARDWARE, AND ASSOCIATED BRACING.

D11. REMOVE EXISTING WINDOW, SAW CUT WALL BELOW FOR INSTALLATION OF NEW DOOR

MISCELLANEOUS

M1. REMOVE AND DISPOSE OF EXISTING CASEWORK AND ACCESSORIES.

M2. REMOVE EXISTING TOILET PARTITIONS AND ALL ASSOCIATED EQUIPMENT, ACCESSORIES, AND HARDWARE IN THEIR ENTIRETY AND DELIVER TO OWNER.

M3. REMOVE EXISTING LOCKERS AND DELIVER TO OWNER.

M4. REMOVE EXISTING COLUMN COVERS AND DELIVER TO OWNER.

M5. REMOVE EXISTING CONCRETE STAIRS OR RAMP AND ASSOCIATED HANDRAILS AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

M6. REMOVE EXISTING WIRE MESH PARTITIONS AND DELIVER TO OWNER.

M7. REMOVE EXISTING CHAIN LINK FENCE PARTITION

M8. REMOVE EXISTING HANDRAILS AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

M9. REMOVE EXISTING MASONRY PLANTERS

M10. REMOVE EXISTING MARKERBOARD OR TACKBOARD. RELOCATE TO NEW LOCATION, REF. PLANS

M11. REMOVE EXISTING MILLWORK, CABINETS AND OR BRACKETS

M12. REMOVE EXISTING ELECTRICAL SERVICE

M13. REMOVE EXISTING THEATRICAL RIGGING, SOUND, LIGHTING EQUIP & SPEAKERS LOCATED WITHIN STAGE AREA

M14. REMOVE EXISTING METAL LOCKERS

M15. REMOVE & RELOCATE EXISTING METAL LOCKERS, REF PLAN

M16. REMOVE EXISTING LOCKER BASE, LOCKERS & BENCHES

M17. REMOVE EXISTING FOLDING PANEL PARTITION

M18. REMOVE EXISTING TOILET ACCESSORIES. PATCH WALL AS REQ'D.

M19. REMOVE EXISTING HVAC UNIT. CAP OFF W/ NEW SHEET MTL & INSULATE OPENING.

M20. REMOVE EXISTING HVAC UNIT & CURB, INFILL OPENING W/ NEW METAL DECK & NEW RIGID INSUL TO MATCH EXISTING ROOF TO FLUSH W/ EXISTING. PROVIDE ADDITIONAL SUPPORT STRUCTURE PER STRUCTURAL.

M21. REMOVE EXISTING SOUND ISOLATION PRACTICE ROOMS; INCLUDING BASE, DOORS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE WALL PARTITION. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING WALLS AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW SOUND ISOLATION ROOMS AS SHOWN ON PLANS.

PLUMBING

P1. REMOVE EXISTING PLUMBING FIXTURES, BRACKETS, AND ACCESSORIES. TERMINATE OR RELOCATE UTILITIES AS REQUIRED. REF. PLUMBING.

P2. REMOVE EXISTING PLUMBING FIXTURES AND REPLACE WITH NEW FIXTURES IN THE SAME LOCATION. REF. PLUMBING.

P3. REMOVE EXISTING PLUMBING FIXTURE, TO BE REPLACED W/ NEW FIXTURE

ROOF

R1. REMOVE PORTION OF EXISTING ROOF, DECK, WALKWAY COVER, AND/ OR CANOPY AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

R2. REMOVE PORTION OF EXISTING ROOF COVER AND INSULATION. PREPARE TO RECEIVE NEW ROOF WITH NEW WORK.

R.3 REMOVE COPING, AND DAMAGED BLOCKING TO PREPARE FOR NEW WORK. PROTECT AND PRESERVE UNDAMAGED FLASHING, ROOFING MEMBRANE, AND BLOCKING. COORDINATE WITH NEW WORK.

STRUCTURAL

S1. REMOVE EXISTING CONCRETE COLUMN. REF. STRUCTURAL DRAWINGS FOR BRACING AND COLUMN REPLACEMENT IF APPLICABLE.

S2. SAW CUT EXISTING CONCRETE SLAB. REF. STRUCTURAL PLANS FOR EXTENTS AND DETAILS.

S3. REMOVE EXISTING STEEL COLUMN. REF. STRUCTURAL DRAWINGS FOR BRACING AND COLUMN REPLACEMENT IF APPLICABLE.

NOTES BY SYMBOL

A. BIDDERS SHALL VISIT SITE AND BE FAMILIAR WITH EXISTING CONDITIONS, INCLUDING BUT NOT LIMITED TO EXISTING DIMENSIONS, EQUIPMENT, LOCATIONS, SIZES, QUANTITIES, AND MATERIALS.

B. EXISTING ELECTRICAL POWER SERVING THE EXISTING FACILITY SHALL REMAIN ON LINE. DISRUPTIONS REQUIRED FOR CONSTRUCTION TO BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

C. EXISTING DRAIN LINES SERVING THE EXISTING FACILITY SHALL REMAIN FUNCTIONAL. ANY DISRUPTIONS REQUIRED FOR NEW TIE-INS DURING CONSTRUCTION MUST BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

D. CONTRACTOR SHALL COORDINATE EXACT SIZES AND LOCATIONS FOR MECHANICAL, PLUMBING, ELECTRICAL, AND TECHNOLOGY PENETRATIONS REQUIRED FOR NEW WORK WITH EACH RESPECTIVE TRADE.

E. CONTRACTOR SHALL KEEP OPENINGS TO THE EXTERIOR TEMPORARILY COVERED FOR PROTECTION FROM WATER.

F. CONTRACTOR SHALL KEEP OPENINGS TEMPORARILY COVERED FOR PEDESTRIAN SAFETY.

G. CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF TEMPORARY SHORING AND BRACING REQUIRED FOR DEMOLITION.

H. REFER TO MECHANICAL, ELECTRICAL, TECHNOLOGY, AND PLUMBING SHEETS FOR ADDITIONAL PENETRATIONS AND OTHER RELATED DEMOLITION OR EQUIPMENT REMOVAL.

I. EXISTING CONSTRUCTION IS SHOWN BASED ON OWNER FURNISHED PLANS, OWNER FURNISHED SURVEYS, AND ONSITE OBSERVATIONS. DISCREPANCIES BETWEEN DRAWINGS AND ACTUAL FIELD CONDITIONS WILL BE REPORTED TO THE ARCHITECT/ENGINEER/PROJECT MANAGER AND ANY ADDITIONAL DIRECTION, IF NECESSARY, SHALL BE OBTAINED FROM THE ARCHITECT PRIOR TO PROCEEDING WITH WORK.

J. EXISTING CONSTRUCTION ADJACENT TO DEMOLITION WORK WILL BE PATCHED AND REPAIRED TO MATCH ORIGINAL CONDITION.

K. DEMOLITION WORK TIMES SHALL BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

L. THIS EXISTING FACILITY SHALL REMAIN FUNCTIONAL DURING THE COURSE OF THE DEMOLITION WORK. CONTRACTOR WILL MAINTAIN DUST BARRIERS, BARRICADES, PEDESTRIAN PROTECTION, WATER PROTECTION, AND SAFETY DEVICES IN PLACE AT ALL TIMES DURING AND AFTER DEMOLITION UNTIL NEW WORK IS INSTALLED AND ACCEPTED BY OWNER.

M. CONTRACTOR SHALL MAINTAIN SITE DRAINAGE DEVICES AND COMPONENTS DURING THE COURSE OF DEMOLITION AND UP UNTIL NEW WORK IS IN PLACE AND ACCEPTED BY OWNER. THIS INCLUDES, BUT IS NOT LIMITED TO, EXISTING SUB-SURFACE DRAINAGE.

N. REFER TO ROOF PLAN AND MECHANICAL, ELECTRICAL, PLUMBING, TECHNOLOGY AND ANY OTHER DISCIPLINES' DRAWINGS FOR ADDITIONAL REQUIRED DEMOLITION NOT SPECIFICALLY COVERED BY ARCHITECTURAL DEMOLITION PLANS, ESPECIALLY WORK RELATED TO MODIFICATIONS TO EXISTING H.V.A.C. SYSTEMS.

O. DEMOLITION, AS NOTED, CONSISTS OF COMPLETE REMOVAL OF THE LISTED ITEMS, RELATED FASTENERS, AND ATTACHMENT MATERIALS LEAVING A CLEAN SURFACE READY TO RECEIVE NOTED MATERIALS OR SCHEDULED FINISHES.

P. UNLESS NOTED OTHERWISE, OR OWNER EXPRESSES A DESIRE TO RETAIN DEMOLISHED ITEMS BEFORE THEY ARE REMOVED FROM THE SITE, ALL DEMOLISHED MATERIALS NOT RETAINED BY OWNER SHALL BE REMOVED FROM THE SITE AND DISPOSED OF AT CONTRACTOR'S SOLE EXPENSE. DISPOSAL SHALL COMPLY WITH ALL CODES AND LAWS, INCLUDING LOCAL, STATE AND FEDERAL LAWS. DURING DEMOLITION, CONTRACTOR SHALL CAREFULLY REMOVE ANY ITEMS FROM THE EXISTING CONSTRUCTION THAT THE OWNER HAS EXPRESSED A DESIRE TO RETAIN AND, IF NECESSARY, TEMPORARILY PROTECT AND STORE THE ITEMS AT THE SITE. THE OWNER MAY REMOVE RETAINED ITEMS FROM THE SITE OR, AS A PART OF THIS CONTRACT AND AT NO ADDITIONAL COST, IF REQUESTED BY OWNER, THE CONTRACTOR SHALL DELIVER OWNER-RETAINED ITEMS TO A LOCAL LOCATION SELECTED BY THE OWNER AND OFFLOAD AND PLACE THE ITEMS FOR STORAGE AS DIRECTED BY THE OWNER.

Q. DEMOLITION SHALL FOLLOW THE CONSTRUCTION SCHEDULE PROVIDED IN THE SPECIFICATIONS.

R. CONTRACTOR SHALL TAKE ALL PRECAUTIONS TO SEPARATE STUDENTS, SCHOOL STAFF, AND VISITORS FROM DEMOLITION AND CONSTRUCTION EFFORTS, ACTIVITIES AND WORK, AS MAY BE NECESSARY TO PROTECT THEIR HEALTH AND SAFETY.

S. ADDITIONAL MATERIALS, WHERE CONSTRUCTION ADJOINS EXISTING, EVEN THOUGH IT MAY NOT BE SPECIFICALLY IDENTIFIED IN THE DEMOLITION PLANS, WILL BE REMOVED AS REQUIRED TO COMPLETE THE CONSTRUCTION. THESE MATERIALS TYPICALLY INCLUDE, BUT ARE NOT NECESSARILY LIMITED TO, MATERIALS PROJECTING BEYOND THE FACE OF THE WALL, SUCH AS BRICK SILLS AND ROOF FLASHINGS, GUTTERS, AND TRIMS. REFER TO WALL SECTIONS FOR SPECIFIC CONDITIONS.

T. INVESTIGATE EXISTING CONSTRUCTION SUBJECT TO DEMOLITION TO DETERMINE IF IT IS INTENDED AS STRUCTURE-BEARING. COORDINATE WITH CONSTRUCTION SEQUENCE AND PROVIDE SHORING AND ANY OTHER TEMPORARY SUPPORT FOR ANY FEATURES CARRYING STRUCTURAL LOAD TO PREVENT ANY CHANGE IN STRUCTURAL INTEGRITY UNTIL NEW OR REPLACEMENT STRUCTURAL SUPPORT IS IN PLACE.

U. REFER TO TECHNOLOGY SHEETS FOR DEMOLITION OF TECHNOLOGY AND TECHNOLOGY-RELATED ITEMS.

DEMOLITION GENERAL NOTES

EXISTING PARTITION TO BE REMOVED. INCLUDING BASE, DOORS, WINDOWS, OUTLETS, DEVICES, SWITCHES, CHALKTACK BOARDS, POWER POLES, WALL CAPS AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE PARTITION. CONTRACTOR WILL VERIFY ELECTRICAL, MECHANICAL, & PLUMBING IN EXISTING WALLS & TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR & CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW PARTITION WALLS AS SHOWN ON FLOOR PLANS.

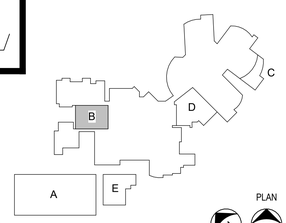
EXISTING DOOR, FRAME, AND HARDWARE TO BE REMOVED UNLESS OTHERWISE NOTED.

EXISTING ITEM TO BE REMOVED AS NOTED.

EXISTING WALL / PARTITION TO REMAIN.

EXISTING DOOR, FRAME, AND HARDWARE TO REMAIN.

DEMOLITION LEGEND



KEY PLAN
NTS

PLAN
N
TRUE

Revision /	Date	
1	05/08/25	ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

05.08.25

Huckabee
www.huckabee-inc.com
800.687.1229

DEMOLITION FLOOR PLAN - AREA B

PACKAGE VOLUME

Job No. 01954-08-01	Sheet No. ISSUE FOR BID AD1.01
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Date: 05/08/2025	

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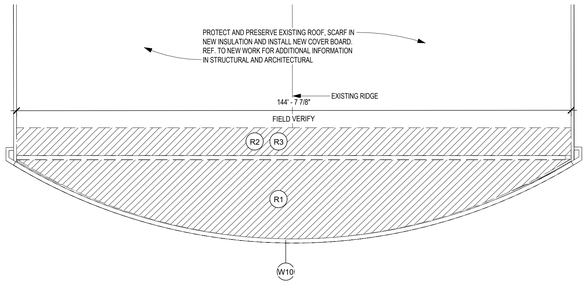
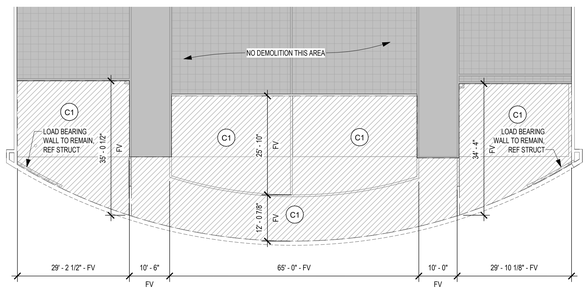


3 DEMOLITION PLAN, AREA C, ROOF
1/16" = 1'-0"



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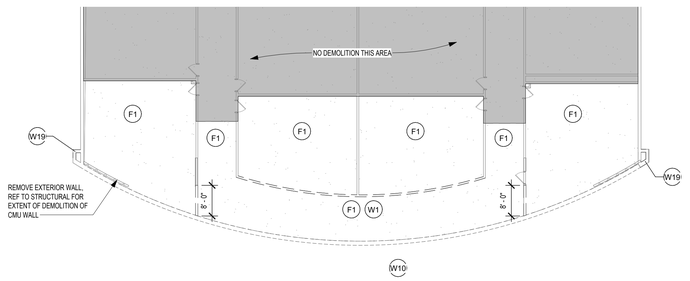
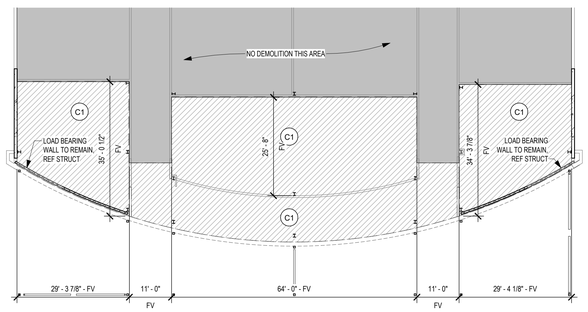
1/16" = 1'-0"



2 DEMOLITION PLAN, AREA C, LEVEL 2
1/16" = 1'-0"



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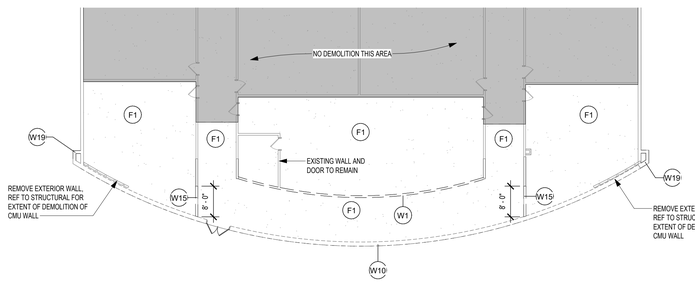
1/16" = 1'-0"



1 DEMOLITION PLAN, AREA C, LEVEL 1
1/16" = 1'-0"

EXTERIOR

E1. SAW CUT & REMOVE EXISTING CONC SIDEWALK, CURB OR FLAT WORK

E2. SAW CUT & REMOVE EXIST CONC LANDING / STEPS & RAILING & RETAINING WALL

E3. REMOVE EXISTING GUTTERS, DOWNSPOUTS & SPLASH BLOCKS

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E6. REMOVE EXIST FLAGPOLE & BASE, AND ASSOCIATED COMPONENTS, OPERATIONAL MECHANISMS, ATTACHMENTS, HARDWARE, AND LIGHTING

FLOORS

F1. REMOVE EXISTING TILE FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F2. REMOVE EXISTING CARPETED FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F3. REMOVE EXISTING VCT FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F4. SAW CUT EXISTING FLOOR SLAB AS REQ'D. FOR INSTALLATION OF NEW PLUMBING. REF PLUMBING DRAWINGS.

F5. SHOT BLAST EXISTING CONC SURFACE FOR PREPARATION OF NEW SCHEDULED FINISH

F6. SAW CUT & REMOVE EXISTING CONC RAMP

F7. SAW CUT & REMOVE PORTION OF EXISTING CONC. SLAB AS SHOWN - REF. STRUCTURAL DRAWINGS

F8. SAW CUT & REMOVE EXISTING FLOOR & CONC SLAB - REMOVE GRADE BEAMS & PIERS TO A DEPTH OF 3'-0" BELOW PROPOSED NEW GRADE AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

F9. CORE EXISTING TWO-WAY CONCRETE SLAB OVER CRAWLSPACE WITH THE MINIMUM DIAMETER CORE POSSIBLE, NOT TO EXCEED 8', WHERE NEEDED FOR NEW SANITARY SEWER PIPING ONLY AFTER USING A HILTI PS-200 FERROSCAN TO LOCATE EXISTING TOP AND BOTTOM BARS IN THE SLAB AND CENTERING THE CORE BETWEEN EXISTING BARS. IF IT IS CONSIDERED NECESSARY TO CUT THROUGH ANY EXISTING REINFORCEMENT, THE CONTRACTOR SHALL SUBMIT AN RFI ASKING FOR VERIFICATION THAT IT IS ACCEPTABLE TO CUT THE BAR.

WALLS

W1. REMOVE EXISTING GYP PARTITION, INCLUDING ANYTHING CONTAINED WITHIN THE PARTITION. CAP OFF / TERMINATE UTILITIES THEREIN

W2. REMOVE EXISTING MASONRY PARTITION, INCLUDING ANYTHING CONTAINED WITHIN THE PARTITION. CAP OFF OR TERMINATE UTILITIES THEREIN

W3. SAW CUT & REMOVE PORTION OF EXISTING MASONRY WALL INDICATED

W4. CUT IN NEW DOOR/WINDOW OPENING IN EXISTING GYP WALL, REF PLAN FOR TYPE

W5. SAW CUT EXISTING MASONRY WALL FOR NEW DOOR/WINDOW OPENING

W6. SAW CUT NEW RECTANGULAR HOLE THROUGH EXISTING WALL FOR HVAC DUCT, REF. MECH. FOR DUCT SIZE

W7. REMOVE EXISTING CERAMIC WALL TILE IN ENTIRE ROOM AND/OR SPACE & PREPARE EXISTING WALL FOR NEW TILE

W8. REMOVE PORTION OF EXISTING FACE BRICK FROM WALL AS INDICATED

W9. REMOVE PORTION OF EXISTING CONC. TILT-WALL PANEL. SHORE AND BRACE AND PREPARE FOR NEW CONSTRUCTION.

W10. REMOVE EXISTING EXTERIOR WALL, INCLUDING BASE, DOORS, WINDOWS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE WALL. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING WALLS AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW CONSTRUCTION AS SHOWN ON PLANS.

W11. REMOVE PORTION OF EXISTING MASONRY WALL TO 1 1/2" BELOW ADJACENT FINISH FLOOR LEVEL. HONE CUT SURFACE SMOOTH AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

W12. REMOVE PORTION OF EXISTING EXTERIOR METAL STUD WALL WITH MASONRY VENEER. SHORE AND BRACE AS REQUIRED AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

W13. REMOVE EXISTING INTERIOR WALL PANELING AND/OR EXISTING VINYL WALL COVERING AND SUBSTRATE AND PROVIDE NEW GYP. BOARD AT CORRIDOR SIDE OF WALL. PREPARE WALL TO RECEIVE NEW FINISH AS SHOWN ON FINISH PLANS.

W14. REMOVE EXTERIOR MASONRY VENEER AND SUBSTRATE FROM EXISTING METAL STUD WALL. PREPARE FOR NEW SUBSTRATE AND MASONRY AS SHOWN.

W15. REMOVE EXISTING PARTITION (SHOWN DASHED, TYPICAL), INCLUDING BASE, DOORS, WINDOWS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE PARTITION. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING WALLS AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PARTITIONS AND PREPARE TO RECEIVE NEW AS SHOWN ON PLANS.

W16. SHORE, BRACE, AND SAW CUT NEW OPENING INTO EXISTING WALL. PREPARE OPENING TO RECEIVE NEW DOOR, FRAME, AND HARDWARE AS SCHEDULED.

W17. SHORE, BRACE, AND SAW CUT NEW OPENING INTO EXISTING WALL. PREPARE OPENING TO RECEIVE NEW LINTEL AND FINISHES AS SPECIFIED.

W18. REMOVE EXISTING CERAMIC TILE AND BACKING LAYER OF GYP. BOARD. REPLACE GYP. BD. WITH NEW GYP. BD. (TILE BACKER BD. AT NEW TILE FINISH) TO CREATE LEVEL WALL WITH ANY ADJACENT EXISTING GYP. WALL BOARD AND PREPARE FOR NEW FINISHES AS SCHEDULED.

W19. PILASTER TO REMAIN.

CEILINGS

C1. REMOVE EXISTING CEILING GRID, TILE, GRILLES, DIFFUSERS, LIGHTING, EQUIPMENT, DEVICES, AND ALL OTHER ITEMS OCCURRING WITHIN THE CEILING PLANE. REMOVE ANY CEILING TRIM WHEREVER IT OCCURS. PREPARE ADJACENT WALLS TO RECEIVE NEW CEILING AS SHOWN ON REFLECTED CEILING PLANS. REF. ELEC.

C2. REMOVE EXISTING PLASTER CEILING SYSTEM AND ASSOCIATED TRIMS, FRAMING, HANGERS, AND HARDWARE, LIGHTING, AND ALL OTHER ITEMS OCCURRING WITHIN THE CEILING PLANE. PREPARE ADJACENT WALLS TO RECEIVE NEW CEILING AS SHOWN ON REFLECTED CEILING PLANS. REF. ELEC.

C3. REMOVE EXISTING ACOUSTICAL CEILING. REMOVE LIGHTING, EQUIPMENT, DEVICES, AND OTHER ITEMS ATTACHED TO CEILING.

C4. REMOVE EXISTING CELLULOSE FIREPROOFING AT DECK & STRUCTURAL FRAMING. POWER WASH REMOVAL IS ACCEPTABLE.

C5. REMOVE EXISTING WOOD PANEL CEILING CLOUDS. PROTECT & STORE FOR REINSTALLATION.

NOTES BY SYMBOL

CEILINGS CONT-

C6. REMOVE EXISTING CEILING SYSTEM IN ENTIRE ROOM AND/OR SPACE. INCLUDING, BUT NOT LIMITED TO, LIGHT FIXTURES, HVAC DIFFUSERS, CEILING-MOUNTED EQUIPMENT, AND CEILING-MOUNTED DEVICES.

C7. REMOVE EXISTING LAY-IN CEILING GRID IN ENTIRE ROOM AND/OR SPACE. RETAIN LIGHTS, HVAC ETC. SUPPORT IN PLACE FOR REINSTALLATION IN NEW CEILING.

C8. REMOVE EXISTING LIGHTS (PULL WIRE BACK TO ELECT PANEL).

C9. REMOVE EXISTING CEILING TILES IN ENTIRE ROOM AND/OR SPACE.

C10. REMOVE EXISTING FURRDOWNS/BULKHEADS ABOVE.

C11. REMOVE PORTION OF CEILING GRID AS REQ'D FOR INSTALLATION OF FIRE SPRINKLER SYSTEM / FIRE ALARM DEVICES.

C12. REMOVE PORTION OF GYP BD CEILING AS REQ'D FOR INSTALLATION OF FIRE SPRINKLER SYSTEM / FIRE ALARM DEVICES. PATCH & TEXTURE CEILING TO MATCH EXISTING.

C13. REMOVE PORTION OF EXISTING CEILING SYSTEM FOR INSTALLATION OF ROOF DRAINS, REF PLUMBING.

C14. REMOVE EXISTING PLASTER CEILING / SOFFIT AND ASSOCIATED TRIMS, FRAMING HANGERS, AND HARDWARE

DOORS & WINDOWS

D1. REMOVE EXISTING DOOR AND/OR WINDOW, FRAME, AND HARDWARE (SHOWN DASHED, TYPICAL). PATCH, REPAIR, AND CLEAN OPENING AND PREPARE TO INFILL OPENING AS SHOWN ON FLOOR PLANS.

D2. REMOVE EXISTING DOOR AND/OR WINDOW AND FRAME (SHOWN DASHED, TYPICAL). PATCH, REPAIR AND CLEAN OPENING AND PREPARE FOR NEW DOOR OR WINDOW AS SHOWN ON FLOOR PLANS.

D3. REMOVE EXISTING OVERHEAD DOOR, TRACKS, HARDWARE, OPERATIONAL MECHANISMS, AND ASSOCIATED SUPPORTS. PREPARE WALL FOR NEW FINISH.

D4. REMOVE EXISTING DOOR (FRAME TO REMAIN, PREP FOR PAINTING AND NEW HARDWARE).

D5. REMOVE EXISTING DOOR, FRAME, AND ASSOCIATED HARDWARE.

D6. REMOVE EXISTING DOOR, INFILL HINGE LOCATIONS TO A SMOOTH FINISH, AND PREP FOR PAINTING.

D7. REMOVE EXISTING OVERHEAD DOOR, TRACKS, HARDWARE, OPERATIONAL MECHANISMS, AND ASSOCIATED SUPPORTS.

D8. REMOVE EXISTING DOOR FRAME. SALVAGE DOOR & PREP DOOR FOR NEW HARDWARE.

D9. REMOVE EXISTING WINDOWS.

D10. REMOVE EXISTING ALUM STOREFRONT SYSTEM OR CURTAIN WALL SYSTEM, GLAZING, GASKETS, HARDWARE, AND ASSOCIATED BRACING.

D11. REMOVE EXISTING WINDOW, SAW CUT WALL BELOW FOR INSTALLATION OF NEW DOOR

MISCELLANEOUS

M1. REMOVE AND DISPOSE OF EXISTING CASEWORK AND ACCESSORIES.

M2. REMOVE EXISTING TOILET PARTITIONS AND ALL ASSOCIATED EQUIPMENT, ACCESSORIES, AND HARDWARE IN THEIR ENTIRETY AND DELIVER TO OWNER.

M3. REMOVE EXISTING LOCKERS AND DELIVER TO OWNER.

M4. REMOVE EXISTING COLUMN COVERS AND DELIVER TO OWNER.

M5. REMOVE EXISTING CONCRETE STAIRS OR RAMP AND ASSOCIATED HANDRAILS AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

M6. REMOVE EXISTING WIRE MESH PARTITIONS AND DELIVER TO OWNER.

M7. REMOVE EXISTING CHAIN LINK FENCE PARTITION

M8. REMOVE EXISTING HANDRAILS AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

M9. REMOVE EXISTING MASONRY PLANTERS

M10. REMOVE EXISTING MARKERBOARD OR TACKBOARD. RELOCATE TO NEW LOCATION, REF. PLANS

M11. REMOVE EXISTING MILLWORK, CABINETS AND OR BRACKETS

M12. REMOVE EXISTING ELECTRICAL SERVICE

M13. REMOVE EXISTING THEATRICAL RIGGING, SOUND, LIGHTING EQUIP & SPEAKERS LOCATED WITHIN STAGE AREA

M14. REMOVE EXISTING METAL LOCKERS

M15. REMOVE & RELOCATE EXISTING METAL LOCKERS, REF PLAN

M16. REMOVE EXISTING LOCKER BASE, LOCKERS & BENCHES

M17. REMOVE EXISTING FOLDING PANEL PARTITION

M18. REMOVE EXISTING TOILET ACCESSORIES. PATCH WALL AS REQ'D.

M19. REMOVE EXISTING HVAC UNIT. CAP OFF W/ NEW SHEET MTL & INSULATE OPENING.

M20. REMOVE EXISTING HVAC UNIT & CURB, INFILL OPENING W/ NEW METAL DECK & NEW RIGID INSUL TO MATCH EXISTING ROOF TO FLUSH W/ EXISTING. PROVIDE ADDITIONAL SUPPORT STRUCTURE PER STRUCTURAL.

M21. REMOVE EXISTING SOUND ISOLATION PRACTICE ROOMS; INCLUDING BASE, DOORS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE WALL PARTITION. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING WALLS AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW SOUND ISOLATION ROOMS AS SHOWN ON PLANS.

PLUMBING

P1. REMOVE EXISTING PLUMBING FIXTURES, BRACKETS, AND ACCESSORIES. TERMINATE OR RELOCATE UTILITIES AS REQUIRED. REF. PLUMBING.

P2. REMOVE EXISTING PLUMBING FIXTURES AND REPLACE WITH NEW FIXTURES IN THE SAME LOCATION. REF. PLUMBING.

P3. REMOVE EXISTING PLUMBING FIXTURE, TO BE REPLACED W/ NEW FIXTURE

ROOF

R1. REMOVE PORTION OF EXISTING ROOF, DECK, WALKWAY COVER, AND/ OR CANOPY AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

R2. REMOVE PORTION OF EXISTING ROOF COVER AND INSULATION. PREPARE TO RECEIVE NEW ROOF WITH NEW WORK.

R.3 REMOVE COPING, AND DAMAGED BLOCKING TO PREPARE FOR NEW WORK. PROTECT AND PRESERVE UNDAMAGED FLASHING, ROOFING MEMBRANE, AND BLOCKING. COORDINATE WITH NEW WORK.

STRUCTURAL

S1. REMOVE EXISTING CONCRETE COLUMN. REF. STRUCTURAL DRAWINGS FOR BRACING AND COLUMN REPLACEMENT IF APPLICABLE.

S2. SAW CUT EXISTING CONCRETE SLAB. REF. STRUCTURAL PLANS FOR EXTENTS AND DETAILS.

S3. REMOVE EXISTING STEEL COLUMN. REF. STRUCTURAL DRAWINGS FOR BRACING AND COLUMN REPLACEMENT IF APPLICABLE.

NOTES BY SYMBOL

A. BIDDERS SHALL VISIT SITE AND BE FAMILIAR WITH EXISTING CONDITIONS, INCLUDING BUT NOT LIMITED TO EXISTING DIMENSIONS, EQUIPMENT, LOCATIONS, SIZES, QUANTITIES, AND MATERIALS.

B. EXISTING ELECTRICAL POWER SERVING THE EXISTING FACILITY SHALL REMAIN ON LINE. DISRUPTIONS REQUIRED FOR CONSTRUCTION TO BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

C. EXISTING DRAIN LINES SERVING THE EXISTING FACILITY SHALL REMAIN FUNCTIONAL. ANY DISRUPTIONS REQUIRED FOR NEW TIE-INS DURING CONSTRUCTION MUST BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

D. CONTRACTOR SHALL COORDINATE EXACT SIZES AND LOCATIONS FOR MECHANICAL, PLUMBING, ELECTRICAL, AND TECHNOLOGY PENETRATIONS REQUIRED FOR NEW WORK WITH EACH RESPECTIVE TRADE.

E. CONTRACTOR SHALL KEEP OPENINGS TO THE EXTERIOR TEMPORARILY COVERED FOR PROTECTION FROM WATER.

F. CONTRACTOR SHALL KEEP OPENINGS TEMPORARILY COVERED FOR PEDESTRIAN SAFETY.

G. CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF TEMPORARY SHORING AND BRACING REQUIRED FOR DEMOLITION.

H. REFER TO MECHANICAL, ELECTRICAL, TECHNOLOGY, AND PLUMBING SHEETS FOR ADDITIONAL PENETRATIONS AND OTHER RELATED DEMOLITION OR EQUIPMENT REMOVAL.

I. EXISTING CONSTRUCTION IS SHOWN BASED ON OWNER FURNISHED PLANS, OWNER FURNISHED SURVEYS, AND ONSITE OBSERVATIONS. DISCREPANCIES BETWEEN DRAWINGS AND ACTUAL FIELD CONDITIONS WILL BE REPORTED TO THE ARCHITECT/ENGINEER/PROJECT MANAGER AND ANY ADDITIONAL DIRECTION, IF NECESSARY, SHALL BE OBTAINED FROM THE ARCHITECT PRIOR TO PROCEEDING WITH WORK.

J. EXISTING CONSTRUCTION ADJACENT TO DEMOLITION WORK WILL BE PATCHED AND REPAIRED TO MATCH ORIGINAL CONDITION.

K. DEMOLITION WORK TIMES SHALL BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

L. THIS EXISTING FACILITY SHALL REMAIN FUNCTIONAL DURING THE COURSE OF THE DEMOLITION WORK. CONTRACTOR WILL MAINTAIN DUST BARRIERS, BARRICADES, PEDESTRIAN PROTECTION, WATER PROTECTION, AND SAFETY DEVICES IN PLACE AT ALL TIMES DURING AND AFTER DEMOLITION UNTIL NEW WORK IS INSTALLED AND ACCEPTED BY OWNER.

M. CONTRACTOR SHALL MAINTAIN SITE DRAINAGE DEVICES AND COMPONENTS DURING THE COURSE OF DEMOLITION AND UP UNTIL NEW WORK IS IN PLACE AND ACCEPTED BY OWNER. THIS INCLUDES, BUT IS NOT LIMITED TO, EXISTING SUB-SURFACE DRAINAGE.

N. REFER TO ROOF PLAN AND MECHANICAL, ELECTRICAL, PLUMBING, TECHNOLOGY AND ANY OTHER DISCIPLINES' DRAWINGS FOR ADDITIONAL REQUIRED DEMOLITION NOT SPECIFICALLY COVERED BY ARCHITECTURAL DEMOLITION PLANS, ESPECIALLY WORK RELATED TO MODIFICATIONS TO EXISTING H.V.A.C. SYSTEMS.

O. DEMOLITION, AS NOTED, CONSISTS OF COMPLETE REMOVAL OF THE LISTED ITEMS, RELATED FASTENERS, AND ATTACHMENT MATERIALS LEAVING A CLEAN SURFACE READY TO RECEIVE NOTED MATERIALS OR SCHEDULED FINISHES.

P. UNLESS NOTED OTHERWISE, OR OWNER EXPRESSES A DESIRE TO RETAIN DEMOLISHED ITEMS BEFORE THEY ARE REMOVED FROM THE SITE, ALL DEMOLISHED MATERIALS NOT RETAINED BY OWNER SHALL BE REMOVED FROM THE SITE AND DISPOSED OF AT CONTRACTOR'S SOLE EXPENSE. DISPOSAL SHALL COMPLY WITH ALL CODES AND LAWS, INCLUDING LOCAL, STATE AND FEDERAL LAWS. DURING DEMOLITION, CONTRACTOR SHALL CAREFULLY REMOVE ANY ITEMS FROM THE EXISTING CONSTRUCTION THAT THE OWNER HAS EXPRESSED A DESIRE TO RETAIN AND, IF NECESSARY, TEMPORARILY PROTECT AND STORE THE ITEMS AT THE SITE. THE OWNER MAY REMOVE RETAINED ITEMS FROM THE SITE OR, AS A PART OF THIS CONTRACT AND AT NO ADDITIONAL COST, IF REQUESTED BY OWNER, THE CONTRACTOR SHALL DELIVER OWNER-RETAINED ITEMS TO A LOCAL LOCATION SELECTED BY THE OWNER AND OFFLOAD AND PLACE THE ITEMS FOR STORAGE AS DIRECTED BY THE OWNER.

Q. DEMOLITION SHALL FOLLOW THE CONSTRUCTION SCHEDULE PROVIDED IN THE SPECIFICATIONS.

R. CONTRACTOR SHALL TAKE ALL PRECAUTIONS TO SEPARATE STUDENTS, SCHOOL STAFF, AND VISITORS FROM DEMOLITION AND CONSTRUCTION EFFORTS, ACTIVITIES AND WORK, AS MAY BE NECESSARY TO PROTECT THEIR HEALTH AND SAFETY.

S. ADDITIONAL MATERIALS, WHERE CONSTRUCTION ADJOINS EXISTING, EVEN THOUGH IT MAY NOT BE SPECIFICALLY IDENTIFIED IN THE DEMOLITION PLANS, WILL BE REMOVED AS REQUIRED TO COMPLETE THE CONSTRUCTION. THESE MATERIALS TYPICALLY INCLUDE, BUT ARE NOT NECESSARILY LIMITED TO, MATERIALS PROJECTING BEYOND THE FACE OF THE WALL, SUCH AS BRICK SILLS AND ROOF FLASHINGS, GUTTERS, AND TRIMS. REFER TO WALL SECTIONS FOR SPECIFIC CONDITIONS.

T. INVESTIGATE EXISTING CONSTRUCTION SUBJECT TO DEMOLITION TO DETERMINE IF IT IS INTENDED AS STRUCTURE-BEARING. COORDINATE WITH CONSTRUCTION SEQUENCE AND PROVIDE SHORING AND ANY OTHER TEMPORARY SUPPORT FOR ANY FEATURES CARRYING STRUCTURAL LOAD TO PREVENT ANY CHANGE IN STRUCTURAL INTEGRITY UNTIL NEW OR REPLACEMENT STRUCTURAL SUPPORT IS IN PLACE.

U. REFER TO TECHNOLOGY SHEETS FOR DEMOLITION OF TECHNOLOGY AND TECHNOLOGY-RELATED ITEMS.

DEMOLITION GENERAL NOTES

EXISTING PARTITION TO BE REMOVED. INCLUDING BASE, DOORS, WINDOWS, OUTLETS, DEVICES, SWITCHES, CHALKTACK BOARDS, POWER POLES, WALL CAPS AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE PARTITION. CONTRACTOR WILL VERIFY ELECTRICAL, MECHANICAL, & PLUMBING IN EXISTING WALLS & TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR & CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW PARTITION WALLS AS SHOWN ON FLOOR PLANS.

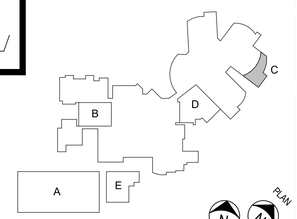
EXISTING DOOR, FRAME, AND HARDWARE TO BE REMOVED UNLESS OTHERWISE NOTED.

EXISTING ITEM TO BE REMOVED AS NOTED.

EXISTING WALL / PARTITION TO REMAIN.

EXISTING DOOR, FRAME, AND HARDWARE TO REMAIN.

DEMOLITION LEGEND



Revision /	Date
1	05/08/25 ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

Huckabee
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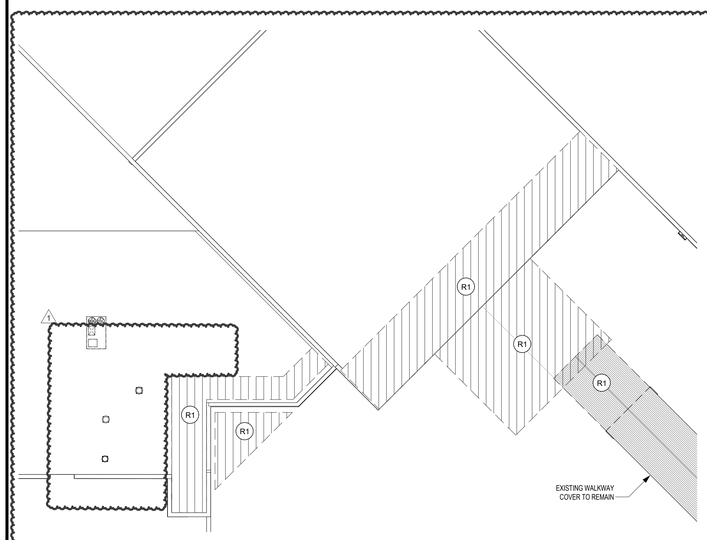
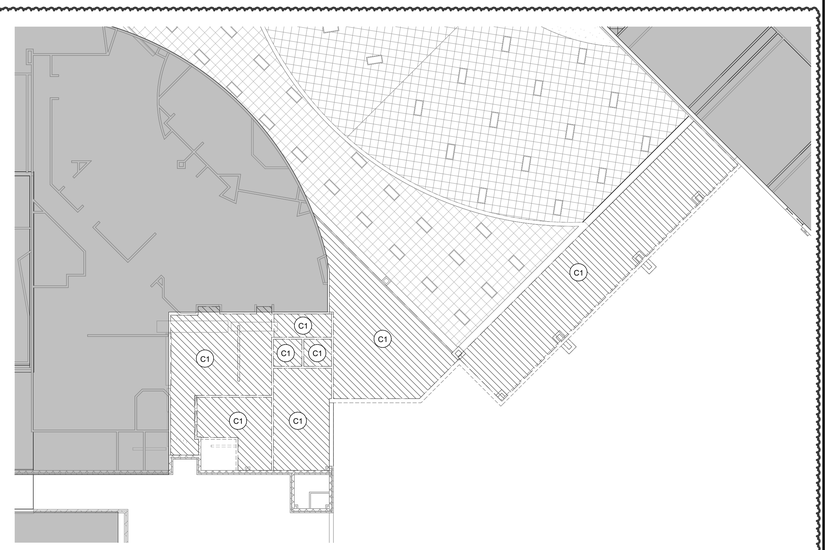
DEMOLITION FLOOR PLAN -AREA C

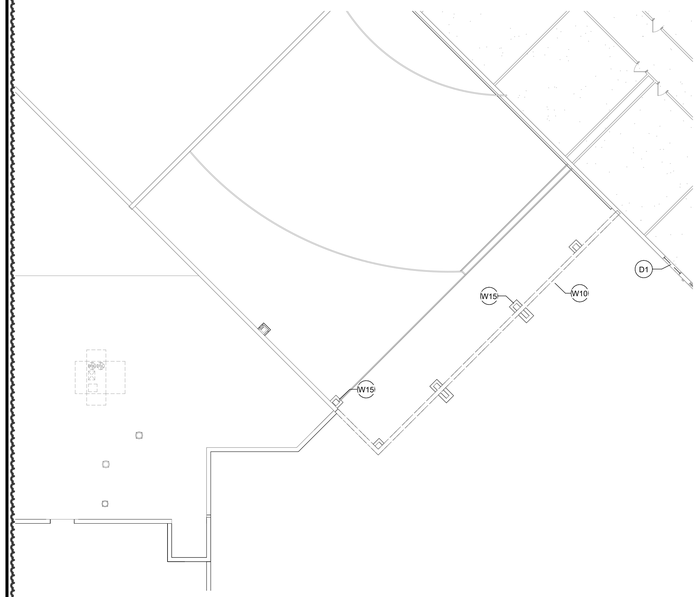
PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. AD1.02
Drawn By: YB/AB	ISSUE FOR BID
Date 05/08/2025	



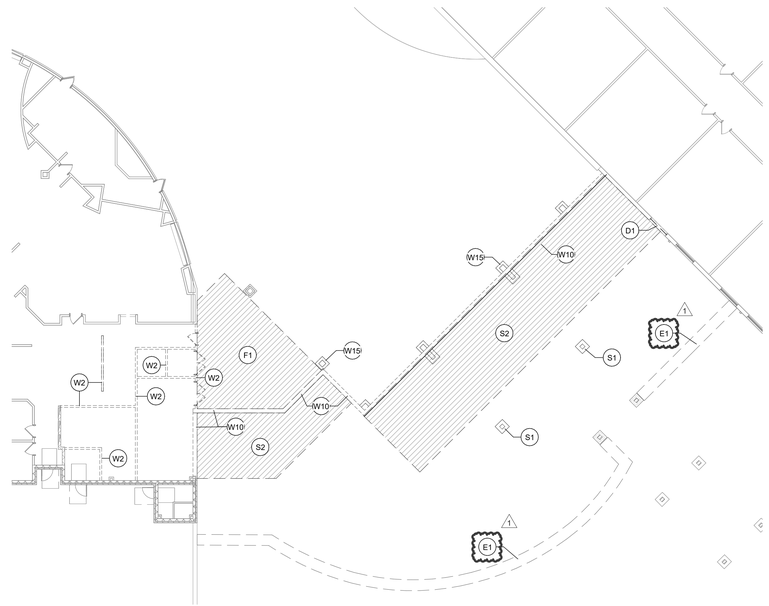
4 DEMOLITION PLAN, AREA D, ROOF
1/16" = 1'-0"



3 DEMOLITION REFLECTED CEILING PLAN, AREA D
1/16" = 1'-0"



2 DEMOLITION PLAN, AREA D, LEVEL 2
1/16" = 1'-0"



1 DEMOLITION PLAN, AREA D, LEVEL 1
1/16" = 1'-0"

EXTERIOR

E1. SAW CUT & REMOVE EXISTING CONC SIDEWALK, CURB OR FLAT WORK

E2. SAW CUT & REMOVE EXIST CONC LANDING / STEPS & RAILING & RETAINING WALL

E3. REMOVE EXISTING GUTTERS, DOWNSPOUTS & SPLASH BLOCKS

E4. REMOVE EXIST BRICK/CONC SCREEN WALL & FOUNDATION

E5. REMOVE EXISTING CANOPY SYSTEM, CONC. FOUNDATION, AND ASSOCIATED COMPONENTS, TRIMS, ATTACHMENTS, AND HARDWARE

E6. REMOVE EXIST FLAGPOLE & BASE, AND ASSOCIATED COMPONENTS, OPERATIONAL MECHANISMS, ATTACHMENTS, HARDWARE, AND LIGHTING

FLOORS

F1. REMOVE EXISTING TILE FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F2. REMOVE EXISTING CARPETED FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F3. REMOVE EXISTING VCT FLOORING & BASE DOWN TO EXISTING SUBSTRATE. PATCH AND/OR REPAIR EXISTING SUBSTRATE. APPLY LEVELING COMPOUND AS REQ'D. FOR NEW SCHEDULED FINISH.

F4. SAW CUT EXISTING FLOOR SLAB AS REQ'D. FOR INSTALLATION OF NEW PLUMBING. REF PLUMBING DRAWINGS.

F5. SHOT BLAST EXISTING CONC SURFACE FOR PREPARATION OF NEW SCHEDULED FINISH

F6. SAW CUT & REMOVE EXISTING CONC RAMP

F7. SAW CUT & REMOVE PORTION OF EXISTING CONC. SLAB AS SHOWN - REF. STRUCTURAL DRAWINGS

F8. SAW CUT & REMOVE EXISTING FLOOR & CONC SLAB - REMOVE GRADE BEAMS & PIERS TO A DEPTH OF 3'-0" BELOW PROPOSED NEW GRADE AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

F9. CORE EXISTING TWO-WAY CONCRETE SLAB OVER CRAWLSPACE WITH THE MINIMUM DIAMETER CORE POSSIBLE, NOT TO EXCEED 8", WHERE NEEDED FOR NEW SANITARY SEWER PIPING ONLY AFTER USING A HILTI PS-200 FERROSCAN TO LOCATE EXISTING TOP AND BOTTOM BARS IN THE SLAB AND CENTERING THE CORE BETWEEN EXISTING BARS. IF IT IS CONSIDERED NECESSARY TO CUT THROUGH ANY EXISTING REINFORCEMENT, THE CONTRACTOR SHALL SUBMIT AN RFI ASKING FOR VERIFICATION THAT IT IS ACCEPTABLE TO CUT THE BAR.

WALLS

W1. REMOVE EXISTING GYP PARTITION, INCLUDING ANYTHING CONTAINED WITHIN THE PARTITION. CAP OFF / TERMINATE UTILITIES THEREIN

W2. REMOVE EXISTING MASONRY PARTITION, INCLUDING ANYTHING CONTAINED WITHIN THE PARTITION. CAP OFF OR TERMINATE UTILITIES THEREIN

W3. SAW CUT & REMOVE PORTION OF EXISTING MASONRY WALL INDICATED

W4. CUT IN NEW DOORWINDOW OPENING IN EXISTING GYP WALL, REF PLAN FOR TYPE

W5. SAW CUT EXISTING MASONRY WALL FOR NEW DOORWINDOW OPENING

W6. SAW CUT NEW RECTANGULAR HOLE THROUGH EXISTING WALL FOR HVAC DUCT, REF. MECH. FOR DUCT SIZE

W7. REMOVE EXISTING CERAMIC WALL TILE IN ENTIRE ROOM AND/OR SPACE & PREPARE EXISTING WALL FOR NEW TILE

W8. REMOVE PORTION OF EXISTING FACE BRICK FROM WALL AS INDICATED

W9. REMOVE PORTION OF EXISTING CONC. TILT-WALL PANEL, SHORE AND BRACE AND PREPARE FOR NEW CONSTRUCTION.

W10. REMOVE EXISTING EXTERIOR WALL, INCLUDING BASE, DOORS, WINDOWS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE WALL. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING WALLS AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW CONSTRUCTION AS SHOWN ON PLANS.

W11. REMOVE PORTION OF EXISTING MASONRY WALL TO 1 1/2" BELOW ADJACENT FINISH FLOOR LEVEL. HONE CUT SURFACE SMOOTH AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

W12. REMOVE PORTION OF EXISTING EXTERIOR METAL STUD WALL WITH MASONRY VENEER. SHORE AND BRACE AS REQUIRED AND PREPARE FOR NEW CONSTRUCTION AS SHOWN.

W13. REMOVE EXISTING INTERIOR WALL PANELING AND/OR EXISTING VINYL WALL COVERING AND SUBSTRATE AND PROVIDE NEW GYP. BOARD AT CORRIDOR SIDE OF WALL. PREPARE WALL TO RECEIVE NEW FINISH AS SHOWN ON FINISH PLANS.

W14. REMOVE EXTERIOR MASONRY VENEER AND SUBSTRATE FROM EXISTING METAL STUD WALL. PREPARE FOR NEW SUBSTRATE AND MASONRY AS SHOWN.

W15. REMOVE EXISTING PARTITION (SHOWN DASHED, TYPICAL), INCLUDING BASE, DOORS, WINDOWS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE PARTITION. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PARTITIONS AND PREPARE TO RECEIVE NEW AS SHOWN ON PLANS.

W16. SHORE, BRACE, AND SAW CUT NEW OPENING INTO EXISTING WALL. PREPARE OPENING TO RECEIVE NEW DOOR, FRAME, AND HARDWARE AS SCHEDULED.

W17. SHORE, BRACE, AND SAW CUT NEW OPENING INTO EXISTING WALL. PREPARE OPENING TO RECEIVE NEW LINTEL AND FINISHES AS SPECIFIED.

W18. REMOVE EXISTING CERAMIC TILE AND BACKING LAYER OF GYP. BOARD, REPLACE GYP. BD. WITH NEW GYP. BD. (TILE BACKER BD. AT NEW TILE FINISH) TO CREATE LEVEL WALL WITH ANY ADJACENT EXISTING GYP. WALL BOARD AND PREPARE FOR NEW FINISHES AS SCHEDULED.

W19. PILASTER TO REMAIN.

CEILINGS

C1. REMOVE EXISTING CEILING GRID, TILE, GRILLES, DIFFUSERS, LIGHTING, EQUIPMENT, DEVICES, AND ALL OTHER ITEMS OCCURRING WITHIN THE CEILING PLANE. REMOVE ANY CEILING TRIM WHEREVER IT OCCURS. PREPARE ADJACENT WALLS TO RECEIVE NEW CEILING AS SHOWN ON REFLECTED CEILING PLANS. REF. ELEC.

C2. REMOVE EXISTING PLASTER CEILING SYSTEM AND ASSOCIATED TRIMS, FRAMING, HANGERS, AND HARDWARE, LIGHTING, AND ALL OTHER ITEMS OCCURRING WITHIN THE CEILING PLANE. PREPARE ADJACENT WALLS TO RECEIVE NEW CEILING AS SHOWN ON REFLECTED CEILING PLANS. REF. ELEC.

C3. REMOVE EXISTING ACOUSTICAL CEILING. REMOVE LIGHTING, EQUIPMENT, DEVICES, AND OTHER ITEMS ATTACHED TO CEILING.

C4. REMOVE EXISTING CELLULOSE FIREPROOFING AT DECK & STRUCTURAL FRAMING. POWER WASH REMOVAL IS ACCEPTABLE.

C5. REMOVE EXISTING WOOD PANEL CEILING CLOUDS. PROTECT & STORE FOR REINSTALLATION.

NOTES BY SYMBOL

CEILINGS CONT-

C6. REMOVE EXISTING CEILING SYSTEM IN ENTIRE ROOM AND/OR SPACE, INCLUDING, BUT NOT LIMITED TO, LIGHT FIXTURES, HVAC DIFFUSERS, CEILING-MOUNTED EQUIPMENT, AND CEILING-MOUNTED DEVICES.

C7. REMOVE EXISTING LAY-IN CEILING GRID IN ENTIRE ROOM AND/OR SPACE. RETAIN LIGHTS, HVAC ETC. SUPPORT IN PLACE FOR REINSTALLATION IN NEW CEILING.

C8. REMOVE EXISTING LIGHTS (PULL WIRE BACK TO ELECT PANEL).

C9. REMOVE EXISTING CEILING TILES IN ENTIRE ROOM AND/OR SPACE.

C10. REMOVE EXISTING FURRDOWNS/BULKHEADS ABOVE.

C11. REMOVE PORTION OF CEILING GRID AS REQ'D FOR INSTALLATION OF FIRE SPRINKLER SYSTEM / FIRE ALARM DEVICES.

C12. REMOVE PORTION OF GYP BD CEILING AS REQ'D FOR INSTALLATION OF FIRE SPRINKLER SYSTEM / FIRE ALARM DEVICES. PATCH & TEXTURE CEILING TO MATCH EXISTING.

C13. REMOVE PORTION OF EXISTING CEILING SYSTEM FOR INSTALLATION OF ROOF DRAINS, REF PLUMBING.

C14. REMOVE EXISTING PLASTER CEILING / SOFFIT AND ASSOCIATED TRIMS, FRAMING HANGERS, AND HARDWARE

DOORS & WINDOWS

D1. REMOVE EXISTING DOOR AND/OR WINDOW, FRAME, AND HARDWARE (SHOWN DASHED, TYPICAL). PATCH, REPAIR, AND CLEAN OPENING AND PREPARE TO INFILL OPENING AS SHOWN ON FLOOR PLANS.

D2. REMOVE EXISTING DOOR AND/OR WINDOW AND FRAME (SHOWN DASHED, TYPICAL). PATCH, REPAIR AND CLEAN OPENING AND PREPARE FOR NEW DOOR OR WINDOW AS SHOWN ON FLOOR PLANS.

D3. REMOVE EXISTING OVERHEAD DOOR, TRACKS, HARDWARE, OPERATIONAL MECHANISMS, AND ASSOCIATED SUPPORTS. PREPARE WALL FOR NEW FINISH.

D4. REMOVE EXISTING DOOR (FRAME TO REMAIN, PREP FOR PAINTING AND NEW HARDWARE).

D5. REMOVE EXISTING DOOR, FRAME, AND ASSOCIATED HARDWARE.

D6. REMOVE EXISTING DOOR, INFILL HINGE LOCATIONS TO A SMOOTH FINISH, AND PREP FOR PAINTING.

D7. REMOVE EXISTING OVERHEAD DOOR, TRACKS, HARDWARE, OPERATIONAL MECHANISMS, AND ASSOCIATED SUPPORTS.

D8. REMOVE EXISTING DOOR FRAME. SALVAGE DOOR & PREP DOOR FOR NEW HARDWARE.

D9. REMOVE EXISTING WINDOWS.

D10. REMOVE EXISTING ALUM STOREFRONT SYSTEM OR CURTAIN WALL SYSTEM, GLAZING, GASKETS, HARDWARE, AND ASSOCIATED BRACING.

D11. REMOVE EXISTING WINDOW, SAW CUT WALL BELOW FOR INSTALLATION OF NEW DOOR

MISCELLANEOUS

M1. REMOVE AND DISPOSE OF EXISTING CASEWORK AND ACCESSORIES.

M2. REMOVE EXISTING TOILET PARTITIONS AND ALL ASSOCIATED EQUIPMENT, ACCESSORIES, AND HARDWARE IN THEIR ENTIRETY AND DELIVER TO OWNER.

M3. REMOVE EXISTING LOCKERS AND DELIVER TO OWNER.

M4. REMOVE EXISTING COLUMN COVERS AND DELIVER TO OWNER.

M5. REMOVE EXISTING CONCRETE STAIRS OR RAMP AND ASSOCIATED HANDRAILS AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

M6. REMOVE EXISTING WIRE MESH PARTITIONS AND DELIVER TO OWNER.

M7. REMOVE EXISTING CHAIN LINK FENCE PARTITION

M8. REMOVE EXISTING HANDRAILS AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

M9. REMOVE EXISTING MASONRY PLANTERS

M10. REMOVE EXISTING MARKERBOARD OR TACKBOARD. RELOCATE TO NEW LOCATION, REF. PLANS

M11. REMOVE EXISTING MILLWORK, CABINETS AND OR BRACKETS

M12. REMOVE EXISTING ELECTRICAL SERVICE

M13. REMOVE EXISTING THEATRICAL RIGGING, SOUND, LIGHTING EQUIP & SPEAKERS LOCATED WITHIN STAGE AREA

M14. REMOVE EXISTING METAL LOCKERS

M15. REMOVE & RELOCATE EXISTING METAL LOCKERS, REF PLAN

M16. REMOVE EXISTING LOCKER BASE, LOCKERS & BENCHES

M17. REMOVE EXISTING FOLDING PANEL PARTITION

M18. REMOVE EXISTING TOILET ACCESSORIES. PATCH WALL AS REQ'D.

M19. REMOVE EXISTING HVAC UNIT. CAP OFF W/ NEW SHEET MTL & INSULATE OPENING.

M20. REMOVE EXISTING HVAC UNIT & CURB, INFILL OPENING W/ NEW METAL DECK & NEW RIGID INSUL TO MATCH EXISTING ROOF TO FLUSH W/ EXISTING. PROVIDE ADDITIONAL SUPPORT STRUCTURE PER STRUCTURAL.

M21. REMOVE EXISTING SOUND ISOLATION PRACTICE ROOMS, INCLUDING BASE, DOORS, OUTLETS, EQUIPMENT, DEVICES, SWITCHES, AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE WALL PARTITION. CONTRACTOR SHALL VERIFY ELECTRICAL, MECHANICAL, AND PLUMBING IN EXISTING WALLS AND TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR, AND CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW SOUND ISOLATION ROOMS AS SHOWN ON PLANS.

PLUMBING

P1. REMOVE EXISTING PLUMBING FIXTURES, BRACKETS, AND ACCESSORIES. TERMINATE OR RELOCATE UTILITIES AS REQUIRED. REF. PLUMBING.

P2. REMOVE EXISTING PLUMBING FIXTURES AND REPLACE WITH NEW FIXTURES IN THE SAME LOCATION. REF. PLUMBING.

P3. REMOVE EXISTING PLUMBING FIXTURE, TO BE REPLACED W/ NEW FIXTURE

ROOF

R1. REMOVE PORTION OF EXISTING ROOF, DECK, WALKWAY COVER, AND/ OR CANOPY AND PREPARE ADJACENT AREAS FOR NEW CONSTRUCTION AS SHOWN.

R2. REMOVE PORTION OF EXISTING ROOF COVER AND INSULATION. PREPARE TO RECEIVE NEW ROOF WITH NEW WORK.

R.3 REMOVE COPING, AND DAMAGED BLOCKING TO PREPARE FOR NEW WORK. PROTECT AND PRESERVE UNDAMAGED FLASHING, ROOFING MEMBRANE, AND BLOCKING. COORDINATE WITH NEW WORK.

STRUCTURAL

S1. REMOVE EXISTING CONCRETE COLUMN. REF. STRUCTURAL DRAWINGS FOR BRACING AND COLUMN REPLACEMENT IF APPLICABLE.

S2. SAW CUT EXISTING CONCRETE SLAB. REF. STRUCTURAL PLANS FOR EXTENTS AND DETAILS.

S3. REMOVE EXISTING STEEL COLUMN. REF. STRUCTURAL DRAWINGS FOR BRACING AND COLUMN REPLACEMENT IF APPLICABLE.

NOTES BY SYMBOL

A. BIDDERS SHALL VISIT SITE AND BE FAMILIAR WITH EXISTING CONDITIONS, INCLUDING BUT NOT LIMITED TO EXISTING DIMENSIONS, EQUIPMENT, LOCATIONS, SIZES, QUANTITIES, AND MATERIALS.

B. EXISTING ELECTRICAL POWER SERVING THE EXISTING FACILITY SHALL REMAIN ON LINE. DISRUPTIONS REQUIRED FOR CONSTRUCTION TO BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

C. EXISTING DRAIN LINES SERVING THE EXISTING FACILITY SHALL REMAIN FUNCTIONAL. ANY DISRUPTIONS REQUIRED FOR NEW TIE-INS DURING CONSTRUCTION MUST BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

D. CONTRACTOR SHALL COORDINATE EXACT SIZES AND LOCATIONS FOR MECHANICAL, PLUMBING, ELECTRICAL, AND TECHNOLOGY PENETRATIONS REQUIRED FOR NEW WORK WITH EACH RESPECTIVE TRADE.

E. CONTRACTOR SHALL KEEP OPENINGS TO THE EXTERIOR TEMPORARILY COVERED FOR PROTECTION FROM WATER.

F. CONTRACTOR SHALL KEEP OPENINGS TEMPORARILY COVERED FOR PEDESTRIAN SAFETY.

G. CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF TEMPORARY SHORING AND BRACING REQUIRED FOR DEMOLITION.

H. REFER TO MECHANICAL, ELECTRICAL, TECHNOLOGY, AND PLUMBING SHEETS FOR ADDITIONAL PENETRATIONS AND OTHER RELATED DEMOLITION OR EQUIPMENT REMOVAL.

I. EXISTING CONSTRUCTION IS SHOWN BASED ON OWNER FURNISHED PLANS, OWNER FURNISHED SURVEYS, AND ONSITE OBSERVATIONS. DISCREPANCIES BETWEEN DRAWINGS AND ACTUAL FIELD CONDITIONS WILL BE REPORTED TO THE ARCHITECT/ENGINEER/PROJECT MANAGER AND ANY ADDITIONAL DIRECTION, IF NECESSARY, SHALL BE OBTAINED FROM THE ARCHITECT PRIOR TO PROCEEDING WITH WORK.

J. EXISTING CONSTRUCTION ADJACENT TO DEMOLITION WORK WILL BE PATCHED AND REPAIRED TO MATCH ORIGINAL CONDITION.

K. DEMOLITION WORK TIMES SHALL BE COORDINATED WITH THE OWNER REPRESENTATIVE ASSIGNED TO THIS SPECIFIC PROJECT.

L. THIS EXISTING FACILITY SHALL REMAIN FUNCTIONAL DURING THE COURSE OF THE DEMOLITION WORK. CONTRACTOR WILL MAINTAIN DUST BARRIERS, BARRICADES, PEDESTRIAN PROTECTION, WATER PROTECTION, AND SAFETY DEVICES IN PLACE AT ALL TIMES DURING AND AFTER DEMOLITION UNTIL NEW WORK IS INSTALLED AND ACCEPTED BY OWNER.

M. CONTRACTOR SHALL MAINTAIN SITE DRAINAGE DEVICES AND COMPONENTS DURING THE COURSE OF DEMOLITION AND UP UNTIL NEW WORK IS IN PLACE AND ACCEPTED BY OWNER. THIS INCLUDES, BUT IS NOT LIMITED TO, EXISTING SUB-SURFACE DRAINAGE.

N. REFER TO ROOF PLAN AND MECHANICAL, ELECTRICAL, PLUMBING, TECHNOLOGY AND ANY OTHER DISCIPLINES' DRAWINGS FOR ADDITIONAL REQUIRED DEMOLITION NOT SPECIFICALLY COVERED BY ARCHITECTURAL DEMOLITION PLANS, ESPECIALLY WORK RELATED TO MODIFICATIONS TO EXISTING H.V.A.C. SYSTEMS.

O. DEMOLITION, AS NOTED, CONSISTS OF COMPLETE REMOVAL OF THE LISTED ITEMS, RELATED FASTENERS, AND ATTACHMENT MATERIALS LEAVING A CLEAN SURFACE READY TO RECEIVE NOTED MATERIALS OR SCHEDULED FINISHES.

P. UNLESS NOTED OTHERWISE, OR OWNER EXPRESSES A DESIRE TO RETAIN DEMOLISHED ITEMS BEFORE THEY ARE REMOVED FROM THE SITE, ALL DEMOLISHED MATERIALS NOT RETAINED BY OWNER SHALL BE REMOVED FROM THE SITE AND DISPOSED OF AT CONTRACTOR'S SOLE EXPENSE. DISPOSAL SHALL COMPLY WITH ALL CODES AND LAWS, INCLUDING LOCAL, STATE AND FEDERAL LAWS. DURING DEMOLITION, CONTRACTOR SHALL CAREFULLY REMOVE ANY ITEMS FROM THE EXISTING CONSTRUCTION THAT THE OWNER HAS EXPRESSED A DESIRE TO RETAIN AND, IF NECESSARY, TEMPORARILY PROTECT AND STORE THE ITEMS AT THE SITE. THE OWNER MAY REMOVE RETAINED ITEMS FROM THE SITE OR, AS A PART OF THIS CONTRACT AND AT NO ADDITIONAL COST, IF REQUESTED BY OWNER, THE CONTRACTOR SHALL DELIVER OWNER-RETAINED ITEMS TO A LOCAL LOCATION SELECTED BY THE OWNER AND OFFLOAD AND PLACE THE ITEMS FOR STORAGE AS DIRECTED BY THE OWNER.

Q. DEMOLITION SHALL FOLLOW THE CONSTRUCTION SCHEDULE PROVIDED IN THE SPECIFICATIONS.

R. CONTRACTOR SHALL TAKE ALL PRECAUTIONS TO SEPARATE STUDENTS, SCHOOL STAFF, AND VISITORS FROM DEMOLITION AND CONSTRUCTION EFFORTS, ACTIVITIES AND WORK, AS MAY BE NECESSARY TO PROTECT THEIR HEALTH AND SAFETY.

S. ADDITIONAL MATERIALS, WHERE CONSTRUCTION ADJOINS EXISTING, EVEN THOUGH IT MAY NOT BE SPECIFICALLY IDENTIFIED IN THE DEMOLITION PLANS, WILL BE REMOVED AS REQUIRED TO COMPLETE THE CONSTRUCTION. THESE MATERIALS TYPICALLY INCLUDE, BUT ARE NOT NECESSARILY LIMITED TO, MATERIALS PROJECTING BEYOND THE FACE OF THE WALL, SUCH AS BRICK SILLS AND ROOF FLASHINGS, GUTTERS, AND TRIMS. REFER TO WALL SECTIONS FOR SPECIFIC CONDITIONS.

T. INVESTIGATE EXISTING CONSTRUCTION SUBJECT TO DEMOLITION TO DETERMINE IF IT IS INTENDED AS STRUCTURE-BEARING. COORDINATE WITH CONSTRUCTION SEQUENCE AND PROVIDE SHORING AND ANY OTHER TEMPORARY SUPPORT FOR ANY FEATURES CARRYING STRUCTURAL LOAD TO PREVENT ANY CHANGE IN STRUCTURAL INTEGRITY UNTIL NEW OR REPLACEMENT STRUCTURAL SUPPORT IS IN PLACE.

U. REFER TO TECHNOLOGY SHEETS FOR DEMOLITION OF TECHNOLOGY AND TECHNOLOGY-RELATED ITEMS.

DEMOLITION GENERAL NOTES

EXISTING PARTITION TO BE REMOVED. INCLUDING BASE, DOORS, WINDOWS, OUTLETS, DEVICES, SWITCHES, CHALKTACK BOARDS, POWER POLES, WALL CAPS AND ANY ITEM ATTACHED TO OR ASSOCIATED WITH THE PARTITION. CONTRACTOR WILL VERIFY ELECTRICAL, MECHANICAL & PLUMBING IN EXISTING WALLS & TERMINATE OR RELOCATE AS REQUIRED FOR CONTRACT WORK. PATCH, REPAIR & CLEAN ALL ADJACENT WALLS AND PREPARE TO RECEIVE NEW PARTITION WALLS AS SHOWN ON FLOOR PLANS.

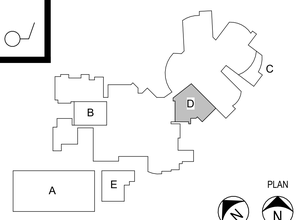
EXISTING DOOR, FRAME, AND HARDWARE TO BE REMOVED UNLESS OTHERWISE NOTED.

EXISTING ITEM TO BE REMOVED AS NOTED.

EXISTING WALL / PARTITION TO REMAIN.

EXISTING DOOR, FRAME, AND HARDWARE TO REMAIN.

DEMOLITION LEGEND



KEY PLAN
NTS

PLAN
TRUE
N

Revision / 1 — Date 05/08/25 — ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT
JAMES JASON HARO
29411
STATE OF TEXAS
05.08.25

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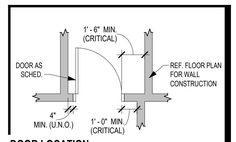
DEMOLITION FLOOR PLAN - AREA D

PACKAGE | VOLUME

Job No. 01954-08-01
Drawn By: Author
Date: 05/08/2025

Sheet No. AD1.03
ISSUE FOR BID



DOOR LOCATION

NOTES:

1. THE CONTRACT DOCUMENTS ARE COMPRISED OF BOTH THE DRAWINGS AND THE SPECIFICATIONS.
2. ALL ARCHITECTURAL FLOOR PLAN DIMENSIONS SHOWN ARE TO FACE OF C.F.M.F. MASONRY OR CONCRETE, U.N.O.
3. SEE STRUCTURAL DRAWINGS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS.
4. SEE SPECIFICATIONS AND G3 SHEETS FOR ADDITIONAL MASONRY AND CFMF PARTITION REQUIREMENTS.
5. SEE PLUMBING PLANS AND ENLARGED FLOOR PLANS FOR LOCATIONS OF FIXTURES.
6. REF. DOOR SCHEDULES AND DETAILS FOR MASONRY ROUGH OPENING SIZES.
7. VERIFY ALL MASONRY OPENINGS WITH MANUFACTURERS' PRODUCTS (E.G. FIRE EXTINGUISHER CABINETS, ELECTRICAL PANELS, ETC.).
8. PROVIDE WD. BLOCKING IN ALL GYP. BD. WALLS AS REQ'D. AT LOCATIONS INCLUDING, BUT NOT LIMITED TO; HANDRAILS, TOILET ACCESSORIES, PROJECTORS, CASEWORK, TACK BD.'S, MARKER BD.'S, WALL STOPS & DOOR HOLDERS.
9. PROVIDE SPRAYED FIRE RESISTANT MATERIAL AT ALL CONCEALED COLUMNS AND OTHER PRIMARY STRUCTURE, AND INSUMESCENT COATING AT ALL EXPOSED (INT + EXT) COLUMNS AS REQUIRED - REF G2 CODE PLANS.

GENERAL FLOOR PLAN NOTES

- **F.E.** — **FIRE EXTINGUISHER (WALL MOUNTED)** - REF. SPEC. SECT. 10 4400. PROVIDE TYPE "K" @ KITCHEN AREAS
- **F.E.C.** — **FIRE EXTINGUISHER CABINET** - REF. SPEC. SECT. 10 4400. PROVIDE TYPE "K" @ KITCHEN AREAS
- **P.S.** — **PROJECTION SCREEN** REF. TYP. DET. ON A4 SHEETS
- **X' M.B.** — **MARKERBOARD** REF. TYP. DET. ON A4 SHEETS
- **X' T.B.** — **TACKBOARD** REF. TYP. DET. ON A4 SHEETS
- **ELECTRIC WATER COOLER** - REF PLUMBING PLAN.
- 1.01 — **DOOR ASSEMBLY NUMBER** - REF. SCHEDULE ON A3 SHTS.
- SF-A — **WINDOW/STOREFRONT/CURTAIN WALL ASSEMBLY TYPE** - REF. A3 SHTS.
- SO1.01 — **SPECIAL OPENING NUMBER (ROLLING DOOR)** - REF. SCHEDULE ON A3 SHTS.
- 1 / A1.1 — **CALLOUT REFERENCE**
- 1 / A101 SIM — **WALL SECTION REFERENCE** XX / A101
- XXX' - X" FIN FLOOR — **ELEVATION REFERENCE**
- 1-01 ROOM — **ROOM NUMBER & NAME**
- **NOT IN CONTRACT** APPLIES TO AREA PLANS

FLOOR PLAN LEGEND

REF. TYP. CONST. DTLS. FOR ADDITIONAL INFO.

- **MASONRY VENEER**
- **STANDARD CMU**
- **CFMF PARTITION**
- **EXISTING CFMF PARTITION**

PARTITION TYPE TAGS

A3 / D — WALL ASSEMBLY / PLAN VIEW

LETTERS TO THE RIGHT INDICATE PARTITION HEIGHT, FIRE RATING AND ACOUSTICAL PROPERTIES.

D	WALL TO DECK / STRUCTURE
C	WALL TO 6" (MIN.) ABOVE CEILING
FR	FIRE RATED WALL TO DECK
AD	ACOUSTICAL WALL TO DECK
AC	ACOUSTICAL WALL TO 6" (MIN.) ABOVE CEILING
AR	ACOUSTICAL RATED WALL TO DECK
GD	GYP WALL ABOVE TO DECK
SR	SMOKE RESISTANT PARTITION

LETTER & NUMBER TO THE LEFT INDICATE PARTITION TYPE & STUD OR CMU SIZE AS FOLLOWS:

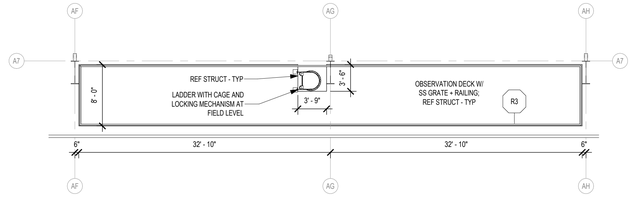
- 1 - 7/8" HAT CHANNEL
- 2 - 1 5/8" CFMF
- 3 - 2 1/2" CFMF
- 4 - 3 5/8" CFMF OR CMU
- 6 - 6" CFMF OR 5 5/8" CMU
- 8 - 8" CFMF OR 7 5/8" CMU
- 10 - 9 5/8" CMU
- 12 - 11 5/8" CMU

PARTITION NOTES:

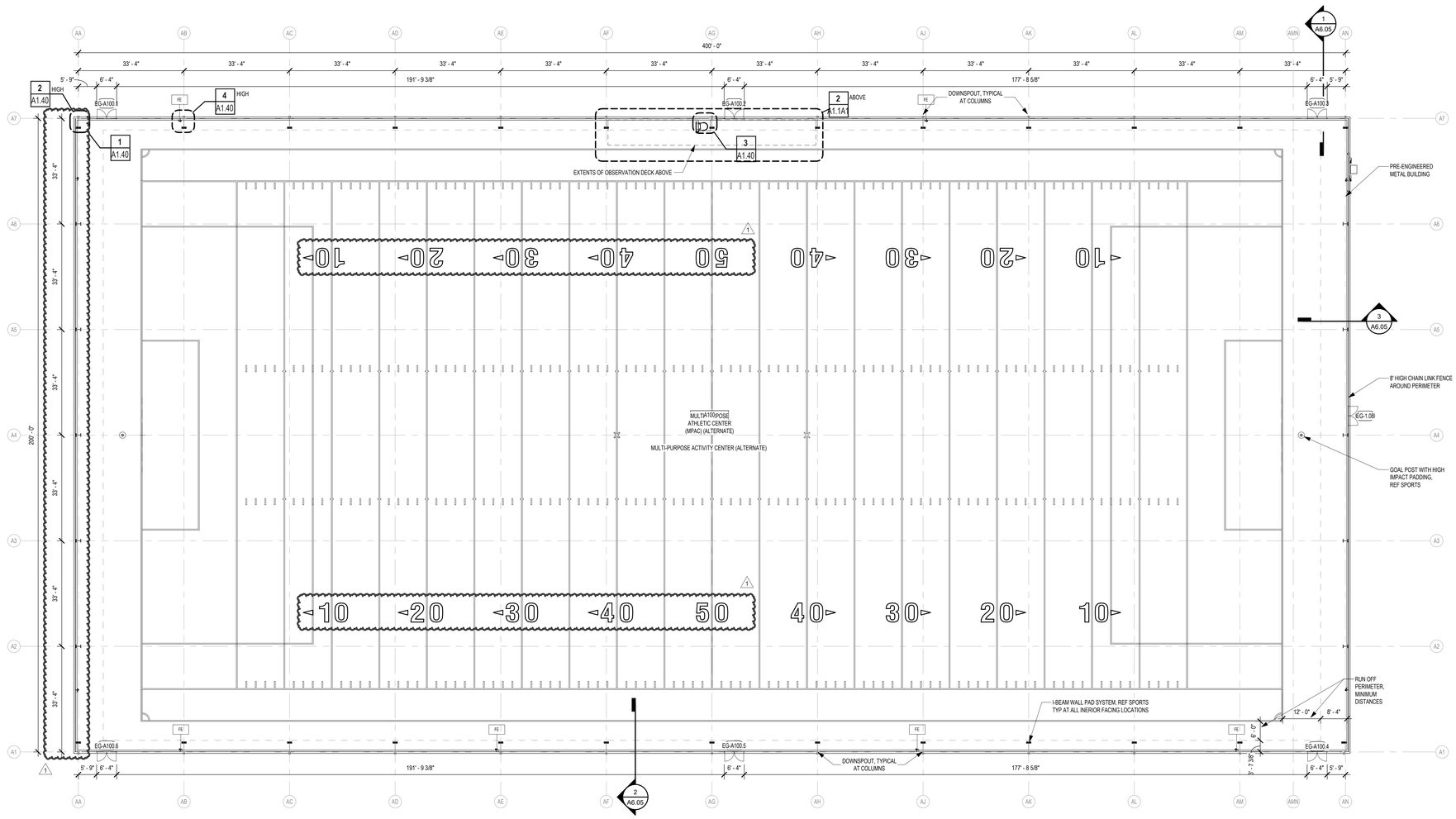
1. ALL INTERIOR WALL/PARTITIONS NOT OTHERWISE DESCRIBED BY A WALL SECTION ARE IDENTIFIED BY A SYMBOL WITH AN ALPHANUMERIC CODE INDICATING MATERIALS (BY LETTER), NOMINAL THICKNESS OF STRUCTURE (BY NUMBER) AND AN ALPHANUMERIC CODE REPRESENTING HEIGHT, FIRE RATING AND ACOUSTICAL PROPERTIES.
2. **UNTAGGED WALL/PARTITIONS AND COLUMN FURRINGS** SHALL BE ASSUMED TO BE **"TYPICAL"**, AS FOLLOWS, FOR BIDDING PURPOSES:
   GYP BOARD (EXTENDS TO 6" ABOVE CEILING U.N.O.)
   WALLS/PARTITIONS - TYPE **H4**, ACOUSTICAL
   COLUMN FURRING - **M4**, NON-ACOUSTICAL
   MASONRY (EXTENDS TO 4" ABOVE CEILING OR TO NEAREST 8" COURSING ABOVE 4" U.N.O.)
   WALLS/PARTITIONS - TYPE **A8**, NON-ACOUSTICAL
   COLUMN FURRING - TYPE **A8**, NON-ACOUSTICAL
   ANY UNTAGGED PARTITIONS WHICH APPEAR INCONSISTENT WITH THE DESCRIPTIONS ABOVE SHALL BE VERIFIED WITH THE ARCHITECT PRIOR TO BIDDING.

REF. G3 SHEETS FOR EXTERIOR WALL TYPES AND INTERIOR PARTITION TYPE ASSEMBLIES.

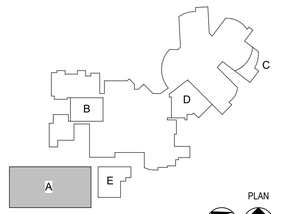
PARTITION LEGEND



2 ENLARGED PLAN @ OBSERVATION DECK
1/8" = 1'-0" BACK REF: A1.1A1



1 FLOOR PLAN - AREA A
1/16" = 1'-0"



KEY PLAN
NTS

Revision /	Date	
1	05/08/25	ADDENDUM 01

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

05.08.25

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FLOOR PLAN - AREA A

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A1.1A1
Drawn By: YR/AB	
Date: 05/08/2025	

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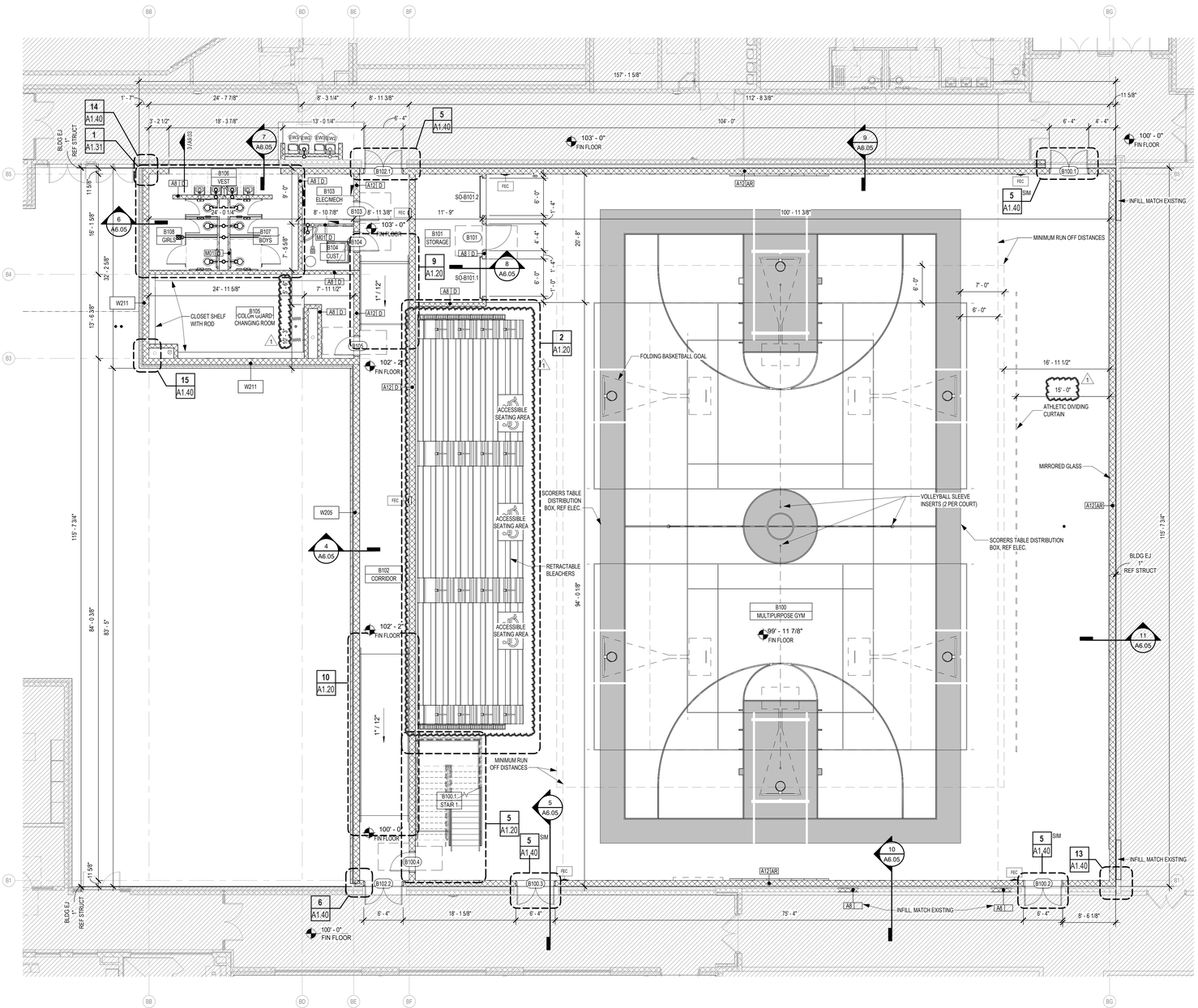
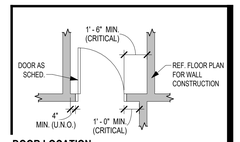


1 FLOOR PLAN - AREA B
1/8" = 1'-0"



DOOR LOCATION

NOTES:

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3. SEE STRUCTURAL DRAWINGS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS.
4. SEE SPECIFICATIONS AND G3 SHEETS FOR ADDITIONAL MASONRY AND CFMF PARTITION REQUIREMENTS.
5. SEE PLUMBING PLANS AND ENLARGED FLOOR PLANS FOR LOCATIONS OF FIXTURES.
6. REF. DOOR SCHEDULES AND DETAILS FOR MASONRY ROUGH OPENING SIZES.
7. VERIFY ALL MASONRY OPENINGS WITH MANUFACTURERS' PRODUCTS (E.G. FIRE EXTINGUISHER CABINETS, ELECTRICAL PANELS, ETC.).
8. PROVIDE WD. BLOCKING IN ALL GYP. BD. WALLS AS REQ'D. AT LOCATIONS INCLUDING, BUT NOT LIMITED TO; HANDRAILS, TOILET ACCESSORIES, PROJECTORS, CASEWORK, TACK BD.'S, MARKER BD.'S, WALL STOPS & DOOR HOLDERS.
9. PROVIDE SPRAYED FIRE RESISTANT MATERIAL AT ALL CONCEALED COLUMNS AND OTHER PRIMARY STRUCTURE, AND INSUMESCENT COATING AT ALL EXPOSED (INT + EXT) COLUMNS AS REQUIRED - REF G2 CODE PLANS.

GENERAL FLOOR PLAN NOTES

FLOOR PLAN LEGEND

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- **F.E.C.** — **FIRE EXTINGUISHER CABINET** - REF. SPEC. SECT. 10 4400. PROVIDE TYPE "K" @ KITCHEN AREAS
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- **NOT IN CONTRACT** APPLIES TO AREA PLANS

REF. TYP. CONST. DTLS. FOR ADDITIONAL INFO.

- **MASONRY VENEER**
- **STANDARD CMU**
- **CFMF PARTITION**
- **EXISTING CFMF PARTITION**

PARTITION TYPE TAGS

A3 / D

WALL ASSEMBLY'N PLAN VIEW

LETTERS TO THE RIGHT INDICATE PARTITION HEIGHT, FIRE RATING AND ACOUSTICAL PROPERTIES.

D	WALL TO DECK / STRUCTURE
C	WALL TO 6" (MIN.) ABOVE CEILING
FR	FIRE RATED WALL TO DECK
AD	ACOUSTICAL WALL TO DECK
AC	ACOUSTICAL WALL TO 6" (MIN.) ABOVE CEILING
AR	ACOUSTICAL RATED WALL TO DECK
GD	GYP WALL ABOVE TO DECK
SR	SMOKE RESISTANT PARTITION

LETTER & NUMBER TO THE LEFT INDICATE PARTITION TYPE & STUD OR CMU SIZE AS FOLLOWS:

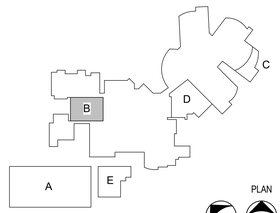
- 1 - 7/8" HAT CHANNEL
- 2 - 1 5/8" CFMF
- 3 - 2 1/2" CFMF
- 4 - 3 5/8" CFMF OR CMU
- 6 - 6" CFMF OR 5 5/8" CMU
- 8 - 8" CFMF OR 7 5/8" CMU
- 10 - 9 5/8" CMU
- 12 - 11 5/8" CMU

PARTITION NOTES:

1. ALL INTERIOR WALL/PARTITIONS NOT OTHERWISE DESCRIBED BY A WALL SECTION ARE IDENTIFIED BY A SYMBOL WITH AN ALPHANUMERIC CODE INDICATING MATERIALS (BY LETTER), NOMINAL THICKNESS OF STRUCTURE (BY NUMBER) AND AN ALPHANUMERIC CODE REPRESENTING HEIGHT, FIRE RATING AND ACOUSTICAL PROPERTIES.
2. **UNTAGGED WALL/PARTITIONS AND COLUMN FURRINGS** SHALL BE ASSUMED TO BE **"TYPICAL"**, AS FOLLOWS, FOR BIDDING PURPOSES:
   GYP BOARD (EXTENDS TO 6" ABOVE CEILING U.N.O.)
   WALLS/PARTITIONS - TYPE **M4**, ACOUSTICAL
   COLUMN FURRING - **M4**, NON-ACOUSTICAL
   MASONRY (EXTENDS TO 4" ABOVE CEILING OR TO NEAREST 8" COURSING ABOVE 4" U.N.O.)
   WALLS/PARTITIONS - TYPE **A8**, NON-ACOUSTICAL
   COLUMN FURRING - TYPE **A6**, NON-ACOUSTICAL
   ANY UNTAGGED PARTITIONS WHICH APPEAR INCONSISTENT WITH THE DESCRIPTIONS ABOVE SHALL BE VERIFIED WITH THE ARCHITECT PRIOR TO BIDDING.

REF. G3 SHEETS FOR EXTERIOR WALL TYPES AND INTERIOR PARTITION TYPE ASSEMBLIES.

PARTITION LEGEND



KEY PLAN
NTS

PLAN NORTH
TRUE NORTH

Revision /	Date	
1	05/08/25	ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT
JAMES JASON JOHNSON
29417
STATE OF TEXAS
05.08.25

Huckabee
www.huckabee-inc.com
800.687.1229

FLOOR PLAN - AREA B - LEVEL 1

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. A1.1B1
Drawn By: YR/AB	ISSUE FOR BID
Date: 05/08/2025	

5/8/2025 2:15:42 PM
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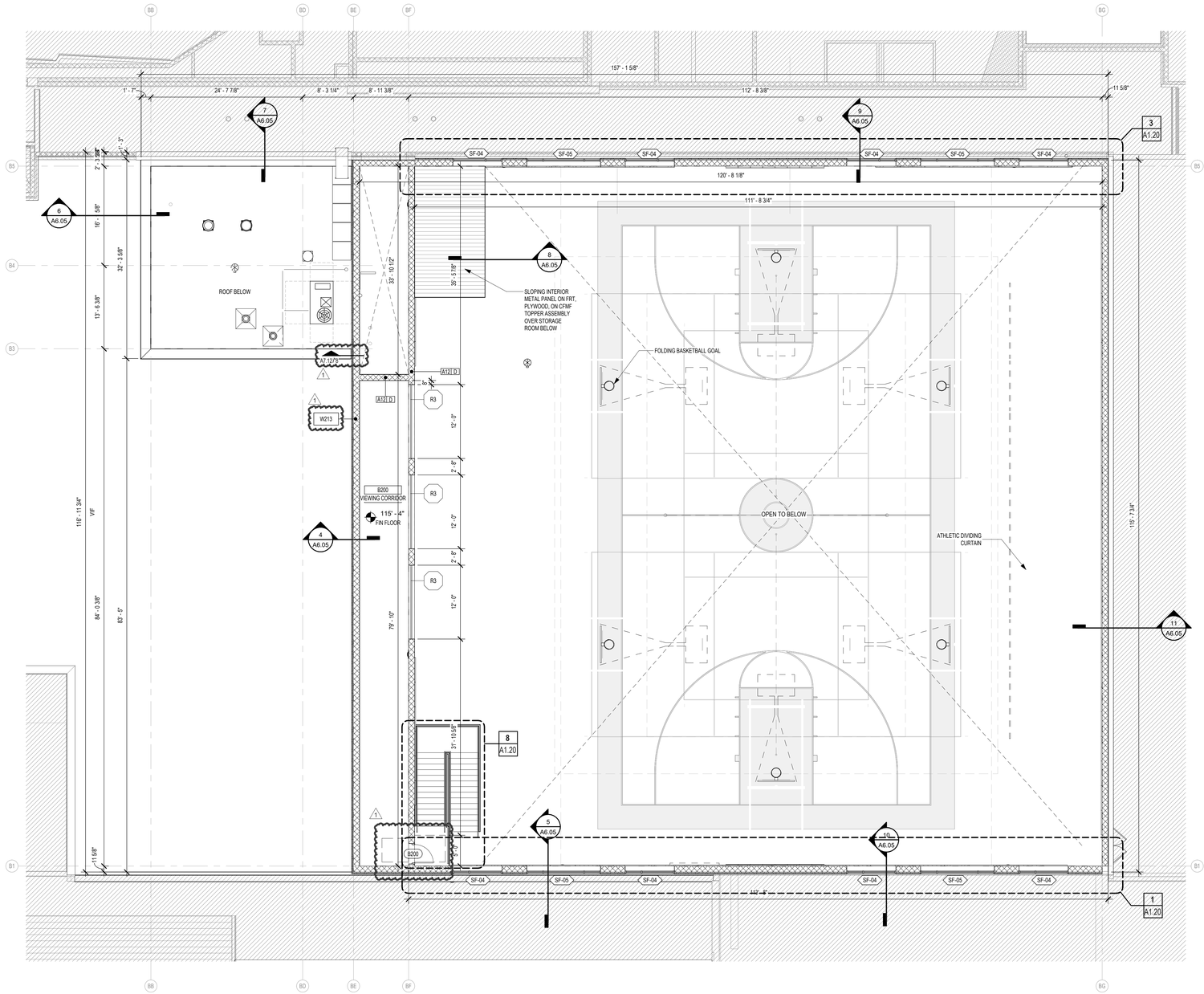
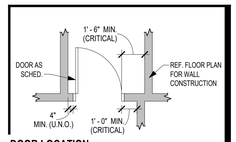


1 FLOOR PLAN - AREA B - LEVEL 2
1/8" = 1'-0"



DOOR LOCATION

NOTES:

1. THE CONTRACT DOCUMENTS ARE COMPRISED OF BOTH THE DRAWINGS AND THE SPECIFICATIONS.
2. ALL ARCHITECTURAL FLOOR PLAN DIMENSIONS SHOWN ARE TO FACE OF C.F.M.F. MASONRY OR CONCRETE, U.N.O.
3. SEE STRUCTURAL DRAWINGS FOR ADDITIONAL REINFORCEMENT REQUIREMENTS.
4. SEE SPECIFICATIONS AND G3 SHEETS FOR ADDITIONAL MASONRY AND CFMF PARTITION REQUIREMENTS.
5. SEE PLUMBING PLANS AND ENLARGED FLOOR PLANS FOR LOCATIONS OF FIXTURES.
6. REF. DOOR SCHEDULES AND DETAILS FOR MASONRY ROUGH OPENING SIZES.
7. VERIFY ALL MASONRY OPENINGS WITH MANUFACTURERS' PRODUCTS (E.G. FIRE EXTINGUISHER CABINETS, ELECTRICAL PANELS, ETC.).
8. PROVIDE WD. BLOCKING IN ALL GYP. BD. WALLS AS REQ'D. AT LOCATIONS INCLUDING, BUT NOT LIMITED TO; HANDRAILS, TOILET ACCESSORIES, PROJECTORS, CASEWORK, TACK BD.'S, MARKER BD.'S, WALL STOPS & DOOR HOLDERS.
9. PROVIDE SPRAYED FIRE RESISTANT MATERIAL AT ALL CONCEALED COLUMNS AND OTHER PRIMARY STRUCTURE, AND INSUMESCENT COATING AT ALL EXPOSED (INT + EXT) COLUMNS AS REQUIRED - REF G2 CODE PLANS.

GENERAL FLOOR PLAN NOTES

F.E. **FIRE EXTINGUISHER (WALL MOUNTED)** - REF. SPEC. SECT. 10 4400. PROVIDE TYPE "K" @ KITCHEN AREAS

F.E.C. **FIRE EXTINGUISHER CABINET** - REF. SPEC. SECT. 10 4400. PROVIDE TYPE "K" @ KITCHEN AREAS

P.S. **PROJECTION SCREEN** REF. TYP. DET. ON A4 SHEETS

X' M.B. **MARKERBOARD** REF. TYP. DET. ON A4 SHEETS

X' T.B. **TACKBOARD** REF. TYP. DET. ON A4 SHEETS

ELECTRIC WATER COOLER - REF PLUMBING PLAN.

1.01 **DOOR ASSEMBLY NUMBER** - REF. SCHEDULE ON A3 SHTS.

SF-A **WINDOW/STOREFRONT/CURTAIN WALL ASSEMBLY TYPE** - REF. A3 SHTS.

S01.01 **SPECIAL OPENING NUMBER (ROLLING DOOR)** - REF. SCHEDULE ON A3 SHTS.

1 / A1.1 **CALLOUT REFERENCE**

1 / A101 SIM **WALL SECTION REFERENCE** XX / A101

XXX' - X" FIN FLOOR **ELEVATION REFERENCE**

1-01 ROOM **ROOM NUMBER & NAME**

NOT IN CONTRACT

APPLIES TO AREA PLANS

FLOOR PLAN LEGEND

REF. TYP. CONST. DTLS. FOR ADDITIONAL INFO.

MASONRY VENEER

STANDARD CMU

CFMF PARTITION

EXISTING CFMF PARTITION

PARTITION TYPE TAGS

A3 / D — WALL ASSEMBLY'N PLAN VIEW

LETTERS TO THE RIGHT INDICATE PARTITION HEIGHT, FIRE RATING AND ACOUSTICAL PROPERTIES.

D	WALL TO DECK / STRUCTURE
C	WALL TO 9' (MIN.) ABOVE CEILING
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AC	ACOUSTICAL WALL TO 9' (MIN.) ABOVE CEILING
AR	ACOUSTICAL RATED WALL TO DECK
GD	GYP WALL ABOVE TO DECK
SR	SMOKE RESISTANT PARTITION

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10 - 9 5/8" CMU
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   COLUMN FURRING - **M4**, NON-ACOUSTICAL
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   WALLS/PARTITIONS - TYPE **A8**, NON-ACOUSTICAL
   COLUMN FURRING - TYPE **A8**, NON-ACOUSTICAL
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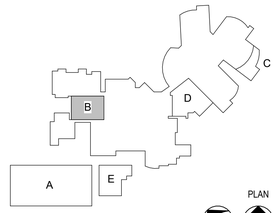
REF. G3 SHEETS FOR EXTERIOR WALL TYPES AND INTERIOR PARTITION TYPE ASSEMBLIES.

PARTITION LEGEND



KEY PLAN
NTS

PLAN TRUE NORTH

Revision /	Date	
1	05/08/25	ADDENDUM 01

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT — BRENT L. JARRARD — 24417 — STATE OF TEXAS
05.08.25

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FLOOR PLAN - AREA B - LEVEL 2

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A1.1B2
Drawn By: YB/AB	
Date: 05/08/2025	

5/8/2025 2:15:49 PM
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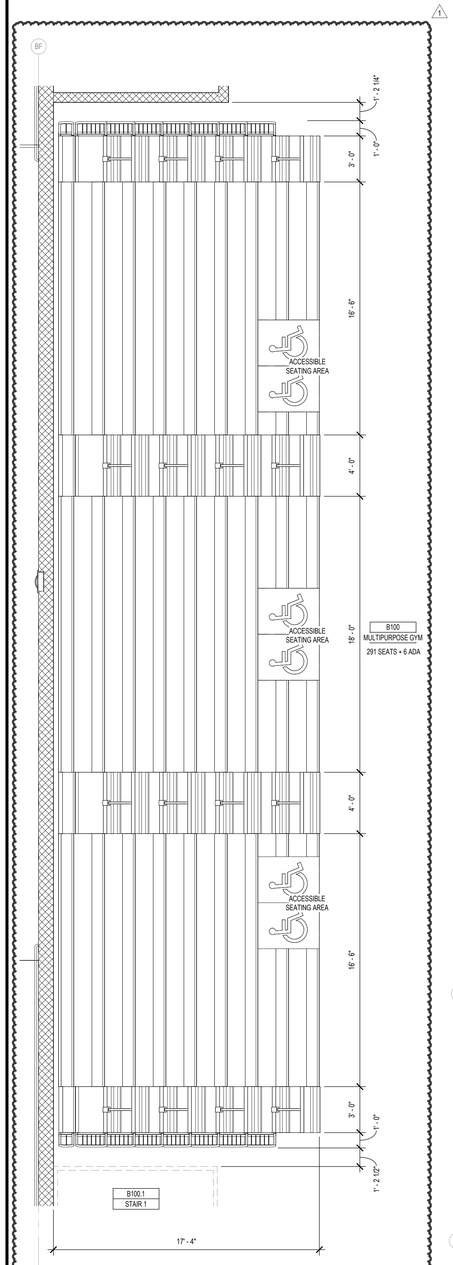
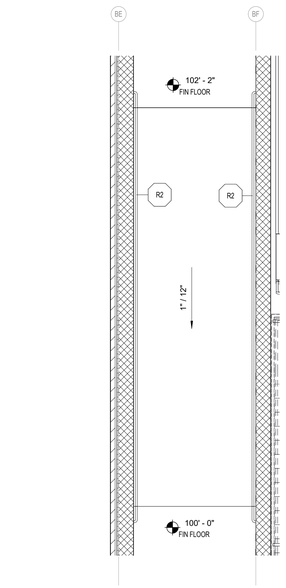
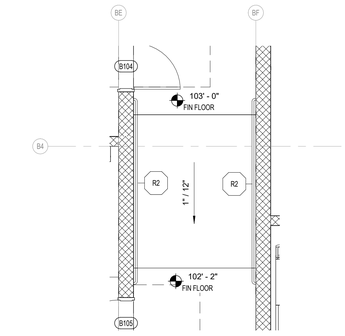


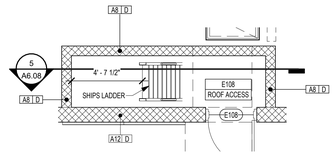
2 ENLARGED FLOOR PLAN - B100 - MULTIPURPOSE ROOM
1/4" = 1'-0" BACK REF: A1.1B1



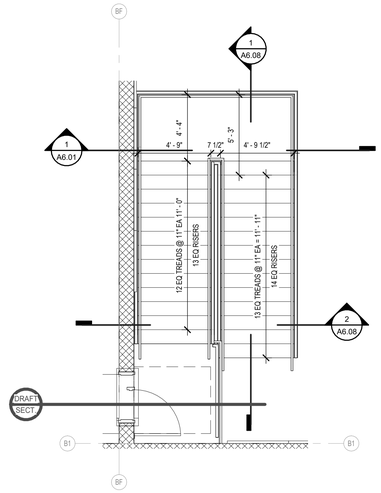
10 ENLARGED RAMP - B101 - MULTIPURPOSE
1/4" = 1'-0" BACK REF: A1.1B1



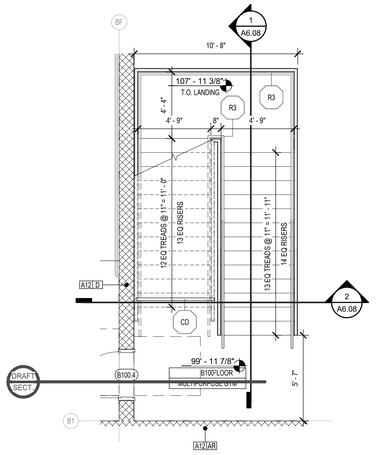
9 ENLARGED RAMP - B101 - MULTIPURPOSE
1/4" = 1'-0" BACK REF: A1.1B1



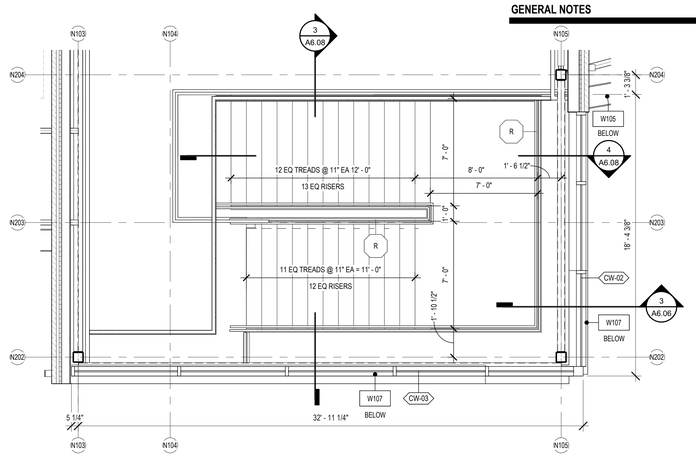
11 ENLARGED STAIR PLAN - F108 ROOF ACCESS
1/4" = 1'-0" BACK REF: A1.1E1



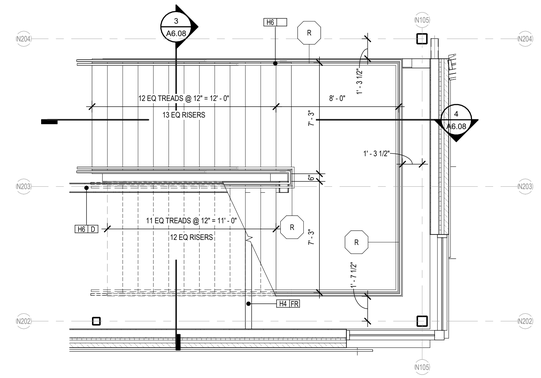
8 ENLARGED STAIR PLAN - B100 - MULTIPURPOSE ROOM
1/4" = 1'-0" BACK REF: A1.1B2



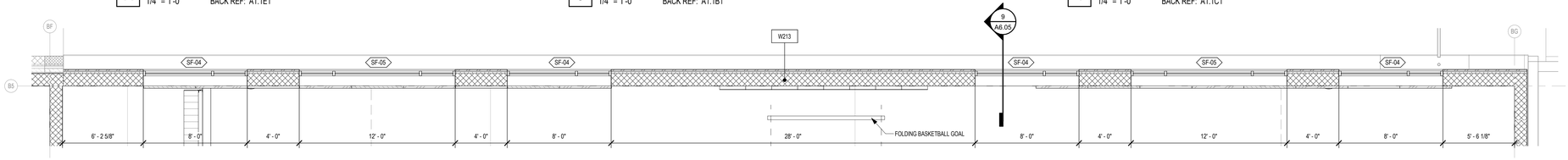
5 ENLARGED STAIR PLAN - B100 - MULTIPURPOSE ROOM
1/4" = 1'-0" BACK REF: A1.1B1



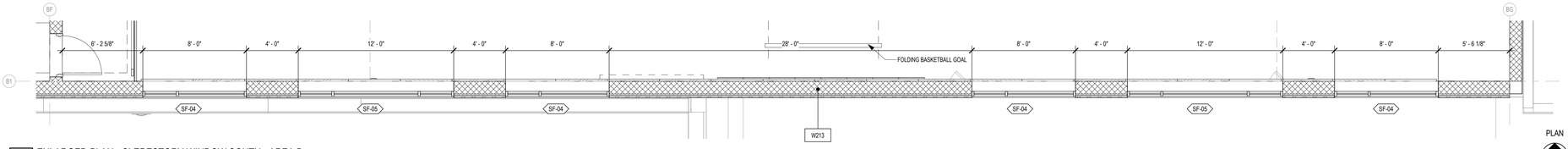
7 CLERESTORY PLAN - AREA C - LEVEL 2
1/4" = 1'-0" BACK REF: A1.1C2



4 ENLARGED STAIR PLAN - C1.30 STAIR
1/4" = 1'-0" BACK REF: A1.1C1



3 ENLARGED PLAN - CLERESTORY WINDOW NORTH - AREA B
1/4" = 1'-0"



1 ENLARGED PLAN - CLERESTORY WINDOW SOUTH - AREA B
1/4" = 1'-0"

PLAN N

GENERAL NOTES

1. FOR ACCESSIBLE FIXTURES & ACCESSORIES REFER TO SHEET A1.30 "TOILET ACCESSORIES AND PLUMBING FIXTURES" FOR MOUNTING HEIGHTS & ACCESSIBILITY REQUIREMENTS. THE ACCESSORIES ARE CALLED OUT IN THE INDIVIDUAL STALL / ROOM TYPES / SHOWER TYPES.
2. FIXTURES THAT STAND ALONE WILL BE TAGGED W/ A SYMBOL; LAVS, URINAL'S & EWC'S
3. PROVIDE PIPE INSULATION AS SPECIFIED AT ALL EXPOSED PIPING AND DRAIN AT ACCESSIBLE LAVATORIES.

PLAN LEGEND

30" x 48" CLEAR FLOOR SPACE PER ACCESSIBILITY REQUIREMENTS

5' - 0" DIAMETER TURNING SPACE PER ACCESSIBILITY REQUIREMENTS

Revision / 1 — Date 05/08/25 — ADDENDUM 01

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT — JAMES R. JOHNSON — 19411 — STATE OF TEXAS
05.08.25

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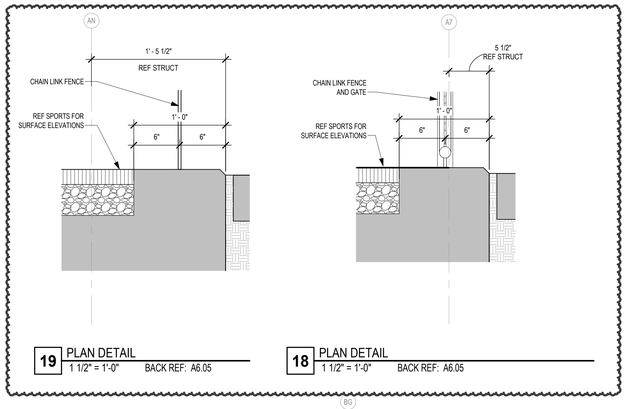
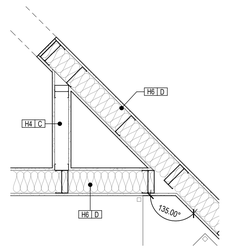
ENLARGED PLANS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. A1.20
Drawn By: YR/AB	ISSUE FOR BID
Date: 05/08/2025	

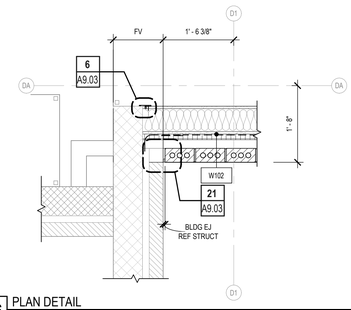


19 PLAN DETAIL
1 1/2" = 1'-0" BACK REF: A6.05

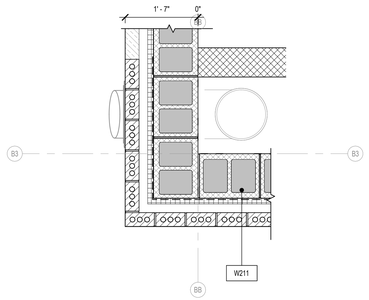
18 PLAN DETAIL
1 1/2" = 1'-0" BACK REF: A6.05



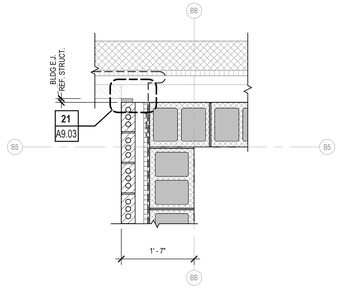
17 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1D1



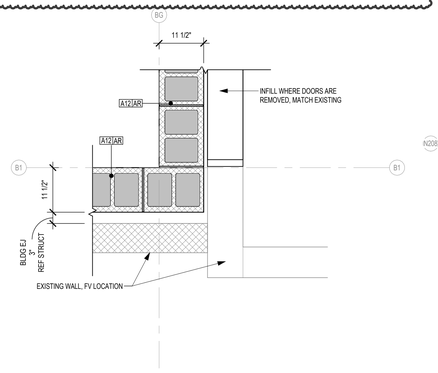
16 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1D1



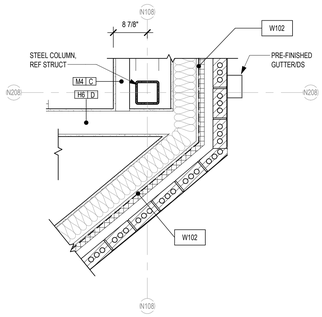
15 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1B1



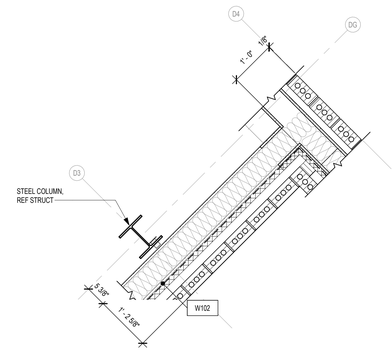
14 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1B1



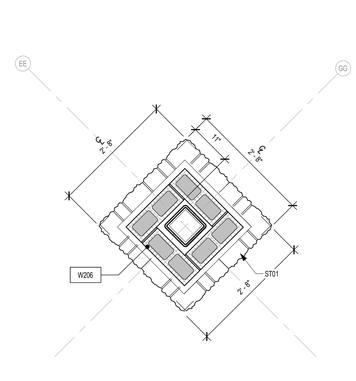
13 FLOOR PLAN - AREA B - LEVEL 1 - Callout 8
3/4" = 1'-0" BACK REF: A1.1B1



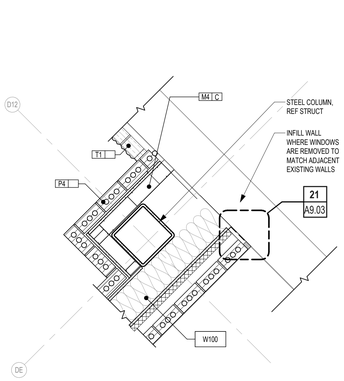
12 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1C1



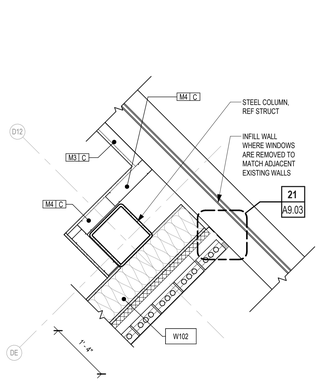
11 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1D1



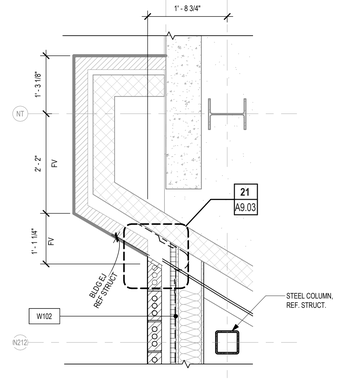
10 FLOOR PLAN - AREA D - LEVEL 1 - Callout 5
3/4" = 1'-0" BACK REF: A1.1D1



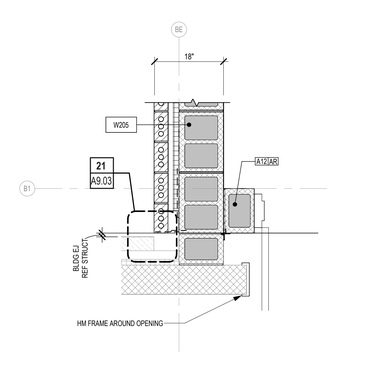
9 PLAN DETAIL - LOW
3/4" = 1'-0" BACK REF: A1.1D1



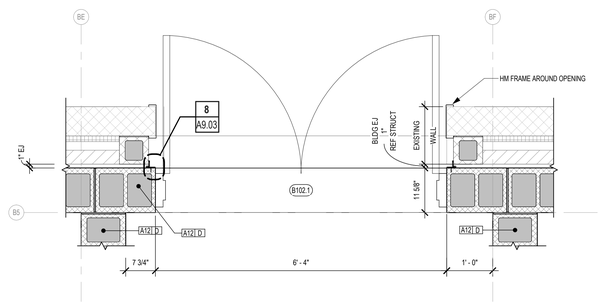
8 PLAN DETAIL - HIGH
3/4" = 1'-0" BACK REF: A1.1D1



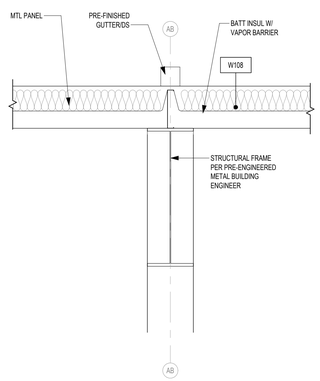
7 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1C1



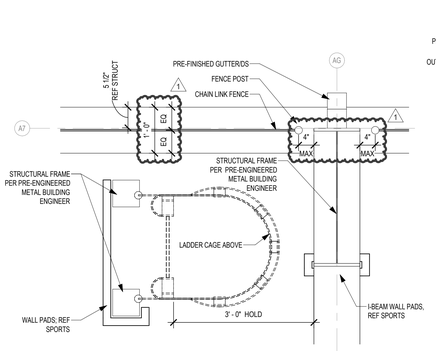
6 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1B1



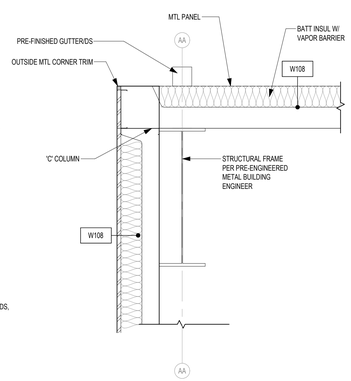
5 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1B1



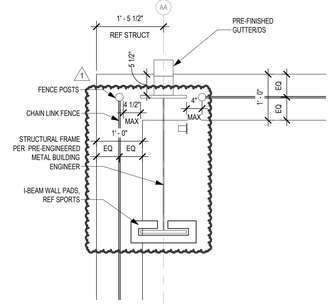
4 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1A1



3 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1A1



2 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1A1



1 PLAN DETAIL
3/4" = 1'-0" BACK REF: A1.1A1

PLAN
N

Revision /	Date
1	05/08/25 ADDENDUM 01

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT
JAMES JASON HUCKABEE
24117
STATE OF TEXAS
05.08.25

Huckabee
www.huckabee-inc.com
800.687.1229

PLAN DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. A1.40
Drawn By: YR/AB	ISSUE FOR BID
Date: 05/08/2025	

1 REFLECTED CEILING PLAN - AREA C - LEVEL 1
1/8" = 1'-0"

1. COORDINATE REFLECTED CEILING PLANS WITH MECHANICAL AND ELECTRICAL DRAWINGS TO AVOID CONFLICTS. VERIFY EXACT MOUNTING LOCATIONS OF ALL CEILING-MOUNTED EQUIPMENT PRIOR TO INSTALLATION. CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR VERIFYING ALL ITEMS, EQUIPMENT, AND/OR DEVICES AT CEILING.
2. PERIMETER WALLS AND/OR PERIMETER PARTITIONS OF ALL SPACES WITH EXPOSED STRUCTURE SHALL EXTEND TO BOTTOM OF DECK ABOVE. FINISH EXPOSED STRUCTURE AS SCHED.
3. CEILINGS AND GRIDS AT ALL MDFS AND IDFS SHALL BE HELD OFF OF DISTRIBUTION WALL 6" TO ACCOMMODATE CABLING.
4. PROVIDE 24"x24" ACCESS PANEL(S) IN ALL HARD CEILINGS FOR MAINTENANCE OF EQUIPMENT.
5. ALL DIMENSIONS SHOWN ON REFLECTED CEILING PLAN DRAWINGS ARE FROM FACE OF FINISH, U.N.O.

REFLECTED CEILING PLAN GENERAL NOTES

REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION REGARDING FINISH MATERIALS AND PAINT FINISH CODES.

CEILING TYPE → C4 ← ROOM NUMBER
CEILING HEIGHT → 9' - 0"

CEILING

- **ES** - EXPOSED DECK AND STRUCTURE
- **ESF** - EXPOSED DECK AND STRUCTURE (FINISHED)
- **GYP** - 5/8" GYPSUM BOARD
- **GYP1** - 3 LAYERS 5/8" TYPE 'X' GYPSUM BOARD (1-HR. FIRE RATED) (UL-I301)
- **L1** - TYPE I 24" x 24" LAY-IN SUSPENDED CEILING TILE (STANDARD)
- **L2** - TYPE II 24" x 24" LAY-IN SUSPENDED CEILING TILE (IMPACT RESISTANT)
- **L3** - TYPE III 24" x 24" LAY-IN SUSPENDED CEILING TILE (HIGH HUMIDITY)
- **L4** - TYPE IV 24" x 24" LAY-IN SUSPENDED CEILING TILE (VINYL COVERED) (FOODSERVICE)
- **L5** - TYPE V 24" x 24" LAY-IN SUSPENDED CEILING TILE (ACOUSTICAL 'REFLECTIVE')
- **L6** - TYPE VI 24" x 24" LAY-IN SUSPENDED CEILING TILE (ACOUSTICAL 'ABSORPTIVE') (GLASS FIBER)
- **L7** - TYPE VII 24" x 24" LAY-IN SUSPENDED CEILING TILE (ACOUSTICAL 'ABSORPTIVE' - GEN. USE)
- **L8** - TYPE VIII 24" x 24" LAY-IN SUSPENDED CEILING TILE (UL FIRE RESISTANT)
- **L9** - TYPE IX LARGE FORMAT LAY-IN SUSPENDED CEILING TILE (GLASS FIBER)
- **MCS** - METAL CEILING SYSTEM
- **MS** - METAL SOFFIT PANEL
- **PL** - PLASTER
- **WD** - LINEAR WOOD CEILING

DEFAULT FINISHES

IF ANY ROOM IS NOT IDENTIFIED BY A ROOM FINISH TAG, THE FOLLOWING FINISHES SHALL BE USED AS "TYPICAL" OR "DEFAULT" FINISHES FOR BIDDING PURPOSES. VERIFY FINISHES WITH ARCHITECT BEFORE INSTALLATION.

CEILING - L1
CEILING HEIGHT - 9' - 0" A.F.F.

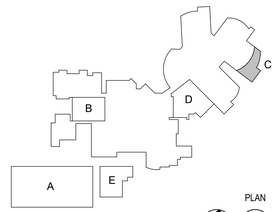
CEILING FINISH LEGEND

- **LAY-IN CEILING**
- C.J. **GYPSUM BOARD CEILING** WITH CONTROL JOINTS . FINISH AS INDICATED
- C.J. **PLASTER SOFFIT** WITH CONTROL JOINTS. FINISH AS INDICATED
- **METAL SOFFIT PANELS**
- **LIGHT FIXTURES** - REF. ELECTRICAL
- P.S. **PROJECTION SCREENS** - REF. A4 SHEETS
- P.S.
- **HVAC GRILLE, DIFFUSER** - REF. MECHANICAL DRAWINGS.
- **SOUND BATT ABOVE CEILING**

  WHERE THIS HATCH IS SHOWN, PROVIDE FULL COVERAGE ACOUSTICAL BATT INSULATION ABOVE FINISHED CEILING.
- **FIRE RATED CEILING ASSEMBLY**
- **EXISTING CEILING**

WHERE THIS HATCH IS SHOWN, PROVIDE A FIRE-RATED CEILING ASSEMBLY AS DETAILED AND NOTED. REFER TO PLAN NOTES AND CONSTRUCTION DETAILING FOR TERMINATION POINT OF ADJACENT FIRE-RATED WALL ASSEMBLIES. RATING SHALL EQUAL THAT OF SURROUNDING WALLS.

REFLECTED CEILING LEGEND



KEY PLAN
NTS

PLAN NORTH
TRUE NORTH

Revision /	Date
1	05/08/25 ADDENDUM 01

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
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REGISTERED ARCHITECT
THOMAS JASON HUCKABEE
29417
STATE OF TEXAS

05.08.25

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800.687.1229

REFLECTED CEILING PLAN - AREA C - LEVEL 1

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A2.1C1
Drawn By: YR/AB	
Date: 05/08/2025	

5/8/2025 2:16:44 PM
Autodesk Docs://HAYS CISD - LEHMAN, JOHNSON HS ADD RENO 2025/01954-08-01_LHS2025_V23.rvt

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CEILING HEIGHT - 9' - 0" A.F.F.

CEILING FINISH LEGEND

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- **GYPSUM BOARD CEILING** WITH CONTROL JOINTS . FINISH AS INDICATED (C.J.)
- **PLASTER SOFFIT** WITH CONTROL JOINTS. FINISH AS INDICATED (C.J.)
- **METAL SOFFIT PANELS**
- **LIGHT FIXTURES** - REF. ELECTRICAL
- **PROJECTION SCREENS** - REF. A4 SHEETS (P.S.)
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REFLECTED CEILING LEGEND

Revision /	Date	
1	05/08/25	ADDENDUM 01

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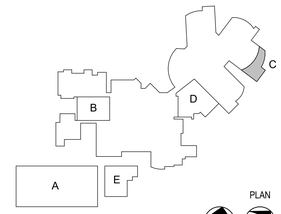


KEY PLAN
NTS.

PLAN
N
TRUE

REFLECTED CEILING PLAN - AREA C - LEVEL 2

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A2.1C2
Drawn By: YR/AB	
Date: 05/08/2025	

1 REFLECTED CEILING PLAN - AREA C - LEVEL 2
1/8" = 1'-0"

A3 - EGRESS GATE SCHEDULE

MARK	AAOS Shared HW Set	GATE HEIGHT	GATE WIDTH	GATE MATERIAL	SECURITY	PANIC HARDWARE	DETAILS AND NOTES - COMMENTS
EG-1.08	J704X	8' - 0"	6' - 0"	CHAIN LINK		Yes	
EG-A100.1	J704X	8' - 0"	6' - 0"	CHAIN LINK		Yes	ALTERNATE NO. 1
EG-A100.2	J704X	8' - 0"	6' - 0"	CHAIN LINK		Yes	ALTERNATE NO. 1
EG-A100.3	J704X	8' - 0"	6' - 0"	CHAIN LINK		Yes	ALTERNATE NO. 1
EG-A100.4	J704X	8' - 0"	6' - 0"	CHAIN LINK		Yes	ALTERNATE NO. 1
EG-A100.5	J704X	8' - 0"	6' - 0"	CHAIN LINK		Yes	ALTERNATE NO. 1
EG-A100.6	J704X	8' - 0"	6' - 0"	CHAIN LINK		Yes	ALTERNATE NO. 1

A3 - LOUVER SCHEDULE

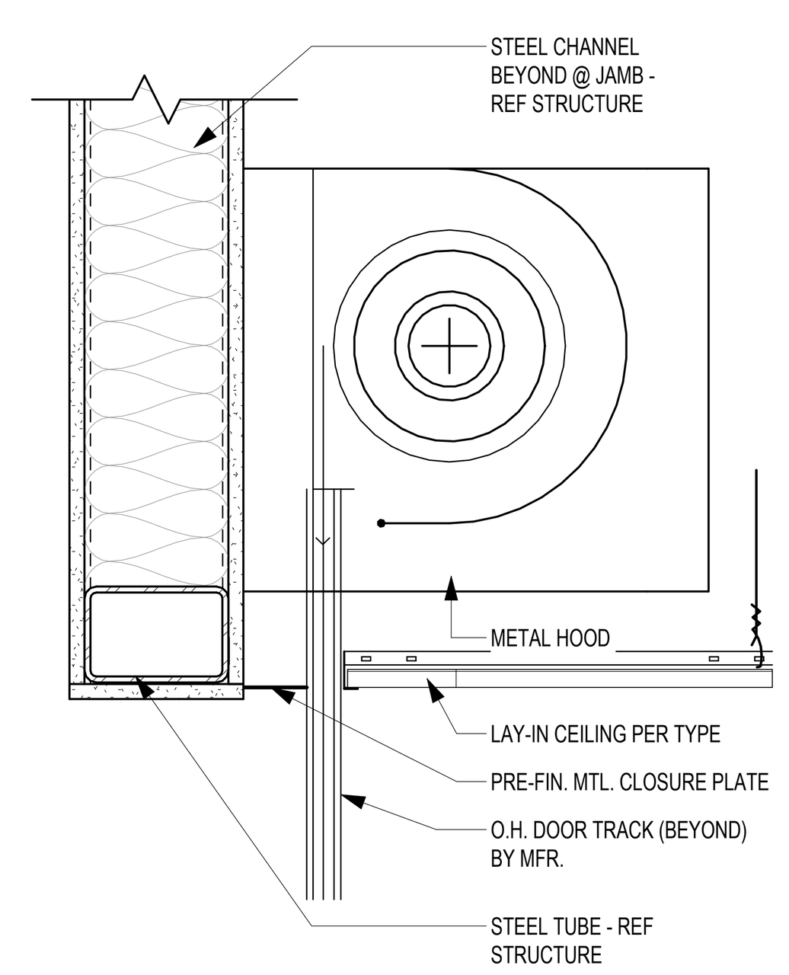
FR TYPE	FRAME DESCRIPTION - FRAME WDTH	FRAME DESCRIPTION - FRAME HT	DETAILS AND NOTES - NOTES
U	9' - 0"	6' - 0"	

A3 - DOOR SCHEDULE

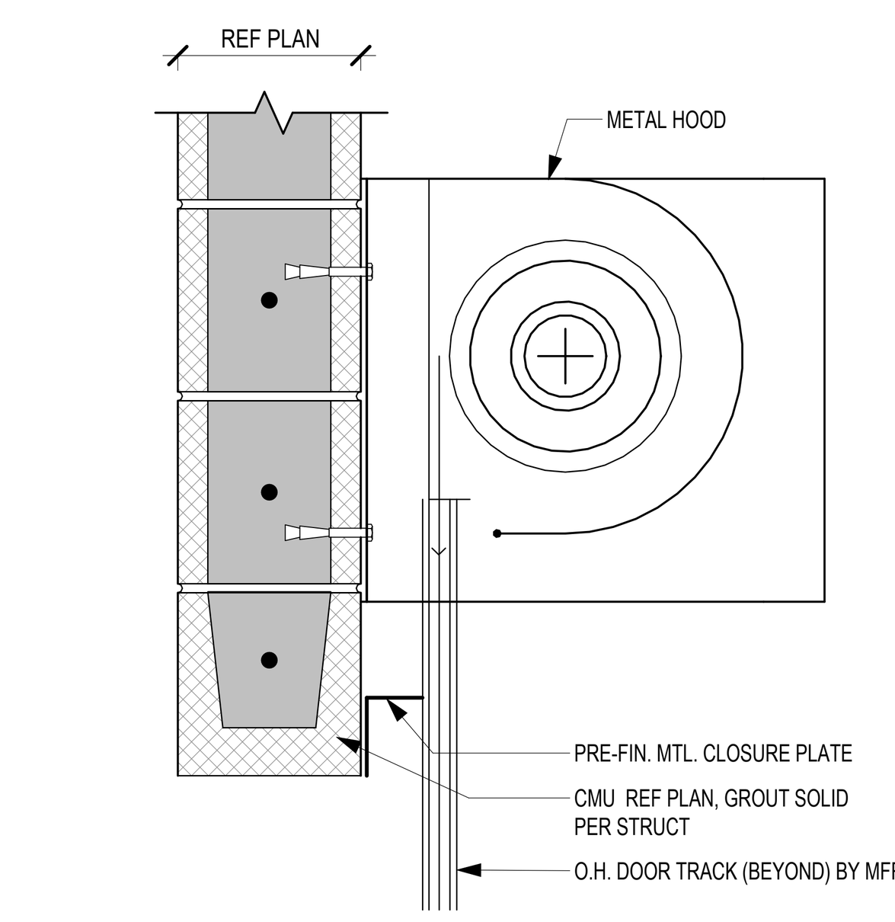
OPENING NUMBER	OPENING - HARDWARE SET	OPENING - FIRE RATING	OPENING - EXTERIOR	DOOR - WIDTH	DOOR - HEIGHT	DOOR - TYPE	DOOR - DR GLAZING	DOOR - PANIC HARDWARE	FRAME - MATERIAL	FRAME - FRAME TYPE	NOTES	OPENING NUMBER
AREA B												
B100.1	D700CM	45 MIN	No	3' - 0"	7' - 0"	HM-NL (PR)	GTC	Yes	HM	B1		B100.1
B100.2	D700CM	45 MIN	No	3' - 0"	7' - 0"	HM-NL (PR)	GTC	Yes	HM	B1		B100.2
B100.3	D700M	45 MIN	Yes	3' - 0"	7' - 0"	HM-NL (PR)	IGSE	Yes	HM	B1		B100.3
B100.4	D701C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B1		B100.4
B101	203SW		No	4' - 0"	7' - 0"	SC-FL	--		HM	B1		B101
B102.1	D700M	45 MIN	No	3' - 0"	7' - 0"	HM-2L (PR)	GTC	Yes	HM	B1		B102.1
B102.2	D700CM	45 MIN	Yes	3' - 0"	7' - 0"	HM-2L (PR)	IGSE	Yes	HM	B1		B102.2
B103	711C	45 MIN	No	3' - 0"	7' - 0"	SC-FL	--		HM	B1		B103
B104	221	45 MIN	No	3' - 0"	7' - 0"	SC-FL	--		HM	B1		B104
B105	221	45 MIN	No	3' - 0"	7' - 0"	SC-FL	--		HM	B1		B105
B200	CE201		No	3' - 0"	7' - 0"	HM-NL	GTC		HM	B1		B200
AREA B - OVERHEAD												
SO-B101.1	001		No	6' - 0"	8' - 0"	OCD-STL			STL		13	SO-B101.1
SO-B101.2	001		No	6' - 0"	8' - 0"	OCD-STL			STL		13	SO-B101.2
AREA C												
C100.1	C714AM		Yes	3' - 0"	7' - 0"	AL-SF (PR)	IGSE	Yes	AL	CW-04		C100.1
C100.2	CE201C		No	3' - 0"	7' - 0"	SC-NL	IGSE	Yes	HM	B		C100.2
C100.3	C714AM		Yes	3' - 0"	7' - 0"	AL-SF (PR)	IGLSA		AL	CW-01		C100.3
C101	CE201C		No	3' - 0"	7' - 0"	SC-NL	IGSE	Yes	HM	B		C101
C102	CE201C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C102
C103	CE201		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C103
C105	CE201C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C105
C106	CE201C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C106
C107	343		No	3' - 0"	7' - 0"	SC-FL	--		HM	B		C107
C108	343		No	3' - 0"	7' - 0"	SC-FL	--		HM	B		C108
C109.1	C711		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C109.1
C109.2	C711		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C109.2
C110	201		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C110
C111	CE201		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C111
C201	CE201C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C201
C202	CE201C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C202
C203	CE201		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C203
C204	C711C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C204
C205	CE201		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C205
C206	C711		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C206
C207	207		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C207
C208.1	CE201C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C208.1
C208.2	203S		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C208.2
C208.3	203S		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B	14	C208.3
C209	C711C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B	14	C209
C210	CE201C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C210

A3 - DOOR SCHEDULE

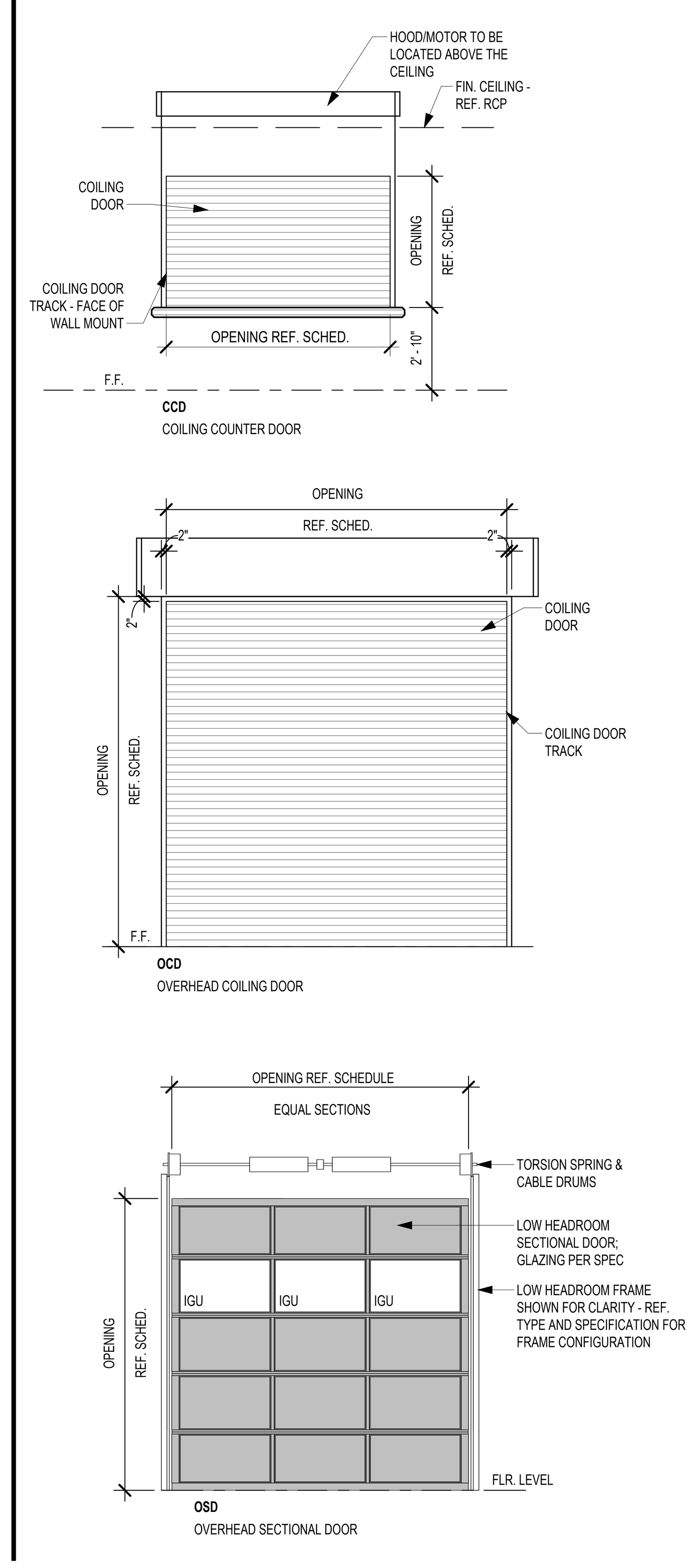
OPENING NUMBER	OPENING - HARDWARE SET	OPENING - FIRE RATING	OPENING - EXTERIOR	DOOR - WIDTH	DOOR - HEIGHT	DOOR - TYPE	DOOR - DR GLAZING	DOOR - PANIC HARDWARE	FRAME - MATERIAL	FRAME - FRAME TYPE	NOTES	OPENING NUMBER
C211	CE201C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C211
C212	C711C		No	3' - 0"	7' - 0"	SC-NL	GTC		HM	B		C212
AREA D												
D100.1	714AM		Yes	3' - 0"	7' - 0"	AL-SF (PR)	IGSE	Yes	AL	CW-06		D100.1
D100.2	714AM		Yes	3' - 0"	7' - 0"	AL-SF (PR)	IGSE	Yes	AL	CW-06		D100.2
D101.1	501		No	3' - 0"	7' - 0"	SC-FL	--		AL			D101.1
D101.2	501		No	3' - 0"	7' - 0"	SC-NL	--		AL			D101.2
D102	212S1		No	3' - 0"	7' - 0"	PL-NL (PR)	GTC		AL			D102
D103	203		No	3' - 0"	7' - 0"	SC-FL	--		AL			D103
AREA D - OVERHEAD												
SO-D101	001		No	10' - 0"	10' - 0"	OCD-SS			STL		MOTOR OPER.	SO-D101
AREA E												
E100.1	C714AM		Yes	3' - 0"	7' - 0"	AL-SF (PR)	IGSE	Yes	AL	SF-02	10	E100.1
E100.2	C714M		Yes	3' - 0"	7' - 0"	HM-NL (PR)	IGSE	Yes	HM	B1	10	E100.2
E100.3	201CR	60 MIN	No	3' - 0"	7' - 0"	HM-RL	GFPR		HM	B1		E100.3
E101	343S		No	3' - 0"	7' - 0"	SC-FL	--		HM	B1		E101
E102	343S		No	3' - 0"	7' - 0"	SC-FL	--		HM	B1		E102
E103	D205		Yes	3' - 0"	7' - 0"	HM-FL	--		HM	B1		E103
E104	711R	60 MIN	No	3' - 0"	7' - 0"	SC-FL	--		HM	B1		E104
E105	203W		No	3' - 6"	7' - 0"	SC-FL	--		HM	B1	9	E105
E106	CE201CR	60 MIN	No	3' - 0"	7' - 0"	SC-FL	--		HM	B1		E106
E107.1	D715		Yes	3' - 0"	7' - 0"	HM-NL	IGSE	Yes	HM	B1	10	E107.1
E107.2	227R	60 MIN	No	3' - 0"	7' - 0"	HM-RL	GFPR		HM	B1		E107.2
E108	201CR	60 MIN	No	3' - 0"	7' - 0"	SC-FL	--		HM	B1		E108
E110.1	805IL		Yes	3' - 0"	7' - 0"	HM-FL	--		HM	B1		E110.1
E110.2	805IL		Yes	3' - 0"	7' - 0"	HM-FL	--		HM	B1		E110.2
E111.1	805IL		Yes	3' - 0"	7' - 0"	HM-FL	--		HM	B1		E111.1
E111.2	805IL		Yes	3' - 0"	7' - 0"	HM-FL	--		HM	B1		E111.2
E112	PAD			8' - 0"	8' - 0"	CL GATE (PR)			CL			E112
E113	205		Yes	3' - 0"	7' - 0"	HM-NL	IGU	No	HM	B1		E113
E113.1	201CR		No	3' - 0"	7' - 0"	SC-NL	IGU		HM	B1		E113.1
AREA E - OVERHEAD												
SO-E100.1	D001		Yes	14' - 0"	14' - 0"	OSD	IGU		STL		MAN. OPER.	SO-E100.1
SO-E100.2	D001		Yes	14' - 0"	14' - 0"	OSD	IGU		STL		MAN. OPER.	SO-E100.2
SO-E107.1	D001		Yes	10' - 0"	10' - 0"	OCD-ST			STL		MAN. OPER.	SO-E107.1
SO-E107.2	001	60 MIN	No	10' - 0"	10' - 0"	OCD-FR			STL			SO-E107.2
SO-E113.1	D001		Yes	10' - 0"	4' - 8"	CCD-ST			STL			SO-E113.1
SO-E113.2	D001		Yes	10' - 0"	4' - 8"	CCD-ST			STL			SO-E113.2



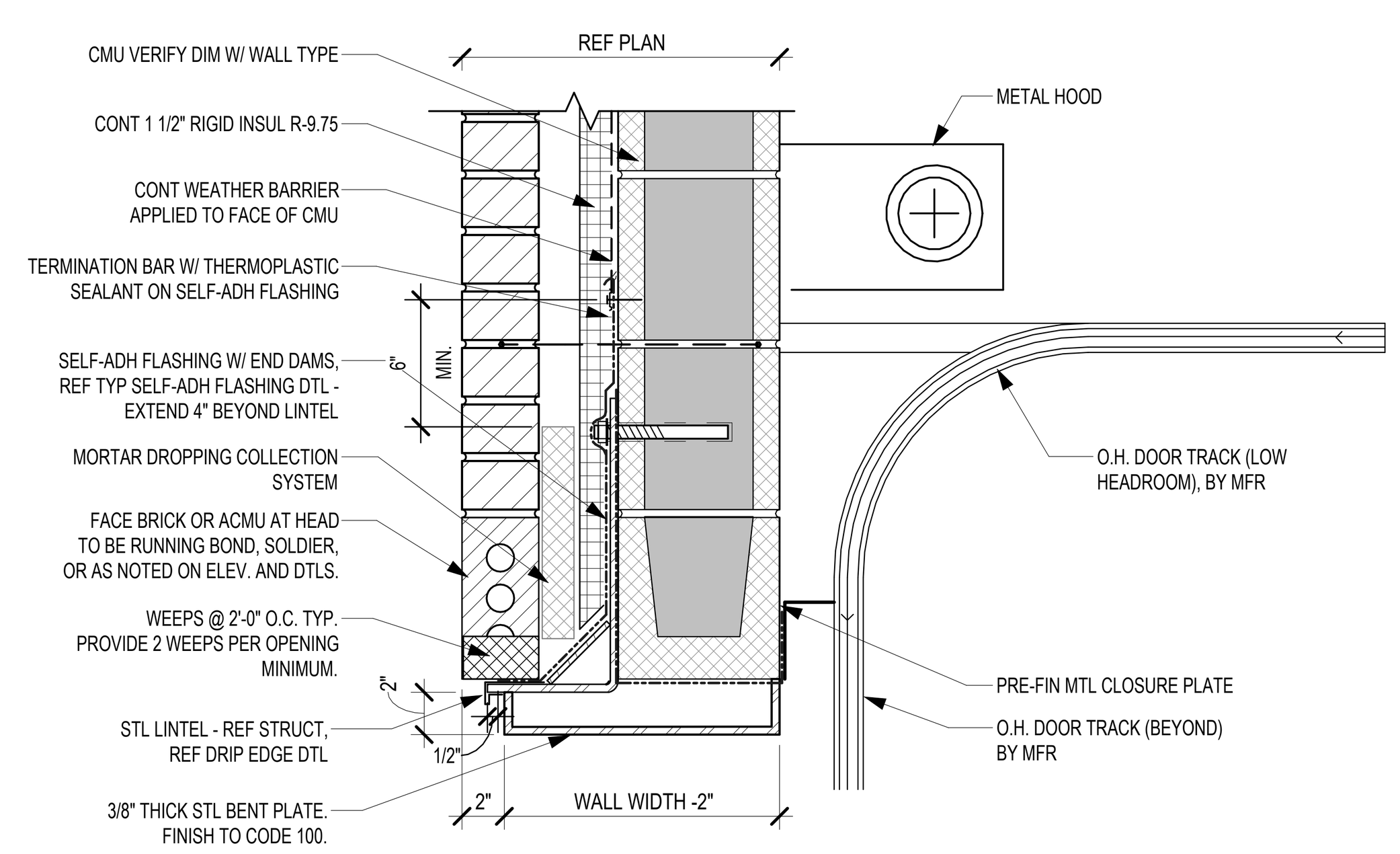
7 HEAD DTL - OCD - MTL STUD
1 1/2" = 1'-0"



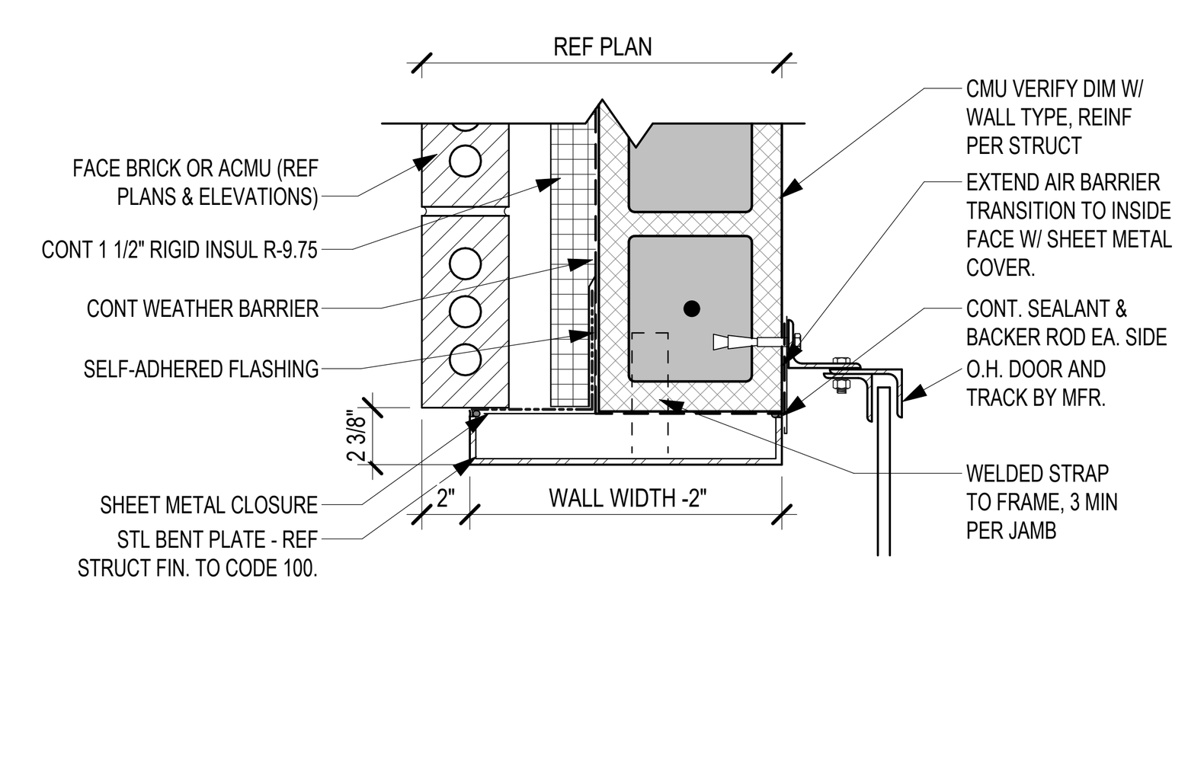
6 HEAD DTL - OCD - CMU
1 1/2" = 1'-0"



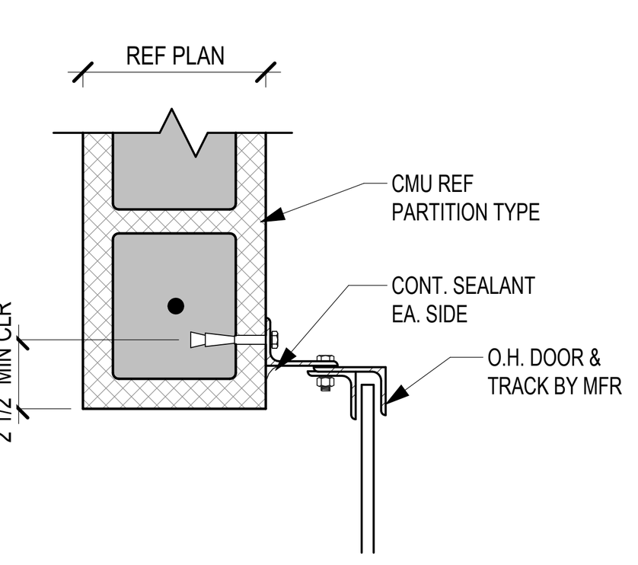
OVERHEAD DOOR CONFIGURATIONS



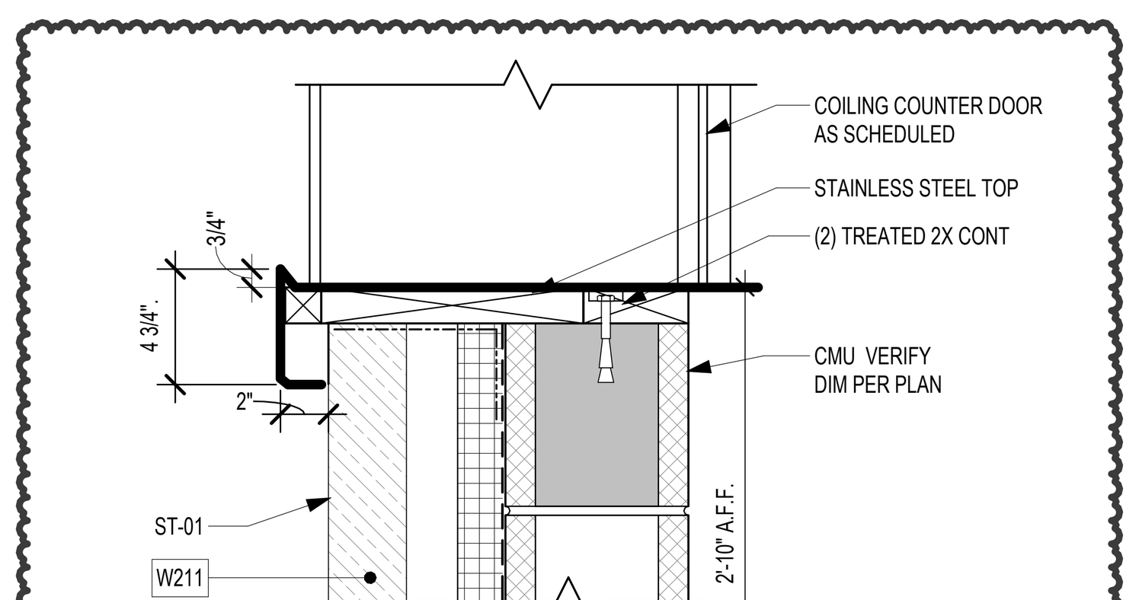
4 HEAD DTL - OSD - CMU / INSUL / MODULAR BRICK
1 1/2" = 1'-0"



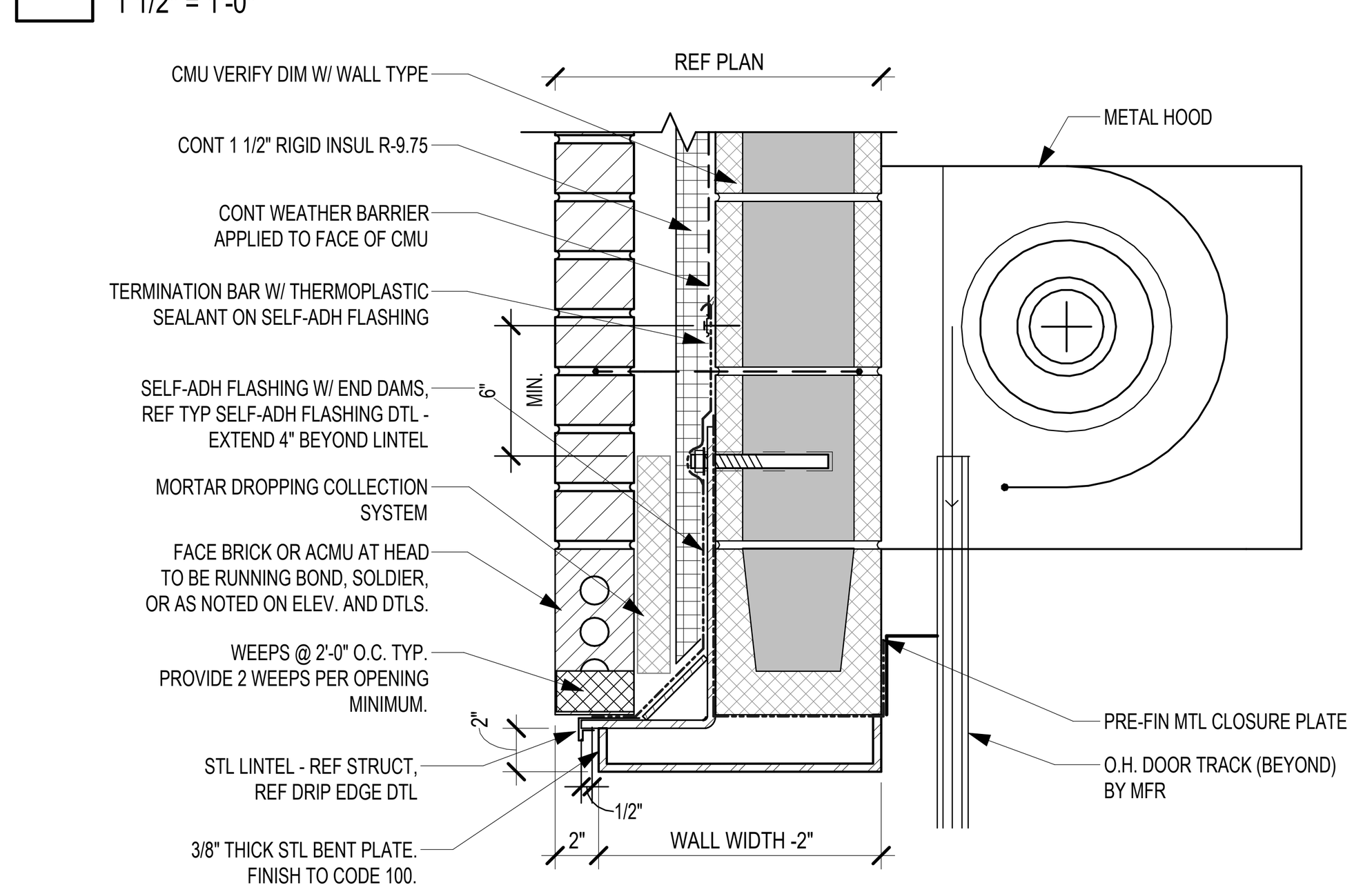
3 JAMB DTL - OCD - CMU / INSUL / MODULAR BRICK
1 1/2" = 1'-0"



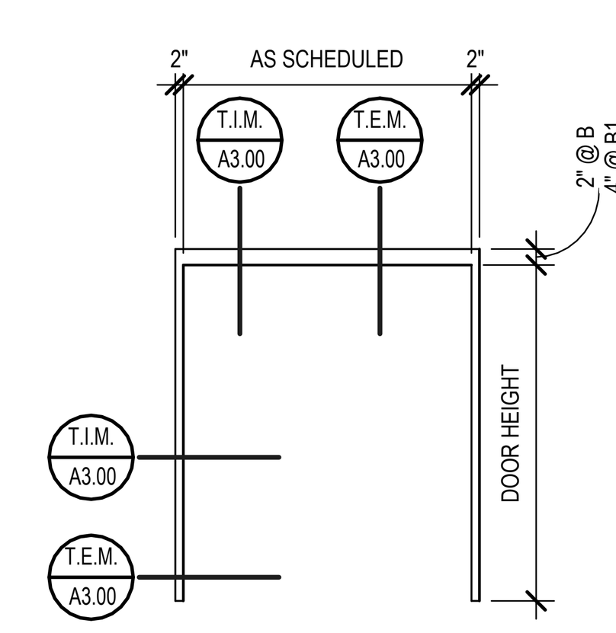
5 JAMB DTL - OCD - CMU
1 1/2" = 1'-0"



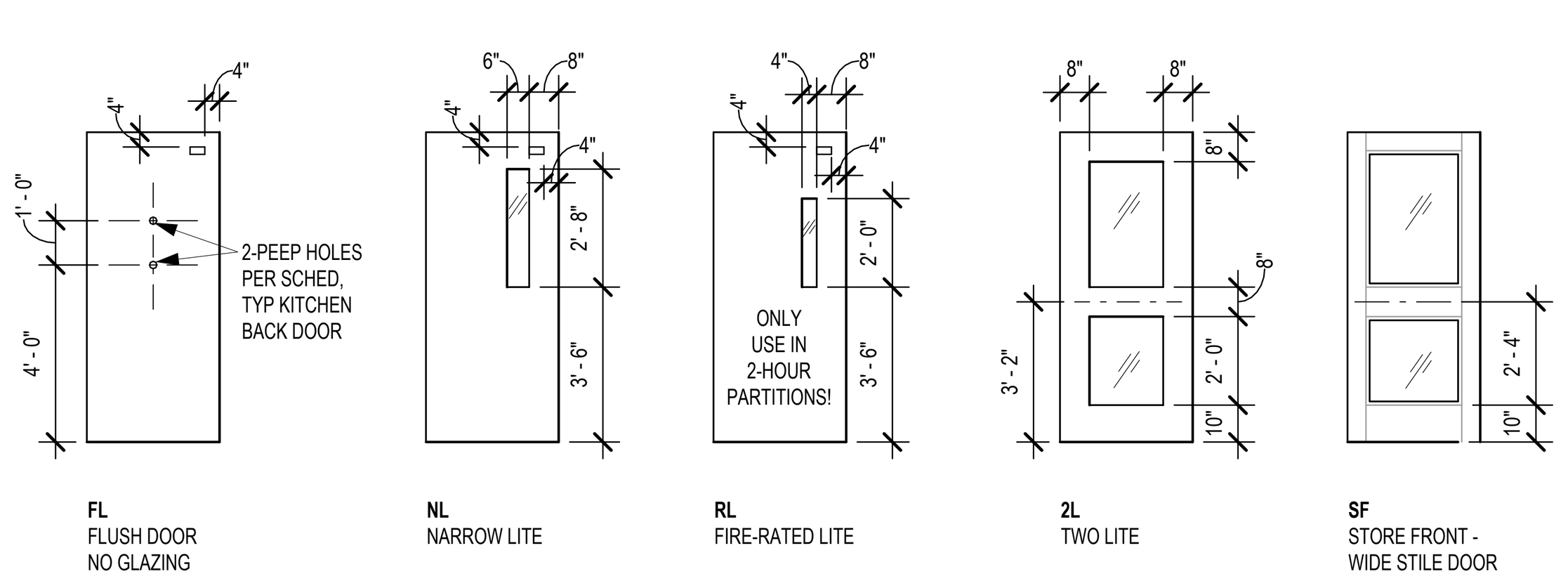
2 SILL DTL - CCD - SS
1 1/2" = 1'-0"



1 HEAD DTL - OCD - CMU / INSUL / MODULAR BRICK
1 1/2" = 1'-0"



B A3 - HM DOOR FRAME B
1/4" = 1'-0"



DOOR CONFIGURATIONS

DEFAULT DOORS

IF A DOOR ASSEMBLY APPEARS ON THE FLOOR PLANS BUT IS NOT IDENTIFIED BY A SCHEDULE NUMBER, THE DOOR SHALL BE INCLUDED IN THE BID AS LISTED BELOW. THE CONTRACTOR SHALL REQUEST CLARIFICATION FROM THE ARCHITECT TO DETERMINE WHAT THE ACTUAL DOOR AND FRAME TYPE AND HARDWARE REQUIREMENTS WILL BE. ADJUSTMENTS IN CONTRACT PRICE WILL BE BASED ON THE DIFFERENCE BETWEEN ACTUAL REQUIREMENTS AND THE DEFAULT DOOR VALUE.

THE DEFAULT EXTERIOR DOOR SHALL BE A TYPE "HM-FL" (SINGLE OR PAIR PER PLAN) IN A TYPE "B" ("B1" AT EXTERIOR MASONRY WALL) FRAME WITH DEFAULT HARDWARE PER SPECIFICATIONS AND FRAME DETAILING PER TYPICAL DETAILS FOR EXTERIOR DOORS.

THE DEFAULT INTERIOR DOOR SHALL BE A TYPE "SC-FL" (SINGLE OR PAIR PER PLAN) IN A TYPE "B" FRAME WITH DEFAULT HARDWARE PER SPECIFICATIONS AND FRAME DETAILING PER TYPICAL DETAILS FOR INTERIOR DOORS.

TYPICAL NOTES

A. DOOR TYPES ARE INDICATED AS A MATERIAL-CONFIGURATION CODE. THE MATERIAL CODE DESIGNATES MATERIALS AND FINISHES AS SPECIFIED. REFER TO SPECIAL NOTES FOR NON-TYPICAL FINISHES. THE CONFIGURATION CODE REFERS TO TYPICAL OR SPECIAL CONFIGURATIONS AS DRAWN.

B. DOOR ASSEMBLIES ARE NUMBERED BY ASSEMBLY. MULTIPLE SETS OF DOORS MAY OCCUR IN THE SAME FRAME ASSEMBLY (REFER TO SCHEDULE AND PLAN FOR QUANTITY). DOORS MAY OCCUR IN PAIRS (REFER TO SCHEDULE AND PLAN FOR INCIDENCE).

C. FRAME CONFIGURATION IS DIAGRAMMATIC AND NOMINAL. DIMENSIONS SHALL BE ADJUSTED TO ACCOMMODATE DOOR SIZES AS SCHEDULED. WITHIN ANY HOLLOW METAL ASSEMBLY, THE WIDTH OF SIDELITES SHALL BE EQUAL AND SHALL NOT EXCEED 3'-0" U.N.O.

D. ALTERNATE FINISHES CAN SOMETIMES HAVE AN IMPACT ON DETAILING (E.G. HALF OR OFFSET SADDLES EMPLOYED IN LIEU OF STANDARD SADDLES WHEN ADJACENT TO THICKER FLOOR FINISHES SUCH AS TERRAZZO OR QUARRY TILE). COORDINATE DETAILS WITH FINAL FINISH SELECTIONS.

E. GLAZED FRAMES ASSOCIATED WITH 20-MINUTE, 45-MINUTE, AND/OR 60-MINUTE FIRE-RATED DOORS SHALL BE FIRE-RATED 1-HOUR. GLAZED FRAMES ASSOCIATED WITH 90-MINUTE FIRE-RATED DOORS SHALL BE FIRE-RATED 2-HOUR. GLAZING MUST MEET CODE REQUIREMENTS, INCLUDING SIZE LIMITATIONS, IN FIRE-RATED FRAMES. REF. IBC TABLE 716.1 (2).

F. PROVIDE 2x WOOD BLOCKING @ CFMF STUD WALLS FOR WALL MOUNTED ITEMS LIKE WALL STOPS, HOLD OPENS, ETC.

G. REFER TO FLOOR PLANS AND EXTERIOR ELEVATIONS FOR ALUMINUM FRAME LOCATIONS.

H. OVERHEAD COILING DOORS ARE MOTORIZED UNLESS NOTED OTHERWISE.

SPECIAL NOTES:

1. MAGNETIC HOLD OPEN - REF. FIRE ALARM SHEETS.
2. MANUAL HOLD OPEN.
3. SOUND CONTROL DOOR ASSEMBLY STC50.
4. SOUND SEAL W/ DOOR BOTTOM STC 40-42.
5. DOOR & FRAME PROVIDED AS PART OF SOUND-ISOLATION ROOM.
6. PROVIDE (2) PEEPHOLES IN DOOR PANEL: (1) LOCATED AT STANDARD HEIGHT AND (1) AT 42" AFF.
7. TORNADO RESISTANT DOOR AND FRAME ASSEMBLY.
8. NO LATCH, PUSH OPEN.
9. NO CLOSER.
10. PANIC BUTTON EMERGENCY OPEN.
11. FAIL SAFE IN OPEN POSITION.
12. FAIL SAFE IN CLOSED POSITION.
13. MANUAL OPERATION.
14. SMOKE DOOR ASSEMBLY COMPLYING WITH NFPA 105. PROVIDE SMOKE SEALS.

DOOR ASSEMBLY NOTES

AL - ALUMINUM

HM - HOLLOW METAL

SC - SOLID CORE WOOD

CL - CHAIN LINK (VINYL-COATED)

DMF - DECORATIVE METAL FENCING

OCD & OCD-FR - OVERHEAD COILING DOOR (FIRE RATED)

OSD - OVERHEAD SECTIONAL DOOR

CCD - COUNTERTOP COILING DOOR

DOOR/FRAME MATERIALS

IGLSA - INSULATED GLASS LAMINATED SAFETY

IGS - INSULATED GLASS SPANDREL

IGSE - INSULATED GLASS SECURITY

IGU - INSULATED GLASS UNIT

GFPR - GLASS FIRE-PROTECTION-RATED

GTC - GLASS TEMPERED CLEAR

MIR - UNFRAMED MIRROR

SCWA - SOUND CONTROL WINDOW ASSEMBLY

1. ALL GLAZING IS 1/4" THICK (U.N.O.). INSULATED GLASS SHALL CONSIST OF AN EXTERIOR LAYER OF GLASS, TINTED AS SPECIFIED, AN INTERIOR LAYER OF CLEAR GLASS, FACTORY SEALED WITH A 1/2" AIR SPACE. WHERE TEMPERED GLASS IS SPECIFIED, BOTH LAYERS OF INSULATED GLASS SHALL BE TEMPERED.
2. ANY WINDOW OR GLAZING PANELS LOCATED AT A INTERSTITIAL SPACE OR NOT READILY VISIBLE SHALL HAVE SPANDREL GLASS INSTALLED, REGARDLESS OF ANY NOTATION ELSEWHERE.

GLAZING LEGEND

FR - FIRE RATED

ST - STRUCTURE

TEM - TYP EXT MASONRY DETAIL - HM

TEMA- TYP EXT MASONRY DETAIL - ALUM

TICF - TYP EXT INSUL CONC FORMS - HM

TICFA - TYP EXT INSUL CONC FORMS - ALUM

TIM - TYP INTERIOR MASONRY DETAIL - HM

TIMA - TYP INTERIOR MASONRY DETAIL - ALUM

TDW - TYP DRYWALL PARTITION DETAIL - HM

TDWA- TYP DRYWALL PARTITION DETAIL - ALUM

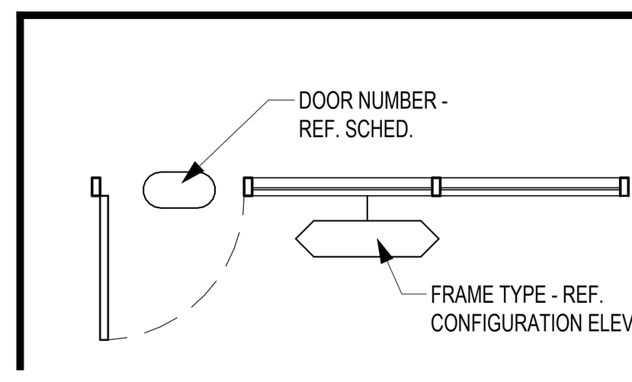
HO - DOOR HOLD OPEN (MAGNETIC OR MANUAL - SEE SPECIAL NOTES FOR MORE INFO)

TH - THRESHOLD

SEC - SECURITY CONTACTS (REF. ELEC. AND TECH.)

OH - DENOTES OPPOSITE HAND CONFIGURATION

SCHEDULE ABBREVIATIONS



STOREFRONT & CURTAIN WALL DR FRAMES

ADDENDUM 01

Revision / Date
1 05/08/25

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

05.08.25

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DOOR AND SPECIAL OPENING SCHEDULES AND CONFIGURATIONS

PACKAGE VOLUME

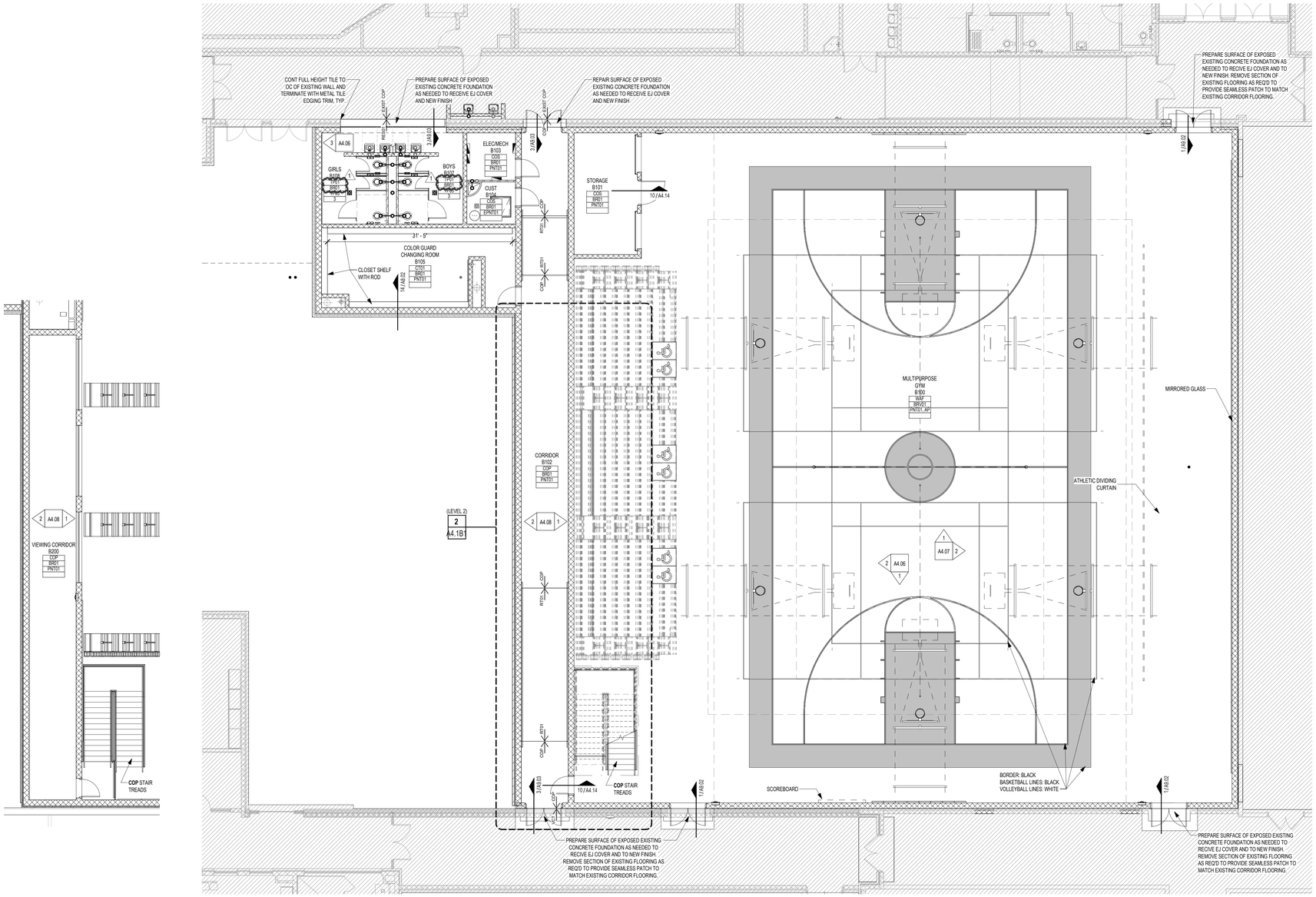
Job No. 01954-08-01

Drawn By: VSS

Date: 05/08/2025

Sheet No. ISSUE FOR BID

A3.01



2 FINISH PLAN - AREA B - LEVEL 2
1/8" = 1'-0"

1 FINISH PLAN - AREA B - LEVEL 1
1/8" = 1'-0"

FINISH PLAN LEGEND

- ELEVATION REFERENCE
- CALLOUT REFERENCE
- WALL SECTION REFERENCE
- B - HORIZONTAL BLINDS
  SMB - MOTORIZED SHADES - BLACKOUT
  SB - WINDOW SHADES - BLACKOUT
  SM - MOTORIZED SHADES
  S - WINDOW SHADES
- FLOOR INSTALLATION DIRECTION
- FLOOR TRANSITION
- WALL FINISH TAG
- EQUIPMENT TAG

FINISH REMARKS

APPLIES TO SECTION PLANS

NOTES APPLY TO ROOMS AS NOTED IN THE ROOM FINISH TAGS. SEE TAG LAYOUT ABOVE.

1. PROVIDE CEILING MOUNT PERIMETER NETTING BEHIND GOAL POSTS. REF. RCP.
2. PROVIDE A GOAL POST AT BOTH ENDS.
3. PROVIDE FULL HEIGHT WALL TILE ON ALL WALLS. START FULL TILE ABOVE RESINOUS BASE.
4. REFER TO INTERIOR ELEVATIONS FOR TYPICAL RESTROOM WALL FINISHES.
5. NEW FLOORING TO TIE INTO EXISTING PATTERN SEAMLESSLY - FULL TILE TO FULL TILE.
6. ALL EXISTING WALL APPLICATIONS SUCH AS ACOUSTICAL PANELS, MARKERBOARDS, AND TACKBOARDS, ARE TO BE INSTALLED IN THE SAME LOCATION.
7. REF SPORTS FOR FLOOR SURFACE MATERIAL WITHIN PERIMETER OF FENCE.
8. REF ELEVATIONS FOR ADDITIONAL INFORMATION
9. PROVIDE FRP FINISH TO 4' - 4" AFF.
10. REF FOOD SERVICE FOR KITCHEN EQUIP TYP. TRAY SLIDES SHALL BE STAINLESS STEEL. SERVING LINE FRONT PANELS SHALL BE PL01.

FINISH PLAN GENERAL NOTES

A. ALL DIMENSIONS SHOW ON FINISH FLOOR PLAN DRAWINGS ARE FROM FACE OF FINISH, U.N.O.

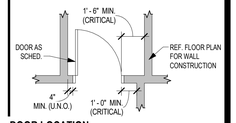
B. INTERIOR ACCESS PANELS TO MATCH ADJACENT FINISH. COORDINATE WITH ARCHITECT.

C. PROVIDE 3/8" MIN SEALANT JOINT WHERE FULL HT TILE MEETS GYP CLG (MATCH CLG COLOR) OR LAY-IN GRID (WHITE)

D. PROVIDE SEALANT JT'S @ INSIDE CORNERS OF TILED WALLS

E. VERIFY FINISH PATTERNS WITH SCHEDULE OF MATERIALS.

ALL SCHEDULED WALL FINISH DIRECTIONS ARE DIRECTIONS PER PLAN NORTH OF THE FINISH PLANS, NOT TRUE NORTH OF THE PROJECT SITE



DOOR LOCATION

REFER TO G5.01 FOR ADDITIONAL INFORMATION REGARDING FINISH MATERIALS AND PAINT FINISH CODES.

CLASSROOM ← ROOM NAME
025 ← ROOM NUMBER

TYP FLOOR FINISH	LVT01
WALL BASE FINISH	BR01
TYP WALL FINISH	PNT01
REMARKS	1

*REMARKS NUMBERS COORDINATED WITH FINISH REMARKS LEGEND LOCATED ON ALL FINISH FLOOR PLAN SHEETS

REFER TO PROJECT SPECIFICATIONS SECTION 01 6210 FOR ADDITIONAL INFORMATION REGARDING FINISH MATERIALS AND PAINT FINISH CODES.

XY ## — XY: MATERIAL TYPE; ##: FINISH, COLOR AND MATERIAL SIZE AS NOTED IN SPEC. SECTION 01 6210

ACMU - ARCHITECTURAL CONCRETE MASONRY UNIT

AL - ALUMINUM

AP - ACOUSTICAL PANELS

ATS - ATHLETIC TURF, SYNTHETIC

BP - BASE, PORCELAIN TILE

BRES - BASE, RESINOUS INTEGRAL

BRK - BRICK

BR - BASE, RUBBER

BRV - BASE, RUBBER VENTED

BTC - BASE, TILE, CERAMIC

BTZ - BASE, TERRAZZO PRECAST

BWD - BASE, WOOD

CCPS - CERAMIC CLADDING PANEL SYSTEM

CDAP - CUSTOM DIGITALLY PRINTED ACOUSTIC PANEL

CDW - CUSTOM DIGITAL WALLCOVERINGS

COP - CONCRETE POLISHED

COS - CONCRETE SEALED

CPT - CARPET, SHEET

CS - CAST STONE

CSM - CONCRETE STONE MASONRY

CT - CARPET, TILED

CWF - CUSTOM WINDOW FILM

EPNT - EPOXY PAINT

FRP - FIBERGLASS REINFORCED PLASTIC PANEL SYSTEM

GFRC - GLASS FIBER REINFORCED CONCRETE

GRF - GROUND FACE CMU (BURNISHED)

LVT - LUXURY VINYL TILE

MB - MARKER BOARD

MCM - METAL COMPOSITE MATERIAL

MP - METAL WALL PANEL

MS - METAL SOFFIT

MTL - METAL WAINSCOT

NB - NO BASE

NS - NATURAL STONE

PL - PLASTIC LAMINATE

PLP - PLASTIC LAMINATE PANEL SYSTEM

PNT - PAINT

QTZ - QUARTZ SURFACING

RAFF - RESILIENT ATHLETIC FLUID FLOORING

RAFP - RESILIENT ATHLETIC FLOORING POLYURETHANE

RAFS - RUBBER FLOORING SHEET

RAFV - RESILIENT ATHLETIC FLOORING VINYL

RES - RESINOUS FLOORING

RT - RUBBER TILE

RTR - RUBBER TREADS AND RISERS

SPEC - SPECIAL FINISHES, REF. ROOM FINISH SCHEDULE & INTERIOR ELEVATIONS

SPF - SPLIT-FACE CONCRETE MASONRY UNIT

SSM - SOLID SURFACE MATERIAL

TB - TACK BOARD

TC - TILE, CERAMIC

TP - TILE, PORCELAIN

TQ - TILE, QUARRY

TS - TACKABLE SURFACE

TZ - TERRAZZO

TZP - TERRAZZO, PRECAST STAIR TREADS AND RISERS

TZT - TERRAZZO TILE

VCT - TILE, VINYL COMPOSITION

VSF - VINYL SHEET FLOORING

WAF - ATHLETIC WOOD FLOORING

WMP - WOOD (MAPLE) STRIP AND PLANK FLOORING

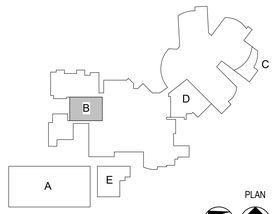
WMS - MASONITE WOOD FLOORING

WOM - WALK-OFF MAT

WOT - WALK-OFF TILE

(NOT ALL ABBREVIATIONS MAY BE USED)

FINISH KEY



KEY PLAN
NTS

PLAN TRUE N

Revision /	Date
1	05/08/25 ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

05.08.25

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FINISH PLAN - AREA B

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A4.1B1
Drawn By: YR/AB	
Date: 05/08/2025	

1 FINISH PLAN - AREA C - LEVEL 1
1/8" = 1'-0"

FINISH PLAN LEGEND

A1.1 1 — ELEVATION REFERENCE

1 / A1.1 — CALLOUT REFERENCE

SIM 1 / A101 — WALL SECTION REFERENCE

B
- B - HORIZONTAL BLINDS
- SMB - MOTORIZED SHADES - BLACKOUT
- SB - WINDOW SHADES - BLACKOUT
- SM - MOTORIZED SHADES
- S - WINDOW SHADES

FLOOR INSTALLATION DIRECTION

FINISH — WALL FINISH TAG

5' MB — EQUIPMENT TAG

FINISH REMARKS APPLIES TO SECTION PLANS

NOTES APPLY TO ROOMS AS NOTED IN THE ROOM FINISH TAGS. SEE TAG LAYOUT ABOVE.

1. PROVIDE CEILING MOUNT PERIMETER NETTING BEHIND GOAL POSTS. REF. RCP.
2. PROVIDE A GOAL POST AT BOTH ENDS.
3. PROVIDE FULL HEIGHT WALL TILE ON ALL WALLS. START FULL TILE ABOVE RESINOUS BASE.
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6. ALL EXISTING WALL APPLICATIONS SUCH AS ACOUSTICAL PANELS, MARKERBOARDS, AND TACKBOARDS, ARE TO BE INSTALLED IN THE SAME LOCATION.
7. REF SPORTS FOR FLOOR SURFACE MATERIAL WITHIN PERIMETER OF FENCE.
8. REF ELEVATIONS FOR ADDITIONAL INFORMATION
9. PROVIDE FRP FINISH TO 4' - 4" AFF.
10. REF FOOD SERVICE FOR KITCHEN EQUIP TYP. TRAY SLIDES SHALL BE STAINLESS STEEL. SERVING LINE FRONT PANELS SHALL BE PLOT.

FINISH PLAN GENERAL NOTES

A. ALL DIMENSIONS SHOW ON FINISH FLOOR PLAN DRAWINGS ARE FROM FACE OF FINISH, U.N.O.

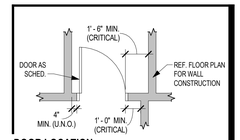
B. INTERIOR ACCESS PANELS TO MATCH ADJACENT FINISH. COORDINATE WITH ARCHITECT.

C. PROVIDE 3/8" MIN SEALANT JOINT WHERE FULL HT TILE MEETS GYP CLG (MATCH CLG COLOR) OR LAY-IN GRID (WHITE)

D. PROVIDE SEALANT JT'S @ INSIDE CORNERS OF TILED WALLS

E. VERIFY FINISH PATTERNS WITH SCHEDULE OF MATERIALS.

ALL SCHEDULED WALL FINISH DIRECTIONS ARE DIRECTIONS PER PLAN NORTH OF THE FINISH PLANS, NOT TRUE NORTH OF THE PROJECT SITE



DOOR LOCATION

REFER TO G5.01 FOR ADDITIONAL INFORMATION REGARDING FINISH MATERIALS AND PAINT FINISH CODES.

CLASSROOM 025 ← ROOM NAME / ROOM NUMBER

TYP FLOOR FINISH	LVT01
WALL BASE FINISH	BR01
TYP WALL FINISH	PNT01
REMARKS	1

*REMARKS NUMBERS COORDINATED WITH FINISH REMARKS LEGEND LOCATED ON ALL FINISH FLOOR PLAN SHEETS

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BRK - BRICK

BR - BASE, RUBBER

BRV - BASE, RUBBER VENTED

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BTZ - BASE, TERRAZZO PRECAST

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FRP - FIBERGLASS REINFORCED PLASTIC PANEL SYSTEM

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LVT - LUXURY VINYL TILE

MB - MARKER BOARD

MCM - METAL COMPOSITE MATERIAL

MP - METAL WALL PANEL

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MTL - METAL WAINSCOT

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RAFP - RESILIENT ATHLETIC FLOORING POLYURETHANE

RAFS - RUBBER FLOORING SHEET

RAFV - RESILIENT ATHLETIC FLOORING VINYL

RES - RESINOUS FLOORING

RT - RUBBER TILE

RTR - RUBBER TREADS AND RISERS

SPEC - SPECIAL FINISHES, REF. ROOM FINISH SCHEDULE & INTERIOR ELEVATIONS

SPF - SPLIT-FACE CONCRETE MASONRY UNIT

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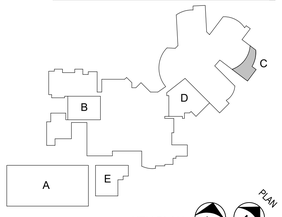

KEY PLAN NTS

N TRUE

N PLAN

Revision /	Date
1	05/08/25 ADDENDUM 01

Project: LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS FOR HAYS CISD
KYLE, TX

05.08.25

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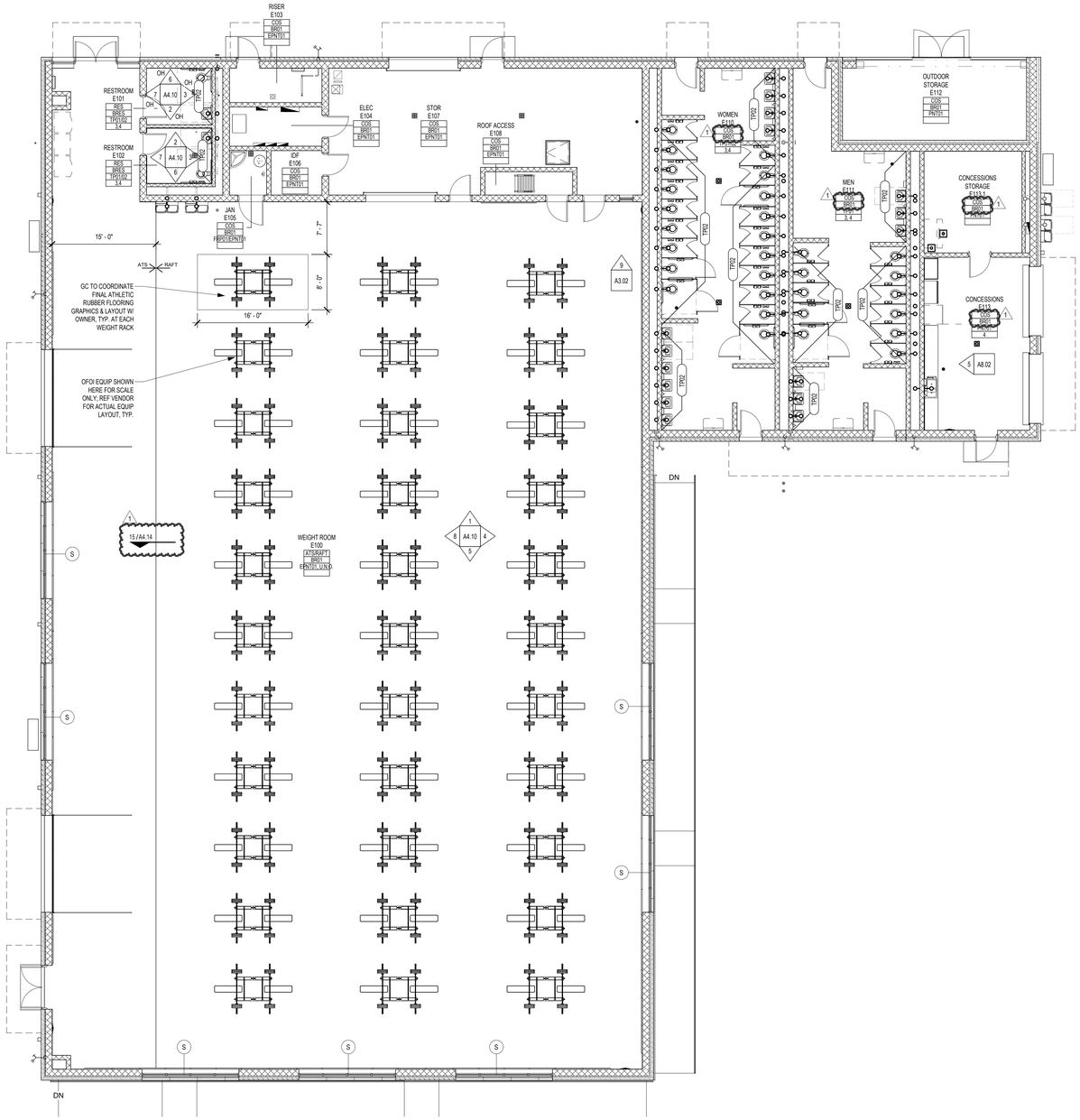
FINISH PLAN - AREA C - LEVEL 1

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A4.1C1
Drawn By: YR/AB	
Date: 05/08/2025	



1 FINISH PLAN - AREA E
1/8" = 1'-0"

FINISH PLAN LEGEND

- A1.1 / 1 — ELEVATION REFERENCE
- 1 / A1.1 — CALLOUT REFERENCE
- 1 / A101 SIM — WALL SECTION REFERENCE
- B — B - HORIZONTAL BLINDS; SMB - MOTORIZED SHADES - BLACKOUT; SB - WINDOW SHADES - BLACKOUT; SM - MOTORIZED SHADES; S - WINDOW SHADES
- FLOOR INSTALLATION DIRECTION
- FLOOR TRANSITION
- FINISH — WALL FINISH TAG
- 8' MB — EQUIPMENT TAG

FINISH REMARKS

APPLIES TO SECTION PLANS

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8. REF ELEVATIONS FOR ADDITIONAL INFORMATION
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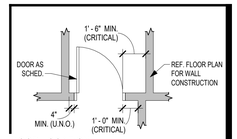
D. PROVIDE SEALANT JT'S @ INSIDE CORNERS OF TILED WALLS

E. VERIFY FINISH PATTERNS WITH SCHEDULE OF MATERIALS.

ALL SCHEDULED WALL FINISH DIRECTIONS ARE DIRECTIONS PER PLAN NORTH OF THE FINISH PLANS, NOT TRUE NORTH OF THE PROJECT SITE



DOOR LOCATION

REFER TO G5.01 FOR ADDITIONAL INFORMATION REGARDING FINISH MATERIALS AND PAINT FINISH CODES.

CLASSROOM — ROOM NAME
025 — ROOM NUMBER
TYP FLOOR FINISH — LVT01
WALL BASE FINISH — BR01
TYP WALL FINISH — PNT01
REMARKS — 1

*REMARKS NUMBERS COORDINATED WITH FINISH REMARKS LEGEND LOCATED ON ALL FINISH FLOOR PLAN SHEETS

REFER TO PROJECT SPECIFICATIONS SECTION 01 6210 FOR ADDITIONAL INFORMATION REGARDING FINISH MATERIALS AND PAINT FINISH CODES.

XY — MATERIAL TYPE
— FINISH COLOR AND MATERIAL SIZE AS NOTED IN SPEC. SECTION 01 6210

ACMU - ARCHITECTURAL CONCRETE MASONRY UNIT

AL - ALUMINUM

AP - ACOUSTICAL PANELS

ATS - ATHLETIC TURF, SYNTHETIC

BP - BASE, PORCELAIN TILE

BRES - BASE, RESINOUS INTEGRAL

BRK - BRICK

BR - BASE, RUBBER

BRV - BASE, RUBBER VENTED

BTC - BASE, TILE, CERAMIC

BTZ - BASE, TERRAZZO PRECAST

BWD - BASE, WOOD

CCPS - CERAMIC CLADDING PANEL SYSTEM

CDAP - CUSTOM DIGITALLY PRINTED ACOUSTIC PANEL

COW - CUSTOM DIGITAL WALLCOVERINGS

COP - CONCRETE POLISHED

COS - CONCRETE SEALED

CPT - CARPET, SHEET

CS - CAST STONE

CSM - CONCRETE STONE MASONRY

CT - CARPET, TILED

CWF - CUSTOM WINDOW FILM

EPNT - EPOXY PAINT

FRP - FIBERGLASS REINFORCED PLASTIC PANEL SYSTEM

GFRC - GLASS FIBER REINFORCED CONCRETE

GRF - GROUND FACE CMU (BURNISHED)

LVT - LUXURY VINYL TILE

MB - MARKER BOARD

MCM - METAL COMPOSITE MATERIAL

MP - METAL WALL PANEL

MS - METAL SOFFIT

MTL - METAL WAINSCOT

NB - NO BASE

NS - NATURAL STONE

PL - PLASTIC LAMINATE

PLP - PLASTIC LAMINATE PANEL SYSTEM

PNT - PAINT

QTZ - QUARTZ SURFACING

RAFF - RESILIENT ATHLETIC FLUID FLOORING

RAFP - RESILIENT ATHLETIC FLOORING POLYURETHANE

RAFS - RUBBER FLOORING SHEET

RAFV - RESILIENT ATHLETIC FLOORING VINYL

RES - RESINOUS FLOORING

RT - RUBBER TILE

RTR - RUBBER TREADS AND RISERS

SPEC - SPECIAL FINISHES, REF. ROOM FINISH SCHEDULE & INTERIOR ELEVATIONS

SPF - SPLIT-FACE CONCRETE MASONRY UNIT

SSM - SOLID SURFACE MATERIAL

TB - TACK BOARD

TC - TILE, CERAMIC

TP - TILE, PORCELAIN

TQ - TILE, QUARRY

TS - TACKABLE SURFACE

TZ - TERRAZZO

TZP - TERRAZZO, PRECAST STAIR TREADS AND RISERS

TZT - TERRAZZO TILE

VCT - TILE, VINYL COMPOSITION

VSF - VINYL SHEET FLOORING

WAF - athletic WOOD FLOORING

WMP - WOOD (MAPLE) STRIP AND PLANK FLOORING

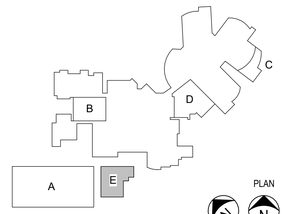
WMS - MASONITE WOOD FLOORING

WOM - WALK-OFF MAT

WOT - WALK-OFF TILE

(NOT ALL ABBREVIATIONS MAY BE USED)

FINISH KEY



KEY PLAN
NTS

PLAN TRUE NORTH

Revision /	Date	
1	05/08/25	ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT — JAMES JARON MARSH — 29417 — STATE OF TEXAS

05.08.25

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800.687.1229

FINISH PLAN - AREA E

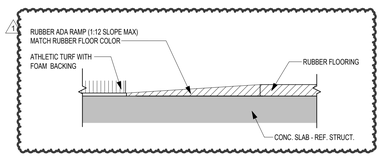
PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A4.1E1
Drawn By: YR/AB	
Date: 05/08/2025	

5/8/2025 4:45:19 PM
Autodesk Docs://HAYS CISD - LEHMAN, JOHNSON HS ADD RENO 2025/01954-08-01_LHS2025_V23.rvt

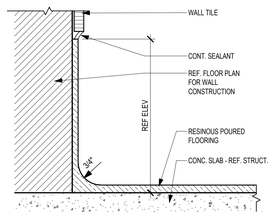
Revision /	Date	
1	05/08/25	ADDENDUM 01

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

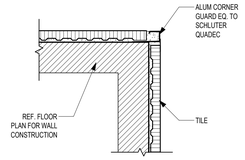
Project:



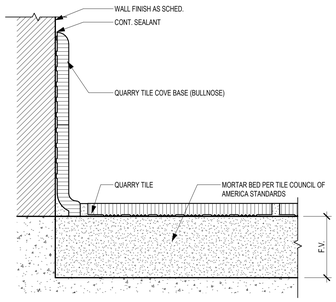
15 TRANSITION - TURF TO RUBBER
3" = 1'-0"



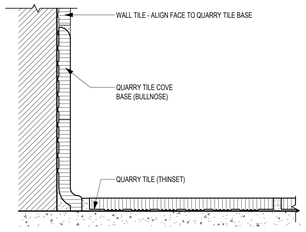
14 TYP RESINOUS COVE BASE W/ TILE
6" = 1'-0"



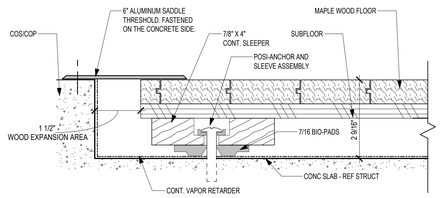
13 TYP TILE CORNER DETAIL
6" = 1'-0"



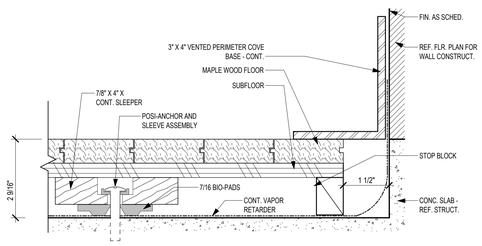
12 QUARRY TILE BASE - THICK SET (AT EXISTING MORTAR BED LOCATIONS)
6" = 1'-0"



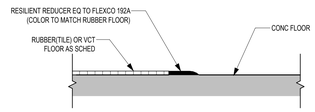
11 TYP QUARRY TILE BASE
6" = 1'-0"



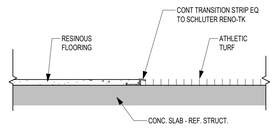
10 TRANSITION - CONCRETE TO WD GYM FLOOR
6" = 1'-0"



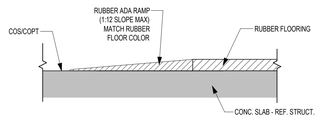
9 TYPICAL GYM FLOOR W/ RUBBER BASE
6" = 1'-0"



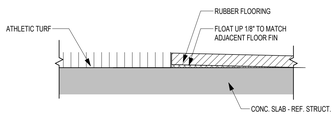
8 TRANSITION - RESILIENT TO CONCRETE
6" = 1'-0"



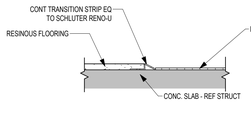
7 TRANSITION - TURF TO RESINOUS
6" = 1'-0"



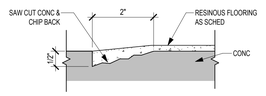
6 TRANSITION - RUBBER(ATHLETIC) TO CONC
3" = 1'-0"



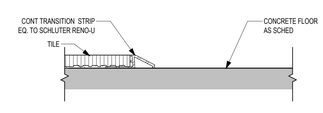
5 TRANSITION - RUBBER(ATHLETIC) TO TURF
3" = 1'-0"



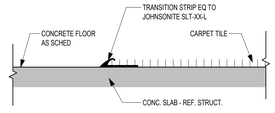
4 TRANSITION - RESINOUS TO RESILIENT
6" = 1'-0"



3 TRANSITION RESINOUS FLUSH TO CONC
6" = 1'-0"



2 TRANSITION - TILE TO CONCRETE
6" = 1'-0"



1 TRANSITION - CARPET TO CONCRETE
6" = 1'-0"

05.08.25

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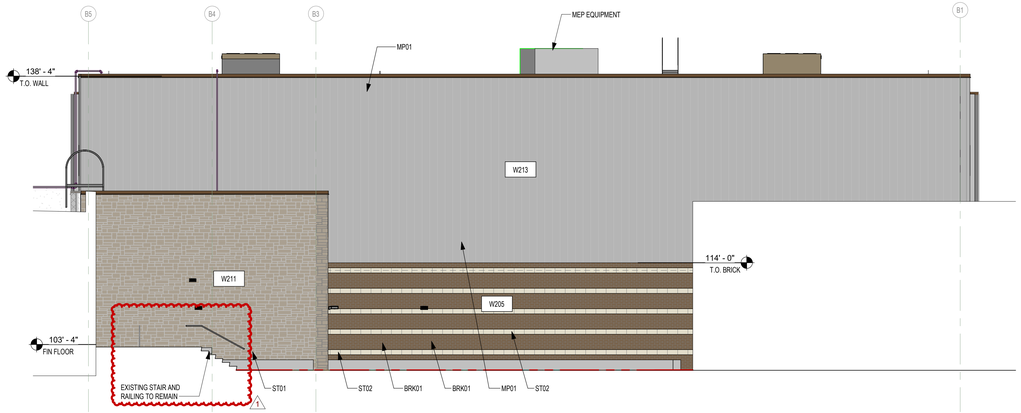
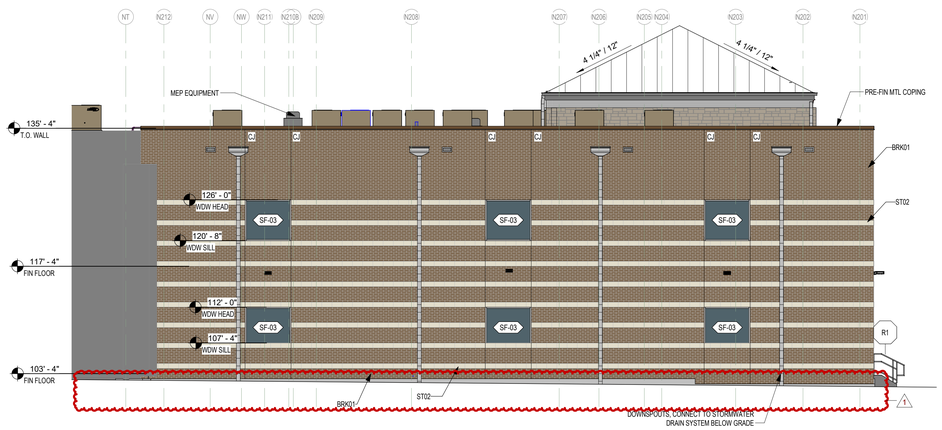
TYPICAL INTERIOR FINISH DETAILS

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A4.14
Drawn By: YR/AB	
Date: 05/08/2025	

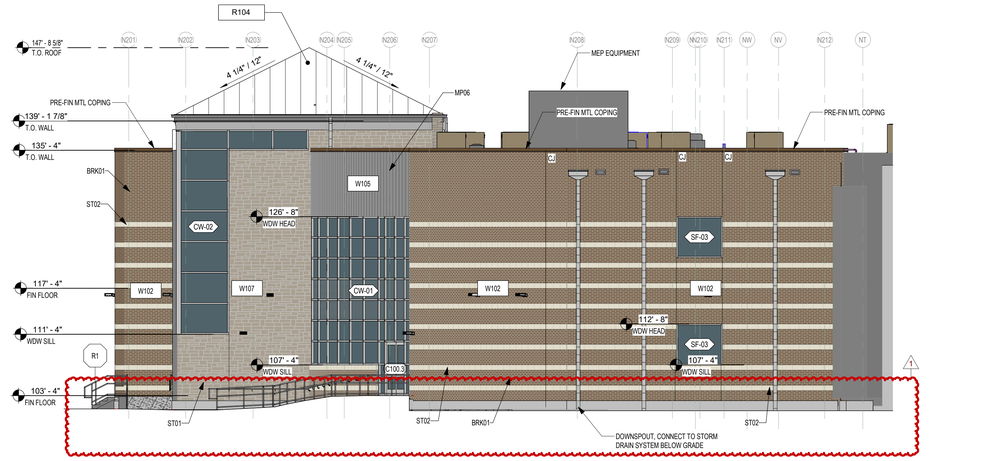
5/8/2025 4:43:29 PM
Autodesk Docs://HAYS CISD - LEHMAN, JOHNSON HS ADD RENO 2025/01954-08-01_LHS2025_V23.rvt



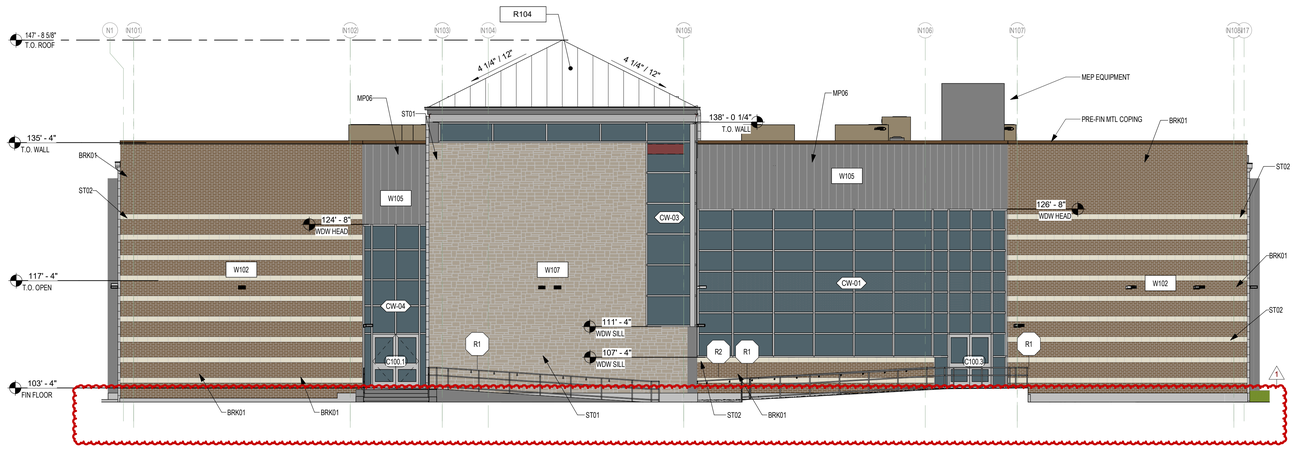
4 EXTERIOR ELEVATION - AREA B - WEST
1/8" = 1'-0"



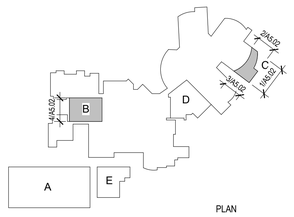
3 EXTERIOR ELEVATION - AREA C - SOUTH WEST
1/8" = 1'-0"



2 EXTERIOR ELEVATION - AREA C - NORTH EAST
1/8" = 1'-0"



1 EXTERIOR ELEVATION - AREA C - SOUTH EAST
1/8" = 1'-0"



Revision /	Date	
1	05/08/25	ADDENDUM 01

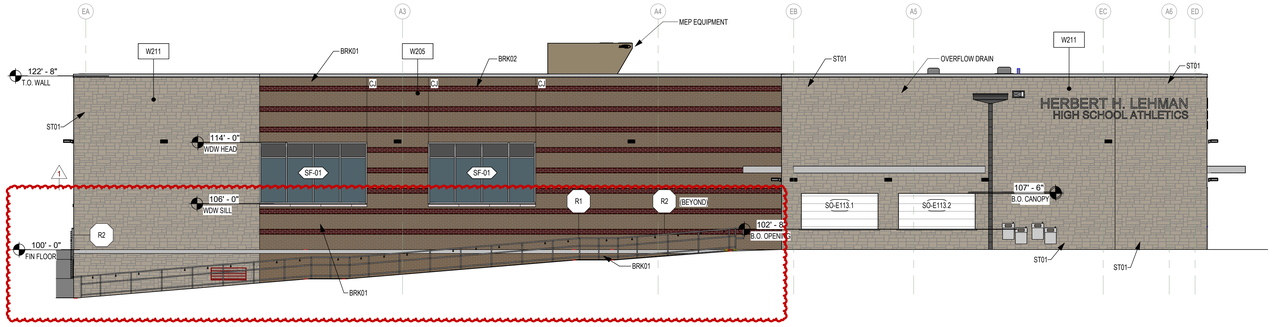
Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT
JAMES JASON HUCKABEE
19417
STATE OF TEXAS
05.08.25

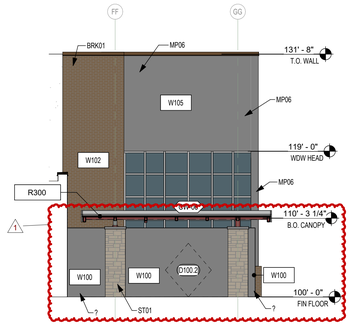
Huckabee
www.huckabee-inc.com
800.687.1229

EXTERIOR ELEVATIONS - AREA B & C

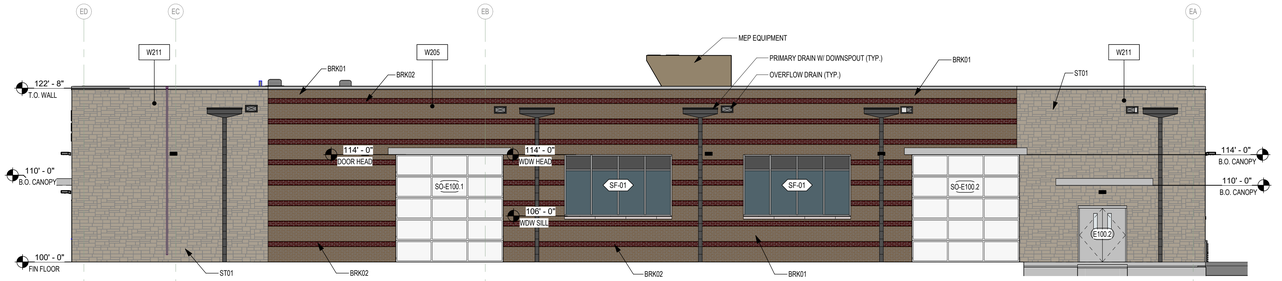
PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A5.02
Drawn By: YR/AB	
Date: 05/08/2025	



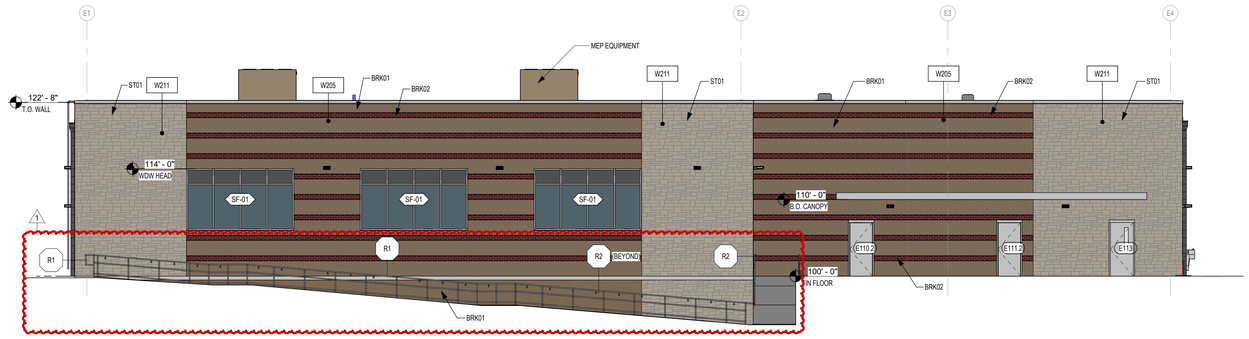
6 EXTERIOR ELEVATION - AREA E - EAST
1/8" = 1'-0"



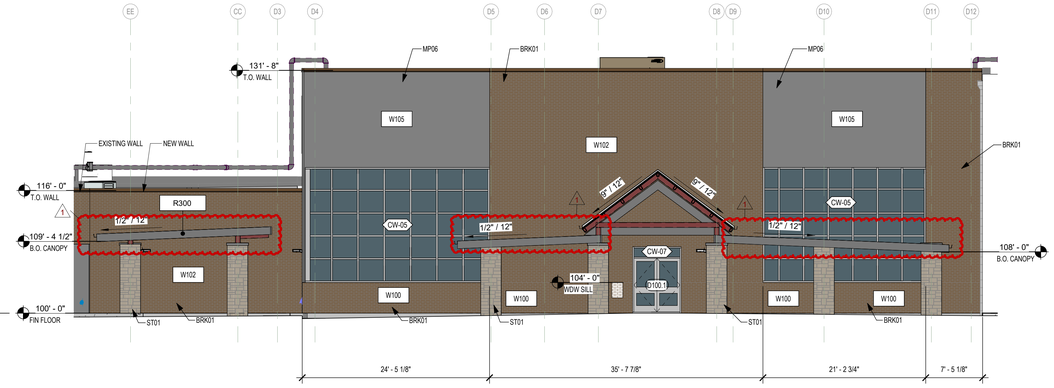
5 EXTERIOR ELEVATION - AREA D - SOUTH WEST
1/8" = 1'-0"



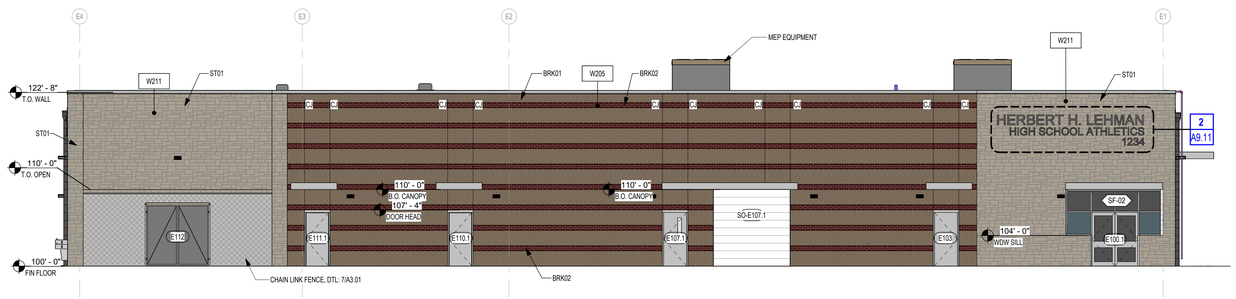
4 EXTERIOR ELEVATION - AREA E - WEST
1/8" = 1'-0"



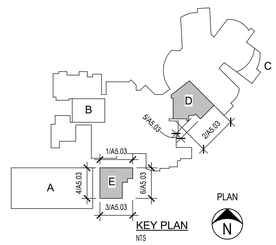
3 EXTERIOR ELEVATION - AREA E - SOUTH
1/8" = 1'-0"



2 EXTERIOR ELEVATION - AREA D - SOUTH EAST
1/8" = 1'-0"



1 EXTERIOR ELEVATION - AREA E - NORTH
1/8" = 1'-0"



KEY PLAN
NTS

Revision / 1
Date 05/08/25
ADDENDUM 01

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT
CHARLES JEROME HUCKABEE
STATE OF TEXAS
29417
05.08.25

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EXTERIOR ELEVATIONS - AREA D & E

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A5.03
Drawn By: YR/AB	
Date: 05/08/2025	

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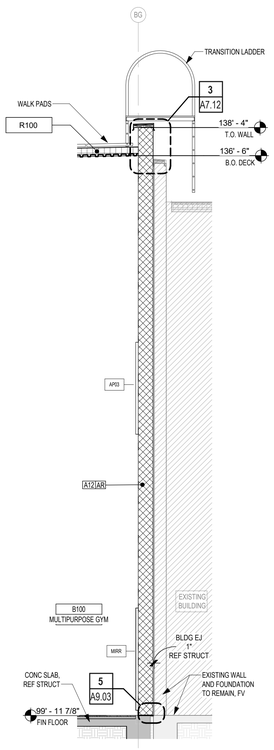
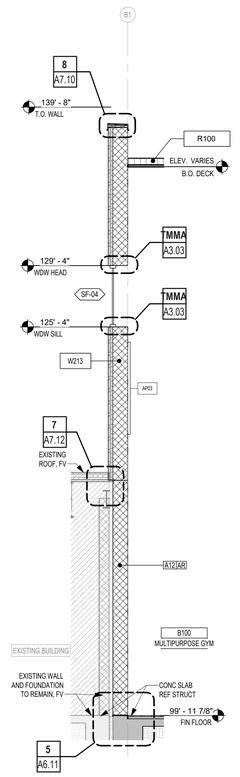
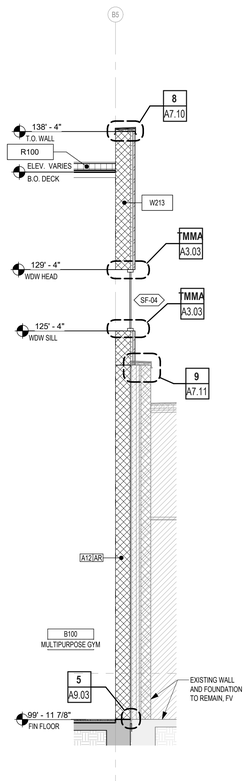


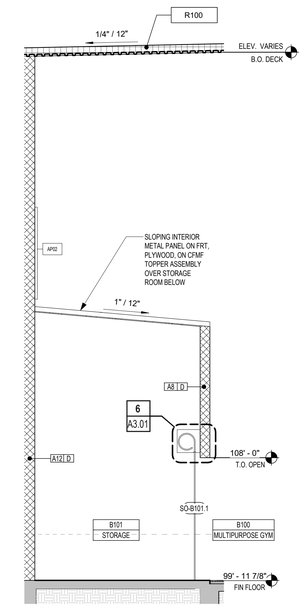
11 WALL SECTION
1/4" = 1'-0" BACK REF: A1.1B1



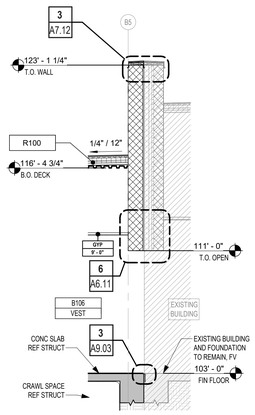
10 WALL SECTION
1/4" = 1'-0" BACK REF: A1.1B1



9 WALL SECTION
1/4" = 1'-0"



8 WALL SECTION
1/4" = 1'-0"



7 WALL SECTION
1/4" = 1'-0" BACK REF: A1.1B1

LETTER INDICATES TYPE

NUMBER INDICATES CONSTRUCTION AS FOLLOWS:

- **R100** - BUILT-UP, INSUL., MTL. DECK
- **R101** - BUILT-UP, INSUL., MTL. DECK
- **R102** - SINGLE PLY, INSUL., MTL. DECK
- **R103** - SINGLE PLY, INSUL., MTL. DECK
- **R104** - STANDING SEAM, 3/4" TREATED PW SHEATHING, INSUL., MTL. DECK
- **R105** - STANDING SEAM, 3/4" TREATED PW SHEATHING, INSUL., MTL. DECK
- **R106** - BUILT-UP, INSUL., CONC. DECK
- **R107** - SINGLE-PLY, INSUL., CONC. DECK

REF. SHT. G3.01 FOR ROOF CONSTRUCTION TYPES

R100
ROOF ASSEMBLY IN SECTION VIEW

ROOF TYPE LEGEND

1. REFER ALSO TO G3 SHTS FOR WALL CONSTRUCTION DETAILS. TYPICAL DETAILS APPLY, AS NEEDED, TO ALL WALL SECTIONS U.N.O.
2. REINFORCING IN CONCRETE BEAMS, FOOTINGS, & PIERS HAS NOT BEEN INDICATED FOR CLARITY. REFER TO STRUCTURAL DRAWINGS FOR THIS REINFORCING.
3. CONTRACTOR SHALL VERIFY THE DIMENSIONAL CONDITIONS BETWEEN ALL STRUCTURAL ELEMENTS & THE ARCHITECTURAL ELEMENTS FOR CONFLICTS. IF DISCREPANCIES ARE ENCOUNTERED, NOTIFY THE ARCHITECT IMMEDIATELY.
4. AT ALL EXTERIOR WALLS, PLACE WEATHER BARRIER AT THE EXTERIOR FACE OF CMU OR SHEATHING. CONTRACTOR TO VERIFY CHEMICAL COMPATIBILITY OF WEATHER BARRIER & SELF-ADHESIVE FLASHING.
5. PROVIDE 3 STACKED BRICK HEADERS OR ONE SOLID BRICK AT ALL SOLDIER OUTSIDE CORNERS. (4"W"X3 5/8")
6. PROVIDE A WEATHERED MORTAR JOINT AT ALL EXTERIOR CONDITIONS WHERE THE MASONRY COURSE BELOW THE JOINT PROJECTS BEYOND THE COURSE ABOVE.
7. WHERE CMU IS SHOWN SHADED, FILL CELL SOLID WITH GROUT UNLESS NOTED IN DETAILS AS MORTAR. COORDINATE ALL LOCATIONS WITH STRUCTURAL DRAWINGS. IN CASES OF APPARENT CONFLICT, STRUCTURAL DRAWINGS SHALL GOVERN DESIGN. GROUT
8. FILL ALL OPEN CELLS OF CMU SOLID WITH FOAM INSULATION AT ALL EXTERIOR CONDITIONS WHERE RIGID INSULATION IS NOT APPLIED IN THE CAVITY WALL.
9. PROVIDE THERMAL BATT INSULATION AT ALL EXTERIOR STUD WALLS & ACOUSTICAL BATT INSULATION AT INTERIOR STUD WALLS AS DESCRIBED IN SECTION 07 2100 OF THE SPECIFICATIONS.
10. REFER TO STRUCTURAL DRAWINGS FOR VERTICAL WALL REINFORCING.
11. ALL MASONRY WALLS SHALL BE REINFORCED WITH CONTINUOUS HORIZONTAL BED/JOINT REINFORCING AS SPECIFIED, AT 16" VERTICAL SPACING. AT VENEER CONDITIONS PROVIDE TIES AS SPECIFIED AT 16" O.C.E.W.
12. INFORMATION SHOWN BEYOND IN WALL SECTIONS IS PROVIDED FOR INFORMATION ONLY & SHALL NOT LIMIT THE SCOPE OF WORK REQUIRED IN THIS PROJECT.
13. SMALLER SCALE DRAWINGS MAY NOT SHOW ALL CONNECTIONS & FINISHES. ALWAYS REFER TO ENLARGED DETAILS, WHERE PROVIDED, FOR ADDITIONAL INFORMATION. IN CASES OF APPARENT CONFLICT, THE LARGER SCALED DETAIL SHALL APPLY.
14. INTERIOR WALL FINISHES ARE NOT SHOWN ON WALL SECTIONS TO AVOID VISUAL CLUTTER. REFER TO A4 SHEETS FINISH SCHED & INTERIOR ELEVATIONS FOR REQUIRED MATERIALS & MATERIAL PATTERNS.

TYPICAL WALL SECTION NOTES

APPLICABLE TO ALL A4 SHEETS

LETTER INDICATES EXTERIOR WALL

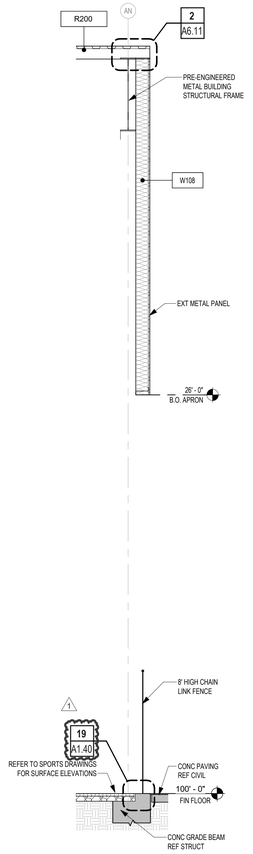
W200
WALL ASSEMBLY IN SECTION VIEW

NUMBER INDICATES BACKUP CONSTRUCTION AS FOLLOWS:

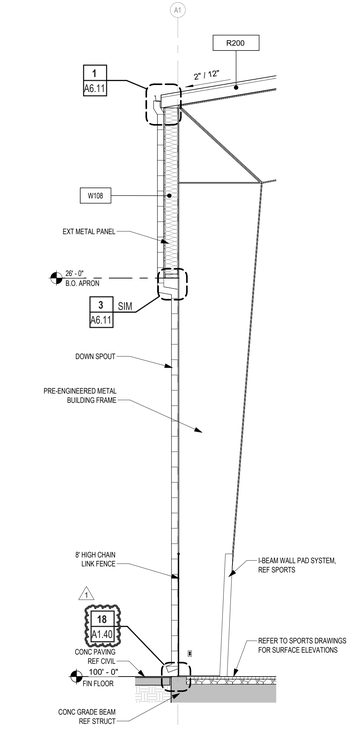
- **100** - CFMF
- **200** - CMU
- **300** - ICF
- **400** - STRUCTURAL CONC. PRECAST PANEL

REF. SHT. G3.01 FOR SPECIFIC WALL TYPES

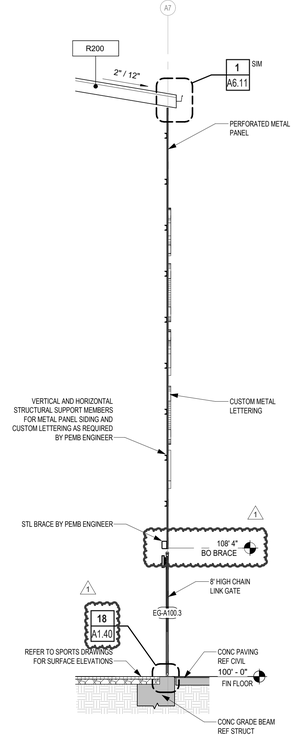
EXTERIOR WALL TYPE LEGEND



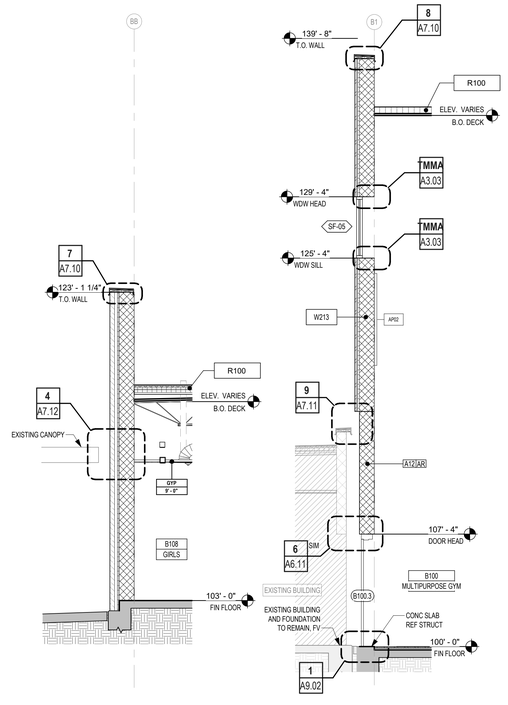
3 WALL SECTION
1/4" = 1'-0" BACK REF: A1.1A1



2 WALL SECTION
1/4" = 1'-0" BACK REF: A1.1A1



1 WALL SECTION
1/4" = 1'-0" BACK REF: A1.1A1



6 WALL SECTION
1/4" = 1'-0"

5 WALL SECTION
1/4" = 1'-0" BACK REF: A1.1B1

4 WALL SECTION
1/4" = 1'-0" BACK REF: A1.1B1

Project: LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

Revision /	Date	
1	05/08/25	ADDENDUM 01

REGISTERED ARCHITECT
JAMES JASON JOHNSON
29417
STATE OF TEXAS
05.08.25

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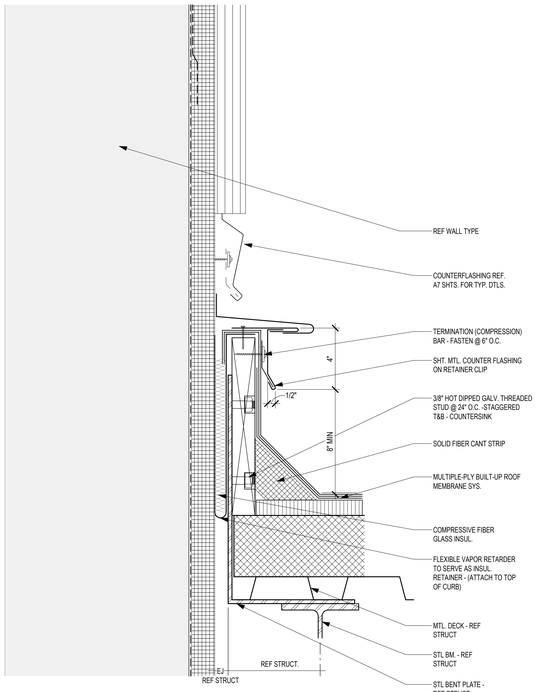
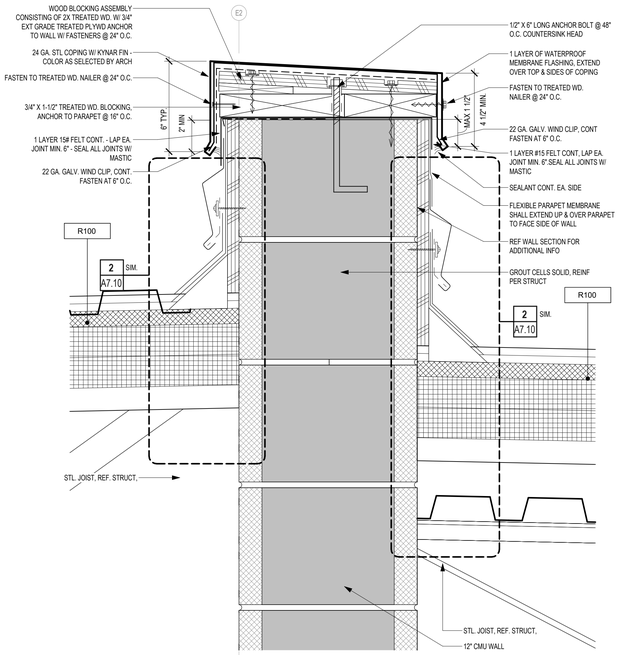
WALL SECTIONS - AREAS A, B

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ISSUE FOR BID A6.05
Drawn By: YR/AB	
Date: 05/08/2025	

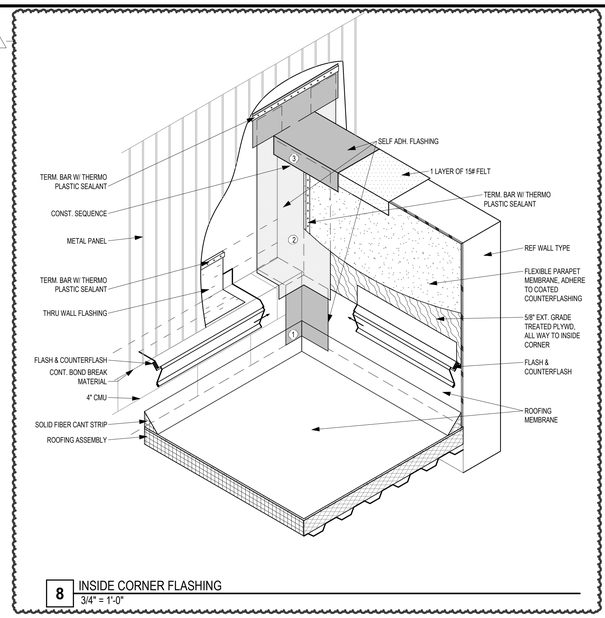
5/8/2025 2:19:42 PM
Autodesk Docs://HAYS CISD - LEHMAN, JOHNSON HS ADD RENO 2025/01954-08-01_LHS2025_Y23.rvt



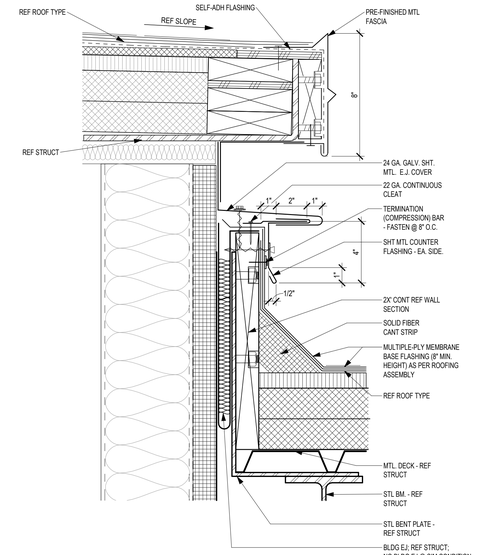
7 EJ AT EXISTING ROOF TO NEW WALL
3" = 1'-0"



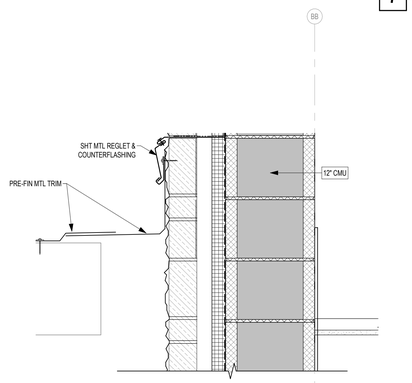
6 A7_CMU DIVIDING WALL - COPING DETAIL
3" = 1'-0" BACK REF:



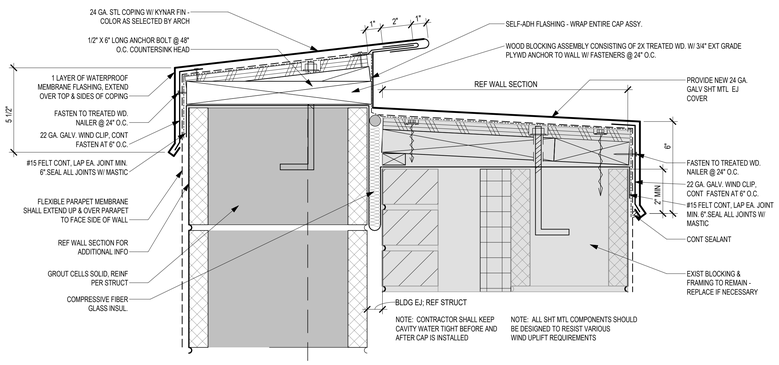
8 INSIDE CORNER FLASHING
3/4" = 1'-0"



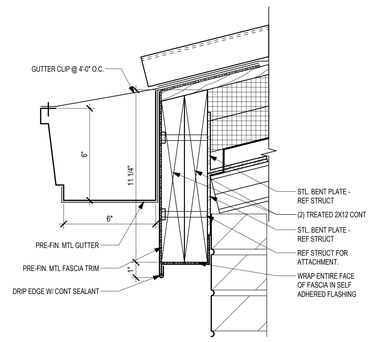
5 DETAIL - GRAVEL GUARD EDGE ABOVE EJ
3" = 1'-0"



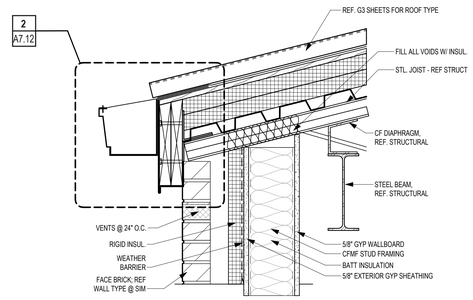
4 WSD - EXTERIOR WALL TO CANOPY FLASHING
1 1/2" = 1'-0" BACK REF: A6.05



3 PARAPET @ EJ NEW WALL TO EXIST WALL
3" = 1'-0"



2 FASCIA DETAIL
3" = 1'-0"



1 EAVE DETAIL
1 1/2" = 1'-0"

Revision / 1 — Date 05/08/25 — ADDENDUM 01

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

REGISTERED ARCHITECT — JAMES A. HUCKABEE — 29417 — STATE OF TEXAS
05.08.25

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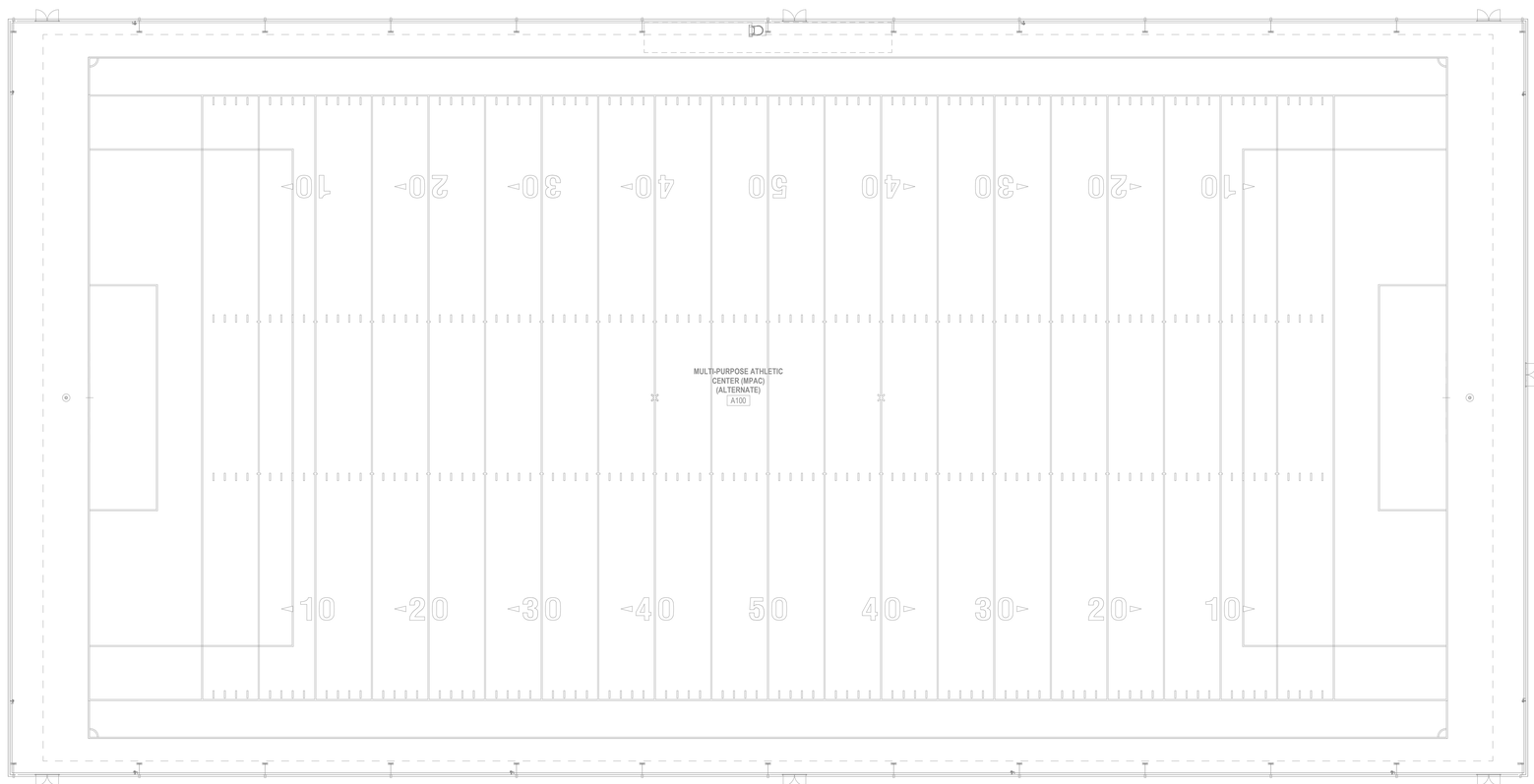
TYPICAL COMMON ROOFING DETAILS

PACKAGE | VOLUME

Job No. 01854-08-01
Drawn By: VSS
Date: 05/08/2025
Sheet No. ISSUE FOR BID A7.12

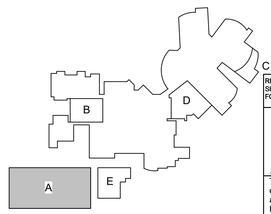
5/8/2025 2:26:03 PM
Autodesk Docs://HAYS CISD - LEHMAN, JOHNSON HS ADD RENO 2025/01954-08-01_LHS2025_V23.rvt



NO PLUMBING SCOPE. REFERENCE CIVIL DRAWINGS AND SPORTS/CEI DRAWINGS FOR HOSE BIBBS SERVING THIS BUILDING.

01 FIRST FLOOR PLAN - AREA A - PLUMBING

SCALE: 1/16" = 1'-0"



KEY PLAN
NTS

Revision /	Date	
1	05/08/25	Addendum No. 1

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

STATE OF TEXAS
B.J. HENDRIX
94813
LICENSED
PROFESSIONAL ENGINEER

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REFERENCE GENERAL NOTES ON SHEETS M0.01, P0.01, AND E0.01 FOR ADDITIONAL INFORMATION

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HCE job no.: 24-034

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FIRST FLOOR PLAN - AREA A - PLUMBING

PACKAGE	VOLUME
Job No. 019S4-08-01	Sheet No. P2.01A1
Drawn By: RD, KAM	ISSUE FOR BID
Date: 04/22/2025	

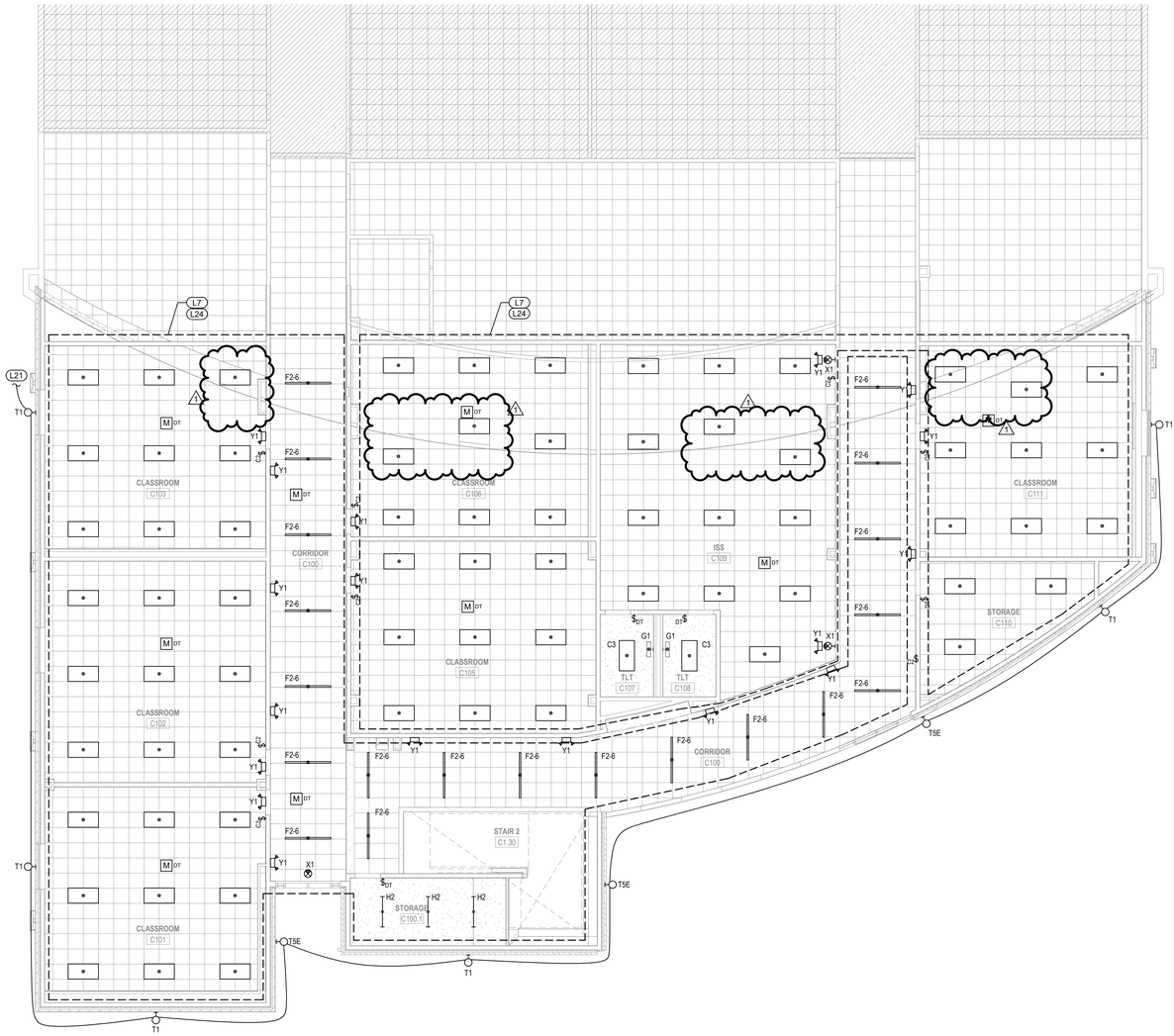
5/6/2025 9:11:25 AM
Autodesk Docs://HAYS CISD - LEHMAN, JOHNSON HS ADD RENO 2025/HS2025_MEP23.rvt

LIGHTING KEY NOTES

THESE NOTES APPLY TO THIS SHEET ONLY

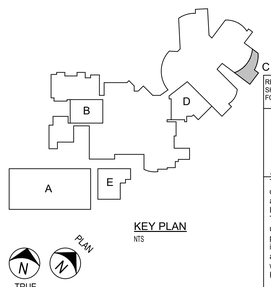
L7	CIRCUIT TO EXISTING 277V LIGHTING PANEL SERVING AREA TO SPARE 20A/1P BREAKER. IF NO BREAKER AVAILABLE, PROVIDE AND INSTALL 20A/1P BREAKER IN PANEL. WIRE/CONDUIT MARK #2.
L21	CONNECT TO EXISTING EXTERIOR LIGHTING CIRCUIT. WIRE/CONDUIT MARK #2.
L24	ALL FIXTURES IN THIS REGION WITHIN DASHED LINE TO BE CIRCUITED TO THE CIRCUIT LISTED.

ALL LIGHT FIXTURES ARE TYPE 'A4' UNLESS NOTED OTHERWISE.



01 FIRST FLOOR PLAN - AREA C - LIGHTING

SCALE: 1/8" = 1'-0"



KEY PLAN
NTS

Revision / Date
1 05/08/25 Addendum No. 1

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
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FIRST FLOOR PLAN - AREA C - LIGHTING

PACKAGE VOLUME

Job No. 01954-08-01

Drawn By: PP

Date: 04/22/2025

Sheet No. ISSUE FOR BID
E2.01C1

5/7/2025 2:02:17 PM
Autodesk Docs://HAYS CISD - LEHMAN, JOHNSON HS ADD RENO 2025/LHS2025_MEP2.rvt

LIGHTING KEY NOTES

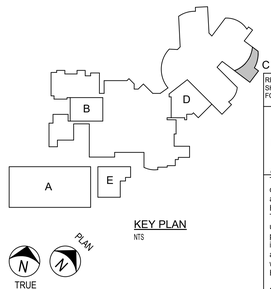
THESE NOTES APPLY TO THIS SHEET ONLY

L1	LOW VOLTAGE LIGHTING CONTROL BUTTON. REFERENCE MISCELLANEOUS EQUIPMENT SCHEDULE.
L7	CIRCUIT TO EXISTING 277/V LIGHTING PANEL SERVING AREA TO SPARE 20A/1P BREAKER. IF NO BREAKER AVAILABLE, PROVIDE AND INSTALL 20A/1P BREAKER IN PANEL. WIRE/CONDUIT MARK #2.
L24	ALL FIXTURES IN THIS REGION WITHIN DASHED LINE TO BE CIRCUITED TO THE CIRCUIT LISTED.

ALL LIGHT FIXTURES ARE TYPE 'A4' UNLESS NOTED OTHERWISE.

01 SECOND FLOOR PLAN - AREA C - LIGHTING

SCALE: 1/8" = 1'-0"



Revision /	Date	
1	05/08/25	Addendum No. 1

Project:

LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
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KYLE, TX

STATE OF TEXAS
P.J. HENDRIX
94813
LICENSED
PROFESSIONAL ENGINEER
ON: 04/22/2025

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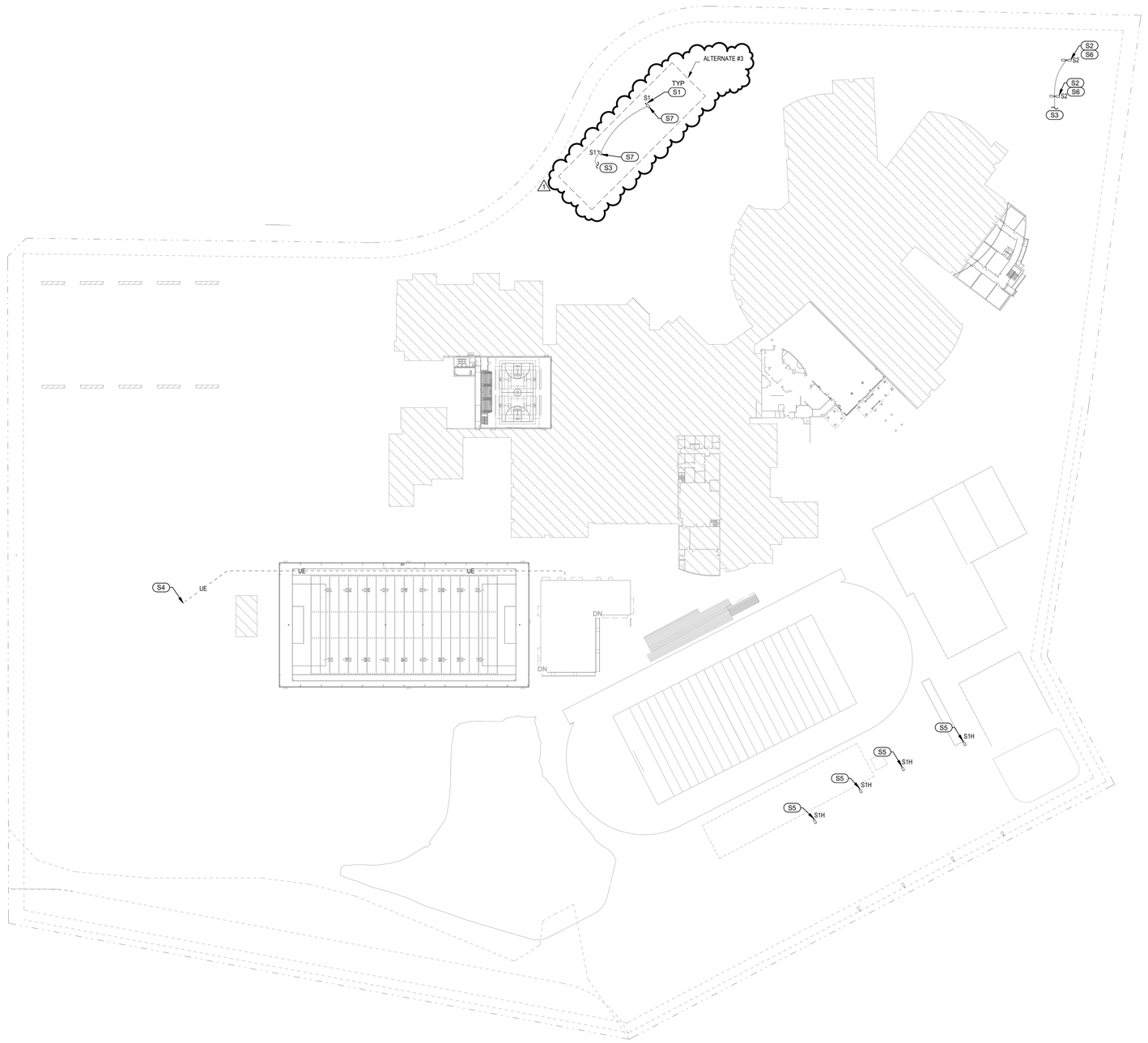
SECOND FLOOR PLAN - AREA C - LIGHTING

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. E2.02C2
Drawn By: PP	ISSUE FOR BID
Date: 04/22/2025	

SITE KEY NOTES

THESE NOTES APPLY TO THIS SHEET ONLY

S1	COORDINATE FINAL POLE LIGHT LOCATIONS WITH CIVIL AND LANDSCAPE CONSULTANTS PRIOR TO DRILLING PIERS. REFERENCE STRUCTURAL ENGINEER FOR STRUCTURAL POLE BASE DETAILS.
S2	EXISTING UNDERGROUND PRIMARY IN AREA. CONTRACTOR TO FIELD VERIFY EXISTING ROUTING AND ADVISE ENGINEER IF NEW POLE LOCATIONS ARE IN CONFLICT WITH EXISTING UNDERGROUND ELECTRICAL.
S3	EXTEND EXISTING SITE LIGHTING CIRCUIT IN AREA TO NEW FIXTURES.
S4	EXISTING BASEBALL/SOFTBALL SPORTS COMPLEX ELECTRICAL SERVICE YARD.
S5	REPLACE EXISTING 30' POLE 'S1' FIXTURE WITH 40' POLE AND TYPE S1H FIXTURE. 'S1' FIXTURE TO BE USED AT NEW POLE LOCATIONS AT NORTH OF SITE. USE EXISTING CIRCUITING.
S6	USE POLE AND HEAD REMOVED FROM BAND PRACTICE AREA. DRILL EXISTING POLE FOR NEW HEAD AND PROVIDE ADDITIONAL HEAD AS SPECIFIED.
S7	USE POLE AND HEAD REMOVED FROM BAND PRACTICE AREA. AT THIS LOCATION.



ALL SITE CIRCUITING IS DIAGRAMMATIC ONLY. DOES NOT INDICATE CONDUIT ROUTING. ELECTRICAL CONTRACTOR IS TO DETERMINE ALL FINAL CONDUIT ROUTING, COORDINATED WITH ALL SITE UTILITIES AND SITE CONDITIONS. REFERENCE CIVIL AND LANDSCAPE PLANS FOR ADDITIONAL INFORMATION.

01 **SITE PLAN - ELECTRICAL**
SCALE: 1" = 80'-0"

PLAN NORTH
TRUE NORTH

Revision /	Date	
1	05/08/25	Addendum No. 1

Project:
LEHMAN HIGH SCHOOL
2025 ADDITIONS + RENOVATIONS
FOR
HAYS CISD
KYLE, TX

STATE OF TEXAS
BRIAN J. HENDRIX
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SITE PLAN - ELECTRICAL

PACKAGE	VOLUME
Job No. 01954-08-01	Sheet No. ES1.00
Drawn By: PP	ISSUE FOR BID
Date: 04/22/2025	

5/7/2025 2:00:39 PM
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